consulting engineers

Transportation Assessment Report

for

Construction of Pipeline & Wastewater Treatment Plant

for

DAWN Meats - Slane
Greenhills, Beauparc, Navan,
Co. Meath,

FINAL ISSUE

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EXECUTIVE SUMMARY

NRB Consulting Engineers Ltd were appointed to address the Traffic & Transportation issues associated with the construction of a Wastewater Treatment Plan and associated Sewer Pipe-laying for DAWN Meats - Slane, Greenhills, Beauparc, Navan, Co. Meath.

The site is long established and the proposed works when operational will, in reality, result in reduced traffic demand. The completed works will remove existing traffic volumes associated with current de-sludging and decanting operations when the pipeline and wastewater treatment plant is installed. This Report addresses the traffic impact associated with the construction of the wastewater treatment plant in tandem with the sewer laying works along the route as illustrated herein as **Appendix A**.

This Transportation Assessment Report (TA) has been prepared to address the Traffic and Transportation issues associated with the construction works, the capacity of the existing road network and the impact of the temporary very small increases in traffic locally. The Report has been prepared in accordance with TII's Traffic & Transportation Assessment Guidelines and addresses the worst case traffic impact of the proposal with both construction operations proceeding.

Comprehensive classified turning movement surveys of the existing affected roads and junctions were carried out during the weekday AM and PM Peak Hours in 2020. These surveys, undertaken in the normal school term, formed the basis of the study. We have utilised adjacent permanent TII Traffic Counters on the N2 to apply 'Covid Factors' to the survey data, and this represents industry standard practice during pandemic times.

The analysis includes the effects of the existing traffic on the local roads and assesses the impact during the traditional peak commuter peaks periods in accordance with Traffic & Transportation Assessment Guidelines.

The Transportation Assessment confirms that the road network and the affected junction capacities are more than adequate to accommodate the worst case traffic associated with the operations. It should be acknowledged that the completion of the works, and the operation of an on-site treatment plant will result in less traffic on the local roads, as in future there will be no requirement to remove bulk sludge for off-site treatment.

Based on our studies, we believe that there are no adverse traffic/transportation capacity or operational issues associated with the construction operations that would prevent planning permission being granted by Meath County Council.



1.0 INTRODUCTION

- 1.1 This Transportation Assessment (TA) has been prepared by NRB Consulting Engineers Ltd and addresses the Traffic/ Transportation issues arising from the proposed construction of the Dawn Meats Slane wastewater treatment plant and the associated public road pipe-laying works.
- 1.2 The DAWN Meats Slane plant is long established in the area and in these terms has very well established traffic generation characteristics in its own right. The proposed works when complete will actually result in reduced traffic as it will mean that waste will no longer have to be regularly removed off-site by heavy goods vehicles for onward treatment. A site location plan for the DAWN site, together with details of the local road Junctions within the Network Area of Influence, is included below as *Figure 1.1*;

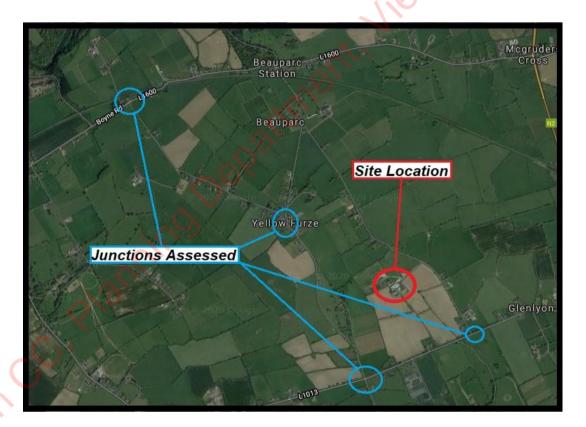


Figure 1.1 - Site Location

1.3 In describing the Receiving Environment and the Roads Environment during the construction works, this report addresses the following aspects of the proposed development:



- Relatively Small Scale of the construction traffic volume in the context of the local road network (Reflected in the very Low Traffic Volume Projections associated with the Construction Works),
- Location of the development within a long established lightly trafficked local road network adjacent the N2 National Road,
- Traffic & Transportation impact,
- Capacity of the Existing Road Network,
- Adequacy of Capacity and safety of the existing roads and junctions locally, within the area of influence.
- Impact upon the adjacent affected junctions locally as per Figure 1.1.
- 1.4 A review of the Road Safety Authority (RSA) online collision database indicates that there are no significant accidents on the affected stretches of road network surrounding the site. There have been a few 'minor' accidents locally as per the illustration below but the statistics are expected to be unaffected by the proposed works.
- 1.5 An extract from the RSA on-line collisions record is included below as *Figure 1.2*.

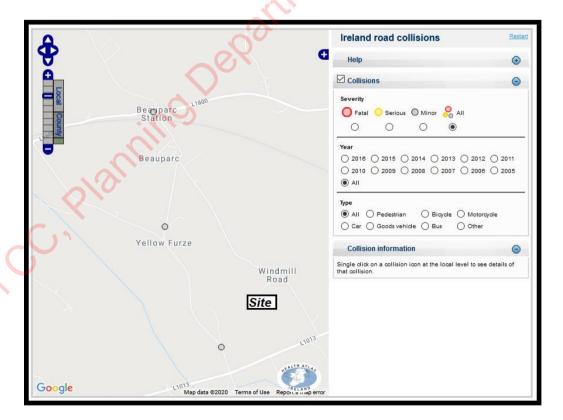


Figure 1.2 - Extract from RSA On-Line Record of Traffic Collisions



- 1.6 The Recommendations contained within this Transportation Assessment are based on the following sources of information and industry-standard practices; -
 - TII Traffic & Transport Assessment Guidelines,
 - Recent Traffic Survey Data, supplemented by TII Traffic Census Data,
 - Relevant Design Guidance,
 - Predicted Construction Traffic AADT Volumes provided by the Consulting Engineer for the Project,
 - Our experience in assessing the impact of Developments of this Nature, and
 - Site Visits and Observations.
- 1.7 The Report has been prepared in accordance with the requirements of the TII's Traffic & Transport Assessment Guidelines. These are the professional Guidelines used to assess the impact of developments on public roads.



2.0 DEVELOPMENT PROPOSALS & EXISTING ROAD CONDITIONS

2.1 The development consists of the construction of a Wastewater Treatment Plant on the site of DAWN Meats - Slane and also the construction of the associated outfall pipe to the River Boyne as detailed on the Engineers Drawings enclosed as *Appendix A*, with an annotated extract included below as *Figure 2.1* for ease of reference. The site compound for the works will be within the DAWN Site.

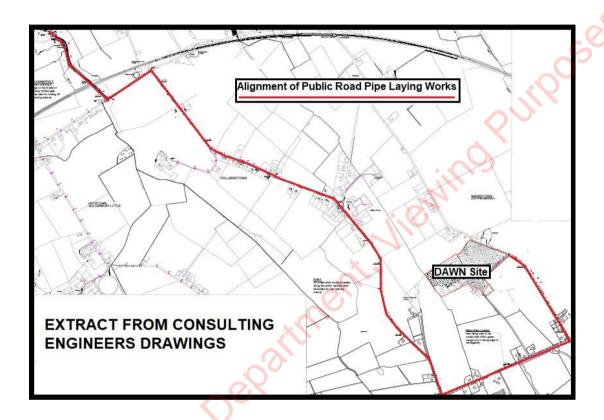


Figure 2.1 – Annotated Extract from Engineers Drawings (Appendix A)

2.2 It is proposed to locate the Site Compound on the DAWN Site, accessible via the L1303 via the adjacent N2. The works compound and DAWN is located approximately 1.3km west of the N2, and in these terms is considered ideal in terms of the import of the expected small volumes of materials, machinery and construction staff. The location is illustrated below as *Figure 2.2*





Figure 2.2 – Location of DAWN Site & Proximity to N2

2.3 The junction of the N2 and the L1303 takes the form of a large capacity priority junction with the provision of a dedicated right turn lane. The form of junction will ensure that the small volumes of construction related traffic will have easy & safe access to the construction compound from the N2. An image of the junction is included below as *Figure 2.3*

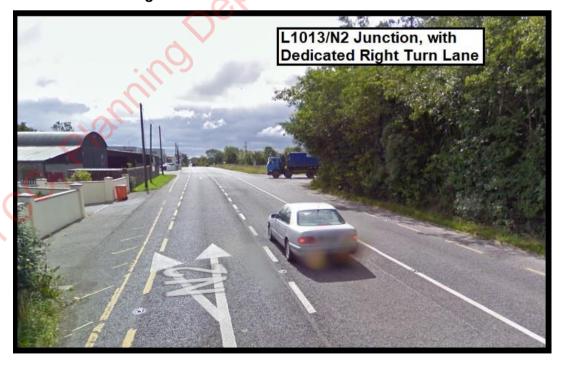


Figure 2.3 – N2/L1303 Boyne Rd (looking south)



- 2.4 The local roads surrounding the site are traditional and rural in nature, all consisting of single carriageway roads bounded by established hedgerows. Each of the affected roads are classified as Local Roads (with the "L" designation), and are all subject to an 80kph speed restriction. Whilst there is an 80kph designation, the speeds are self-regulating in terms of the undulating and rural nature of the carriageways. In terms of the capacity of the local roads, ultimately these have a theoretical traffic or link capacity of approximately 800-1,000 PCUs per direction per hour. The existing flows on the local roads need to be considered in this context.
- 2.5 The L1600 Boyne Road is orientated in an E-W direction and, in addition to the L1303, it can serves as a secondary or backup route from the N2 to the area. The L1600 currently carries a weekday AM Peak Hour 2-way flow of approximately 165 PCUs & a weekday PM Peak Hour 2-Way flow of approximately 171 PCUs. In these terms, it is clearly lightly trafficked in terms of its theoretical link capacity as illustrated above.
- 2.6 The local road past Yellow Furze, linking the L1600 to the L1013 to the south, carries a weekday AM Peak Hour 2-way flow of approximately 40 PCUs & a weekday PM Peak Hour 2-Way flow of approximately 45 PCUs. In these terms, it is clearly very lightly trafficked indeed.
- 2.7 The L1013 to the south, which will serve as the primary access from the N2, is also orientated in an E-W direction. It has a primary high quality access to/from the N2 by way of a large capacity Priority Junction (with Right Turn Ghost Island). It currently carries a weekday AM Peak Hour 2-way flow of approximately 90 PCUs & a weekday PM Peak Hour 2-Way flow of approximately 155 PCUs. In these terms, it is also considered lightly trafficked in terms of its theoretical link capacity as illustrated above.
- 2.8 Of course, the capacity of any road or link is ordinarily determined by the capacity of terminal junctions, and the capacity of junctions is therefore addressed in more detail within the main body of this report.



3.0 VEHICULAR TRIP GENERATION, ASSIGNMENT & DISTRIBUTION

- 3.1 The Trip Rate Information Computer System (TRICS) database is ordinarily used to ascertain vehicular trip generation associated with the use of any particular site. This represents industry standard practice for Transportation Assessments in Ireland, in the assessment of traditional commercial developments. In this case, an assessment of construction traffic impact is required, and an alternative traffic generation calculation methodology is used to quantify the traffic generated by the construction operations.
- 3.2 An assessment of the worst case daily traffic volumes associated with both the pipe laying works (on the public roads) and the waste water treatment plant (within DAWN Meats) has been undertaken and this is illustrated below as *Figure 3.1*

Construction-Activity#	Movements-Generated¤		
	LGV¤ 🗸	HGV¤	p
Extension-to-WWTP-(Phase-1)¤	40¤	30¤	p
Rising-Main-Pipeline [®]	60¤	40¤	ŭ

Table-3-1:-Annual-Average-Daily-Traffic-(Construction-Traffic)

Figure 3.1 Daily Traffic Volumes Generated by Construction Operations

3.3 Through the use of standard vehicle conversion factors to convert vehicle types to Passenger Car Units ('Car Equivalents'), we have utilised this information to quantify the traffic generated by both construction projects. In this case, a Light Goods Vehicle (LGV) is equivalent to 1 PCU and a Heavy Goods Vehicle to 2 PCUs for assessment purposes. This is illustrated below as *Table 3.2* and *Table 3.3*.

Table 3.2 – Total Traffic Generated by Waste Water Treatment Plant Works (PCUs)

Natural Paris I	Arrivals	Departures	
Network Period	(PCUs)	(PCUs)	
24 Hr AADT	100	100	
Weekday AM Peak Hr*	13	13	
Weekday PM Peak Hr*	13	13	



Table 3.3 – Total Traffic Generated by Pipe Laying Works (PCUs)

Notice als Device d	Arrivals	Departures
Network Period	Period (PCUs) (PCU	(PCUs)
24 Hr AADT	140	140
Weekday AM Peak Hr*	18	18
Weekday PM Peak Hr*	18	18

^{*} Industry Standard Method of Calculating Weekday Peak Hr is to Divide 24Hr AADT by 10 - We have Divided by 8 for Robustness, giving higher Peak Hr Flow Volumes for Assessment Purposes.

3.4 These volumes of traffic have then been applied to the established road network and the resultant impact considered. The assessment is undertaken in accordance with the Guidelines in the context of the demonstrably low levels of traffic generated by the proposed construction operations.

Assignment/Distribution - Future Year Traffic

- 3.5 We have used hand assignment techniques based on the observed patterns, with the worst case traffic assigned to the roads based on the observed established traffic patterns.
- The standard methodology applied was to firstly ascertain the base background traffic conditions for both the weekday AM and weekday PM Commuter Peak periods. We then used the TII Project Appraisal Guidelines Unit 5.3 (Travel Demand Projections Table 5.3.2) to establish worst case projected construction year 2022 on the local road network. (We selected 2022, however an earlier or indeed a later start to the construction will not have any material impact upon the conclusions).
- 3.7 The worst case traffic based on a combination of the content of *Table 3.2* and *Table 3.3* above was then applied in order to establish Construction Year Traffic Conditions for both the pipeline works and WWTP occurring in tandem. This is all included in the calculations included herein as *Appendix C*.
- 3.8 It should be noted that we have selected the construction year of 2022 as being reasonable and appropriate, however, in our experience varying this by 1-2 years will have no significant impact upon the conclusions of the study. In addition, given the favourable results reported in this study, if required to apply higher background traffic conditions for any reason we would not anticipate any changes whatsoever to the conclusions.



- 3.9 In light of the covid pandemic, we have used historic and publicly available TII Traffic Census Data from the nearby N2 Ashbourne/Slane to establish 'normal' non-covid pandemic traffic conditions on the local network. In essence, we applied a 'Covid Factor' to the surveyed AM & PM flows through this exercise, and this accords with normal accepted practice during these emergency times. This is as set out in *Appendix C* (Page 1).
- 3.10 Normal traffic growth factors for future year assessments were calculated from data obtained in the TII Travel Demand Projections Unit 5.3, which provides the recommended method of predicting future year traffic growth on Roads. Calculations of the relevant growth factors are included in *Table 3.4* below (based on tabulated 'medium growth' in the Meath Area).

Table 3.4 - Traffic Growth Rates, TII Travel Demand Projections Unit 5.3

Year	to Year	Table 5.5.1:
2020	2022	1.022

3.11 The resulting Traffic Flow Projections and Figures within *Appendix C* allowed the assessment of impact of the construction operations to be undertaken.



4.0 TRAFFIC IMPACT - THRESHOLD ASSESSMENT/TRAFFIC CAPACITY ANALYSIS

- 4.1 The Institution of Highways and Transportation (IHT) Guidelines for Traffic Impact Assessment and the TII Traffic and Transport Assessment Guidelines sets out a methodology for assessment of developments of this nature and determining whether further assessment is indeed required.
- 4.2 This TII Traffic and Transport Assessment Guidelines normally requires a **Threshold Assessment** of the impact on the local roads to be provided in order to determine whether further more detailed modelling and assessment of particular critical junctions is necessary. However, in this case, given the very lightly trafficked nature of the local roads even a small increase in traffic related to construction activities can have a significant Net Effect.
- 4.3 For robustness we have therefore undertaken an assessment of the worst case traffic capacity impact, with junction capacity modelling undertaken for each of the 4 local junctions within the network area of influence of the construction works;
 - The Windmill Rd/L1013 T-Junction near DAWN Meats,
 - The L1013/Yellow Furze Rd Crossroads to the west.
 - The T-Junction at Yellow Furze, and
 - The L1600 Boyne Road T-Junction nearest the site compound.
- 4.4 Clearly the WWTP Works at Dawn Meats is a fixed site as a construction traffic destination, however the pipelaying contract works will move throughout the programme as the piping progresses. In this regard, we have undertaken an assessment of the progression of the pipe contract works in 5 separate locations (referred to herein as 'Scenarios') in order to determine the worst case traffic demands at each junction. Traffic to and from the works sites will originate to/from the dedicated site compound at the Dawm Meats Plant.
- 4.5 The scenarios considered are;
 - Scenario A WWTP works at DAWN & Pipelaying on Windmill Rd.,
 - Scenario B WWTP works at DAWN & Pipelaying on L1013.
 - Scenario C WWTP works at DAWN & Pipelaying South of Yellow Furze
 - Scenario D WWTP works at DAWN & Pipelaying North of Yellow Furze
 - Scenario E WWTP works at DAWN & Pipelaying on the L1600.



4.6 The entire assessment is included herein as Appendix C. A summary showing the worst case traffic conditions at each of the local junctions is below as Table 4.1

Table 4.1 – Total Traffic Generated by Pipe Laying Works (PCUs)

	Total Worst Case Predicted Traffic Flow Through Junction									
Network Junction	Scenario A		Scenario B		Scenario C		Scenario D		Scenario E	
	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM
Windmill Rd/L1013	271	253	270	252	270	252	270	252	270	252
L1013 Crossroads	182	194	183	194	218	229	218	229	218	229
T Junction Yellow Furze	42	47	42	47	42	47	77	82	77	82
L1600/Yellow Furze T Junc	216	242	216	242	216	242	216	242	251	277

- 4.7 The assessment demonstrates that **Scenario E** unsurprisingly represents the worst case in traffic terms at each of the network junctions. Therefore a junction capacity assessment for Scenario E at each junction has been undertaken.
- 4.8 We have used the TII-approved software package 'Junctions 9' PICADY' (**P**riority Intersection **C**apacity and **D**elay) software package (as part of the TRL Package 'Junction 9') to assess the capacity of each of the junctions. PiCADY produces results based on a ratio of flow to capacity (RFC) and period mean-maximum queue length. An RFC greater that 1.00 indicates that a junction is operating at or above capacity, with 0.85 considered to be the optimum RFC value. We have appended the detailed computer simulation model results (PiCADY Outputs) of the junction modelling for the proposed site access in *Appendix D, E, F and G*. Summaries of the results of the modelling is included below.

Windmill Rd/L1013 T-Junction

4.9 The detailed simulation output is included as **Appendix D**, and a summary of the results is reproduced below as **Table 4.2**

Table 4.3 - Junctions9 PiCADY Summary Results, Windmill Rd/L1013 T-Junction

Modelled Scenario	Period Mean Max Q (PCUs)	Period Max RFC
2022 Construction Year AM Peak	<1	0.11
2022 Construction Year PM Peak	<1	0.10

L1013 Crossroads Junction

The detailed simulation output is included as **Appendix E**, and a summary of the results is reproduced below as **Table 4.4**

Table 4.4 - Junctions 9 PiCADY Summary Results, L1013 Crossroads Junction

Modelled Scenario	Period Mean Max Q (PCUs)	Period Max RFC
2022 Construction Year AM Peak	<1	0.07
2022 Construction Year PM Peak	<1	0.08



T-Junction at Yellow Furze

4.11 The detailed simulation output is included as *Appendix F*, and a summary of the results is reproduced below as *Table 4.5*

Table 4.5 - Junctions 9 PiCADY Summary Results, Yellow Furze T Junction

Modelled Scenario	Period Mean Max Q (PCUs)	Period Max RFC
2022 Construction Year AM Peak	<1	0.02
2022 Construction Year PM Peak	<1	0.01

L1600 Boyne Rd T-Junction (North of Yellow Furze)

4.12 The detailed simulation output is included as *Appendix G*, and a summary of the results is reproduced below as *Table 4.6*

Table 4.6 - Junctions9 PiCADY Summary Results, Boyne Rd T-Junction North of Yellow Furze

Modelled Scenario	Period Mean Max Q (PCUs)	Period Max RFC
2022 Construction Year AM Peak	<1	0.07
2022 Construction Year PM Peak	<1	0.7

Conclusion - Modelling Above

4.13 The results of the modelling above clearly shows that all of the affected junctions will have significantly more than adequate capacity to accommodate the worst case traffic associated with both construction projects progressing in tandem. All of the RFCs are **way below** the theoretical optimum capacity of 0.85 and no queuing whatsoever is anticipated.



5.0 CONCLUSIONS

- 5.1 This Transportation Assessment Report assesses the traffic and transportation impact associated with the construction of a pipelaying contract and a wastewater treatment plant (WWTP) running in tandem, associated with the DAWN Meats Slane Plant in Co Meath.
- 5.2 This Report has been prepared in accordance with the TII Traffic & Transport Assessment Guidelines, and it provides for an onerous and robust assessment of the impact of the construction traffic associated with the contracts.
- 5.3 The impact of the traffic on the local roads has been modelled and assessed, based on a comprehensive new classified vehicle turning movement survey undertaken for the purposes of this study, during normal school period, factored using industry standard methods to reflect non-Covid 19 Pandemic times.
- This report assessed the WWTP and the Pipelaying in tandem, assessing the worst case traffic at each junction associated with the entire length of the Pipe Laying works.
- 5.5 The assessment demonstrates that the construction traffic will have an absolutely negligible impact upon the established local traffic conditions and can easily be accommodated on the road network without any capacity concerns arising.
- 5.6 It should be remembered that, whilst there will be a very small increase in traffic conditions locally during the construction works, the construction of the WWTP will result in a medium and long term improvement in traffic conditions locally, as large tanker vehicles will no longer be required to export effluent from the site for off-site treatment.
- 5.7 It is considered that there are no significant Operational Traffic Safety or Road Capacity issues that prevent a positive determination of the application by Meath County Council.



APPENDICES - CONTENT

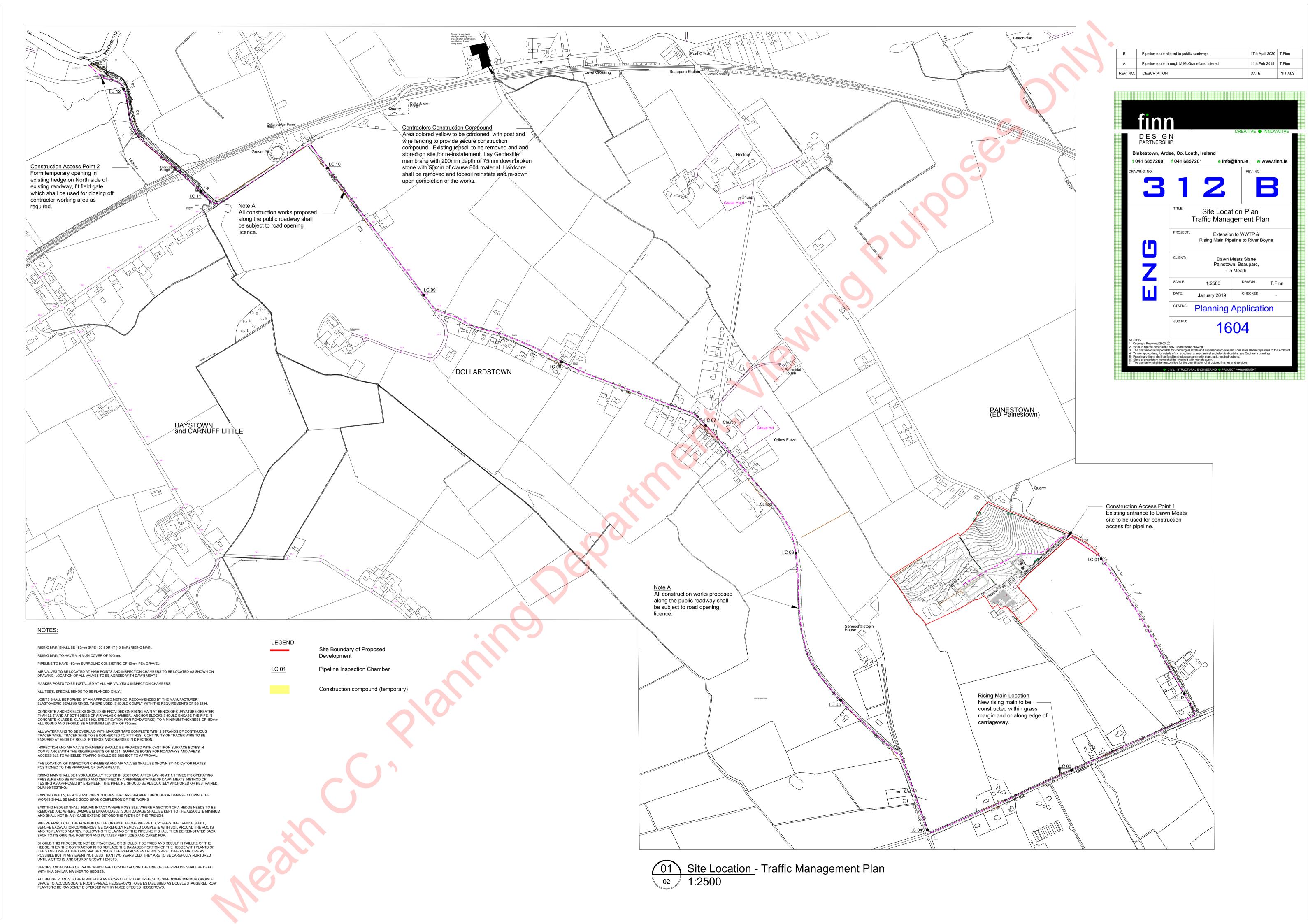
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APPENDIX A

Proposed Route of Pipe Laying Works (For Reference)

Meath, Co. Planning,





APPENDIX B

Raw Traffic Survey Data Output

Site Locations

Movement Numbering





Job number: TRA/20/123	Job date: November 2020	Drawing No: TRA/20/123-01	traffinomics
Client: NRB Consulting Engineers	Job day Midweek – School day	Map – Survey Location	w ie

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED JUNCTION TURNING COUNTS

NOVEMBER 2020 TRA/20/123

SITE: 01 DATE: 11th November 2020

LOCATION: L1600/Yellow Furze Road DAY: Wednesday

		N	/OVEN	MENT	1					ľ	MOVE	JENT	2					N	MOVE	MENT	3			
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
07:30	0	0	25	10	0	1	36	37	0	0	5	0	0	0	5	5	0	0	6	2	0	2	10	12
07:45	0	0	26	5	0	0	31	31	0	0	1	0	0	0	1	1	0	0	1	0	0	0	1	1
08:00	0	0	22	7	1	0	30	31	0	0	1	1	1	0	3	4	0	0	3	0	0	0	3	3
08:15	0	0	23	1	1	0	25	26	0	0	4	0	0	0	4	4	0	0	2	0	0	1	3	4
н/тот	0	0	96	23	2	1	122	125	0	0	11	1	1	0	13	14	0	0	12	2	0	3	17	20
08:30	0	0	23	2	1	0	26	27	0	0	4	2	0	0	6	6	0	0	0	0	0	0	0	0
08:45	0	0	18	4	1	0	23	24	0	0	6	1	0	0	7	7	0	0	1	4	0	0	5	5
09:00	0	0	16	4	0	1	21	22	0	0	18	1	0	1	20	21	0	0	20	2	0	1	23	24
09:15	0	0	8	1	0	0	9	9	0	0	11	0	0	0	11	11	0	0	22	2	0	0	24	24
н/тот	0	0	65	11	2	1	79	82	0	0	39	4	0	1	44	45	0	0	43	8	0	1	52	53
P/TOT	0	0	161	34	4	2	201	207	0	0	50	5	1	1	57	59	0	0	55	10	0	4	69	73

8 21 12

		N	NOVEN	MENT	1					N	JOVEN	/IENT	2					N	/OVEN	/IENT	3			
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
16:00	0	0	6	4	0	0	10	10	0	0	5	0	0	0	5	5	0	0	5	2	0	0	7	7
16:15	0	0	6	4	1	0	11	12	0	0	4	2	0	0	6	6	0	0	2	2	0	0	4	4
16:30	0	0	7	3	0	1	11	12	0	0	5	1	0	0	6	6	0	0	5	1	0	0	6	6
16:45	0	0	11	3	1	0	15	16	0	0	5	1	0	0	6	6	0	0	3	3	0	0	6	6
Н/ТОТ	0	0	30	14	2	1	47	50	0	0	19	4	0	0	23	23	0	0	15	8	0	0	23	23
17:00	0	0	10	3	0	0	13	13	0	0	2	0	0	0	2	2	0	0	5	1	0	0	6	6
17:15	0	0	12	0	0	0	12	12	0	0	4	0	0	0	4	4	0	0	3	1	0	0	4	4
17:30	0	0	11	1	0	0	12	12	0	0	3	1	0	0	4	4	0	0	4	0	0	0	4	4
17:45	0	0	7	0	0	0	7	7	0	0	5	0	0	0	5	5	0	0	4	1	0	0	5	5
н/тот	0	0	40	4	0	0	44	44	0	0	14	1	0	0	15	15	0	0	16	3	0	0	19	19
18:00	0	0	10	2	0	0	12	12	0	0	5	0	0	0	5	5	0	0	7	0	0	0	7	7
18:15	0	0	3	1	0	0	4	4	0	0	4	0	0	0	4	4	0	0	3	0	0	0	3	3
н/тот	0	0	13	3	0	0	16	16	0	0	9	0	0	0	9	9	0	0	10	0	0	0	10	10
P/TOT	0	0	83	21	2	1	107	110	0	0	42	5	0	0	47	47	0	0	41	11	0	0	52	52

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED JUNCTION TURNING COUNTS

NOVEMBER 2020 TRA/20/123

SITE: 01 DATE: 11th November 2020

LOCATION: L1600/Yellow Furze Road DAY: Wednesday

		N	/IOVE	MENT	4					Ŋ	MOVE	JENT	5					N	10VEN	/IENT	6			
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	2	0	0	8	8
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19	3	1	2	25	28
08:00	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	13	2	1	0	16	17
08:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	1	0	0	6	6
н/тот	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	43	8	2	2	55	59
08:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	3	1	0	15	16
08:45	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1	0	0	8	4	2	0	14	16
09:00	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1	0	0	10	3	1	0	14	15
09:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	5	1	1	17	19
н/тот	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	2	0	0	39	15	5	1	60	66
P/TOT	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	2	0	0	82	23	7	3	115	125

MOVEMENT 4 MOVEMENT 5 MOVEMENT 6 TIME PCL MCL CAR LGV **HGV BUS** TOT PCU PCL MCL CAR LGV HGV TOT **PCU PCL** MCL CAR LGV HGV TOT PCU 16:00 16:15 16:30 16:45 H/TOT 17:00 17:15 17:30 17:45 H/TOT 18:00 18:15 H/TOT P/TOT

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED JUNCTION TURNING COUNTS

NOVEMBER 2020 TRA/20/123

SITE: 02 DATE: 11th November 2020

LOCATION: Yellow Furze Village Junction DAY: Wednesday

		N	/OVE	MENT	1					N	MOVEN	MENT	2					N	MOVEN	JENT	3			
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
07:30	0	0	0	0	0	0	0	0	0	0	6	0	0	0	6	6	0	0	5	1	0	1	7	8
07:45	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	3	0	0	0	0	0	0	0	0
08:00	0	0	0	1	0	0	1	1	0	0	0	0	1	0	1	2	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0	0	5	0	0	0	5	5	0	0	0	0	0	1	1	2
н/тот	0	0	0	1	0	0	1	1	0	0	14	0	1	0	15	16	0	0	5	1	0	2	8	10
08:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	0	1	0	0	1	1	0	0	6	1	0	0	7	7	0	0	1	3	0	0	4	4
09:00	0	0	2	0	0	0	2	2	0	0	24	3	0	1	28	29	0	0	17	2	0	1	20	21
09:15	0	0	0	0	0	0	0	0	0	0	15	0	0	0	15	15	0	0	24	2	0	0	26	26
н/тот	0	0	2	1	0	0	3	3	0	0	45	4	0	1	50	51	0	0	42	7	0	1	50	51
P/TOT	0	0	2	2	0	0	4	4	0	0	59	4	1	1	65	67	0	0	47	8	0	3	58	61

MOVEMENT 1 MOVEMENT 2 MOVEMENT 3 TIME PCL MCL CAR LGV **HGV BUS** TOT PCU PCL MCL CAR LGV HGV TOT **PCU PCL** MCL CAR LGV HGV TOT **PCU** 16:00 16:15 16:30 16:45 H/TOT 17:00 17:15 17:30 17:45 H/TOT 18:00 18:15 H/TOT P/TOT

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED JUNCTION TURNING COUNTS

NOVEMBER 2020 TRA/20/123

SITE: 02 DATE: 11th November 2020

LOCATION: Yellow Furze Village Junction DAY: Wednesday

)			
		N	VOVE	MENT	4					ľ	MOVE	JENT	5					N	NOVEN	JENT	6			
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
07:30	0	0	1	0	0	0	1	1	0	0	1	0	0	0	1	1	0	0	0	1	0	1	2	3
07:45	0	0	1	0	0	0	1	1	0	0	1	0	0	0	1	1	0	0	1	0	0	0	1	1
08:00	0	0	0	0	0	0	1	0	0	0	2	0	0	0	2	2	0	0	4	0	0	0	4	4
08:15	0	0	1	0	0	0	1	1	0	0	0	0	1	0	1	2	0	0	1	0	0	0	1	1
н/тот	0	0	3	0	0	0	3	3	0	0	4	0	1	0	5	6	0	0	6	1	0	1	8	9
08:30	0	0	3	0	0	0	3	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	2	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
09:00	0	0	5	0	0	0	5	5	0	0	10	0	0	0	10	10	0	0	1	0	0	0	1	1
09:15	0	0	13	1	0	0	14	14	0	0	6	1	0	0	7	7	0	0	0	0	0	0	0	0
н/тот	0	0	23	1	0	0	24	24	0	0	16	1	0	0	17	17	0	0	1	1	0	0	2	2
P/TOT	0	0	26	1	0	0	27	27	0	0	20	1	1	0	22	23	0	0	7	2	0	1	10	11

MOVEMENT 4 MOVEMENT 5 MOVEMENT 6 PCU TIME PCL MCL CAR LGV HGV BUS TOT PCU PCL MCL CAR LGV HGV TOT PCU **PCL** MCL CAR LGV HGV TOT 16:00 16:15 16:30 16:45 H/TOT 17:00 17:15 17:30 17:45 H/TOT 18:00 18:15 H/TOT P/TOT

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED JUNCTION TURNING COUNTS

NOVEMBER 2020 TRA/20/123

SITE: 03 DATE: 11th November 2020

LOCATION: L1013/Windmill Road DAY: Wednesday

		N	/OVE	MENT	1					N	NOVEN	/ENT	2					N	MOVEN	JENT	3			
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
07:30	0	0	1	0	1	0	2	3	0	0	0	1	0	0	1	1	0	0	1	0	0	0	1	1
07:45	0	0	1	1	0	0	2	2	0	0	1	0	0	0	1	1	0	0	1	0	0	0	1	1
08:00	0	0	6	0	3	0	9	12	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
08:15	0	0	1	2	1	0	4	5	0	0	2	0	1	0	3	4	0	0	0	0	0	0	0	0
н/тот	0	0	9	3	5	0	17	22	0	0	3	1	1	0	5	6	0	0	3	0	0	0	3	3
08:30	0	0	2	0	1	0	3	4	0	0	1	0	0	0	1	1	0	0	2	1	0	0	3	3
08:45	0	0	2	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
09:00	0	0	3	0	0	0	3	3	0	0	2	0	0	0	2	2	0	0	1	0	0	0	1	1
09:15	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	2	0	0	1	0	0	0	1	1
н/тот	0	0	7	0	1	0	8	9	0	0	5	0	0	0	5	5	0	0	5	1	0	0	6	6
P/TOT	0	0	16	3	6	0	25	31	0	0	8	1	1	0	10	11	0	0	8	1	0	0	9	9

MOVEMENT 1 MOVEMENT 2 MOVEMENT 3 TIME PCL MCL CAR LGV HGV BUS TOT PCU PCL MCL CAR LGV HGV TOT PCU **PCL** MCL CAR LGV HGV TOT **PCU** 16:00 16:15 16:30 16:45 H/TOT 17:00 17:15 17:30 17:45 H/TOT 18:00 18:15 H/TOT P/TOT

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED JUNCTION TURNING COUNTS

NOVEMBER 2020 TRA/20/123

SITE: 03 DATE: 11th November 2020

LOCATION: L1013/Windmill Road DAY: Wednesday

		N	MOVE	MENT	4					N	MOVEN	/ENT	5					N	MOVEN	JENT	6			
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
07:30	0	0	16	2	1	1	20	22	0	0	4	1	0	1	6	7	0	0	1	0	0	0	1	1
07:45	0	0	15	4	0	0	19	19	0	0	13	4	1	0	18	19	0	0	1	0	1	0	2	3
08:00	0	0	19	5	3	0	1	30	0	0	14	2	3	0	19	22	0	0	3	3	3	0	9	12
08:15	0	0	7	0	1	0	8	9	0	0	14	3	1	0	18	19	0	0	2	0	1	0	3	4
н/тот	0	0	57	11	5	1	74	80	0	0	45	10	5	1	61	67	0	0	7	3	5	0	15	20
08:30	0	0	10	2	2	0	14	16	0	0	7	3	0	0	10	10	0	0	1	3	0	0	4	4
08:45	0	0	11	2	0	1	14	15	0	0	9	2	2	1	14	17	0	0	0	0	0	0	0	0
09:00	0	0	14	1	3	2	20	25	0	0	20	1	1	0	22	23	0	0	1	1	0	0	2	2
09:15	0	0	13	2	1	0	16	17	0	0	7	6	9 ₁	0	14	15	0	0	4	2	2	0	8	10
н/тот	0	0	48	7	6	3	64	73	0	0	43	12	4	1	60	65	0	0	6	6	2	0	14	16
P/TOT	0	0	105	18	11	4	138	153	0	0	88	22	9	2	121	132	0	0	13	9	7	0	29	36

MOVEMENT 4 MOVEMENT 5 MOVEMENT 6 TIME PCL MCL CAR LGV HGV BUS TOT PCU MCL CAR LGV HGV TOT **PCU** PCL MCL CAR LGV HGV TOT **PCU** 16:00 16:15 16:30 16:45 H/TOT 17:00 17:15 17:30 17:45 H/TOT 18:00 18:15 H/TOT P/TOT

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED ENTRY/EXIT COUNTS

NOVEMBER 2020 TRA/20/123

SITE: ATC Location DATE:

LOCATION: Dawn Meats Access DAY: Wednesday

vn Meats	s Acc	ess												DAY	:		Wednesday
			EN.	TRY							EX	(IT					Wednesday
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	13
00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
00:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
00:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-5
00:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	600
н/тот	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
01:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
01:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
01:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
01:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
н/тот	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
02:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
02:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
02:30	0	0	1	0	0	0	1	1	0	0	0	0	0	0	0	0	
02:45	0	0	2	0	0	0	2	2	0	0	0	0	0	0	0	0	
Н/ТОТ	0	0	3	0	0	0	3	3	0	0	0	0	0	0	0	0	
03:00	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1	
03:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
03:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
03:45	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1	
Н/ТОТ	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	2	
04:00	0	0	1	0	0	0	1	1	0	0	0	0	0	0	0	0	
04:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
04:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
04:45	0	0	2	0	0	0	2	2	0	0	0	0	0	0	0	0	
H/TOT	0	0	3	0	0	0	3	3	0	0	0	0	0	0	0	0	
05:00 05:15	0	0		0	0	0	1	1	0	0	0	0	0	0	0	0	
05:15	0					0	3	3	0						0	0	
05:45	0	0	3 7	0	0	0	7	7	0	0	0 1	0	0	0	1	1	
H/TOT	0	0	15	0	0	0	15	15	0	0	1	0	0	0	1	1	
06:00	0	0	4	0	1	0	5	6	0	0	0	0	0	0	0	0	
06:15	0	0	3	0	0	0	3	3	0	0	0	0	1	0	1	2	
06:30	0	0	2	0	0	0	2	2	0	0	0	0	0	0	0	0	
06:45	0	0	4	0	0	0	4	4	0	0	0	0	0	0	0	0	
Н/ТОТ	0	0	13	0	1	0	14	15	0	0	0	0	1	0	1	2	
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:15	0	0	1	2	1	0	4	5	0	0	0	0	0	0	0	0	
07:30	0	0	1	1	0	0	2	2	0	0	0	2	1	0	3	4	
07:45	0	0	1	1	0	0	2	2	0	0	0	1	0	0	1	1	
н/тот	0	0	3	4	1	0	8	9	0	0	0	3	1	0	4	5	
,		-		-				,		-		,	•		-	,	J

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED ENTRY/EXIT COUNTS

NOVEMBER 2020 TRA/20/123

SITE: ATC Location DATE:

LOCATION: Dawn Meats Access DAY: Wednesday

n Meats	Acc	ess												DAY	:	
			EN ⁻	TRY							EX	ΊΤ				
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
08:00	0	0	2	1	1	0	4	5	0	0	0	1	0	0	1	1
08:15	0	0	2	0	1	0	3	4	0	0	1	0	1	0	2	3
08:30	0	0	5	0	1	0	6	7	0	0	0	0	0	0	0	0
08:45	0	0	0	1	0	0	1	1	0	0	3	0	2	0	5	7
н/тот	0	0	9	2	3	0	14	17	0	0	4	1	3	0	8	11
09:00	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
09:15	0	0	1	1	0	0	2	2	0	0	1	0	0	0	1	1
09:30	0	0	2	1	2	0	5	7	0	0	1	2	0	0	3	3
09:45	0	0	2	1	3	0	6	9	0	0	1	1	2	0	4	6
н/тот	0	0	5	3	5	0	13	18	0	0	3	5 🔷	2	0	10	12
10:00	0	0	3	0	1	0	4	5	0	0	4	0	2	0	6	8
10:15	0	0	2	0	0	0	2	2	0	0	2	0	1	0	3	4
10:30	0	0	2	1	2	0	5	7	0	0	0	0	1	0	1	2
10:45	0	0	0	1	0	0	1	1	0	0	0	2	0	0	2	2
н/тот	0	0	7	2	3	0	12	15	0	0	6	2	4	0	12	16
11:00	0	0	1	0	2	0	3	5	0	0	0	0	1	0	1	2
11:15	0	0	1	0	0	0	1	1	0	0	0	0	2	0	2	4
11:30	0	0	1	0	2	0	3	5	0	0	0	0	2	0	2	4
11:45	0	0	0	1	2	0	3	5	0	0	0	0	0	0	0	0
н/тот	0	0	3	1	6	0	10	16	0	0	0	0	5	0	5	10
12:00	0	0	2	0	10	0	3	4	0	0	2	1	2	0	5	7
12:15	0	0	1	1	1	0	3	4	0	0	0	0	1	0	1	2
12:30	0	0	0	0	0	0	0	0	0	0	2	0	1	0	3	4
12:45	0	0	3	0	1	0	4	5	0	0	2	0	1	0	3	4
Н/ТОТ	0	0	6	1	3	0	10	13	0	0	6	1	5	0	12	17
13:00	0	0	1	0	2	0	3	5	0	0	1	0	0	0	1	1
13:15	0	0	2	1	0	0	3	3	0	0	7	3	1	0	11	12
13:30	0	0	1	0	0	0	1	1	0	0	1	0	1	0	2	3
13:45	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
н/тот	0	0	4	1	2	0	7	9	0	0	10	3	2	0	15	17
14:00	0	0	1	1	2	0	4	6	0	0	1	0	1	0	2	3
14:15	0	0	0	0	1	0	1	2	0	0	2	1	0	0	3	3
14:30	0	0	1	0	1	0	2	3	0	0	4	0	1	0	5	6
14:45	0	0	0	1	0	0	1	1	0	0	5	0	0	0	5	5
н/тот	0	0	2	2	4	0	8	12	0	0	12	1	2	0	15	17
15:00	0	0	1	0	2	0	3	5	0	0	7	0	3	0	10	13
15:15	0	0	2	0	0	0	2	2	0	0	4	0	0	0	4	4
15:30	0	0	0	0	2	0	2	4	0	0	5	0	1	0	6	7
15:45	0	0	0	1	1	0	2	3	0	0	3	0	0	0	3	3
H/TOT	0	0	3	1	5	0	9	14	0	0	19	0	4	0	23	27

DAWN MEATS, SLANE TRAFFIC COUNTS MANUAL CLASSIFIED ENTRY/EXIT COUNTS

NOVEMBER 2020 TRA/20/123

SIIE: AIC L	ATC	ITE:	: ATC		DATE
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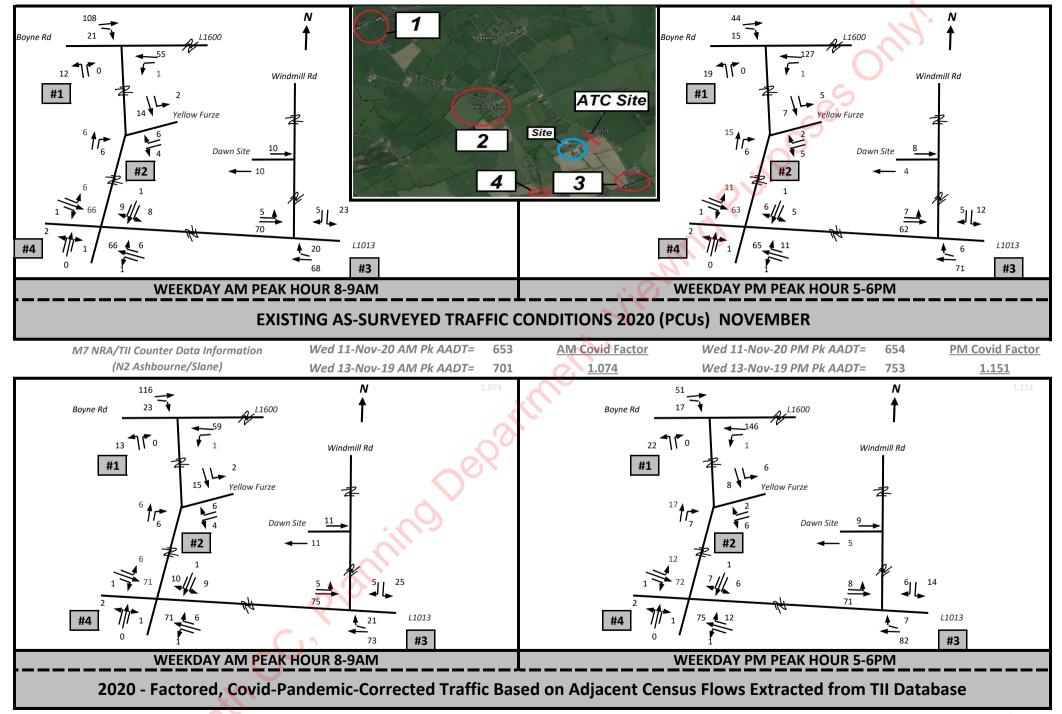
LOCATION: Dawn Meats Access DAY: Wednesday

n Meats	Acc	ess												DAY	:	
	ENTRY					EXIT										
TIME	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU	PCL	MCL	CAR	LGV	HGV	BUS	тот	PCU
16:00	0	0	1	0	0	0	1	1	0	0	1	0	1	0	2	3
16:15	0	0	0	0	0	0	0	0	0	0	1	1	3	0	5	8
16:30	0	0	0	0	0	0	0	0	0	0	5	0	0	0	5	5
16:45	0	0	2	0	1	0	3	4	0	0	2	0	0	0	2	2
H/TOT	0	0	3	0	1	0	4	5	0	0	9	1	4	0	14	18
17:00	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	3
17:15	0	0	2	0	1	0	3	4	0	0	0	0	1	0	1	2
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	3
Н/ТОТ	0	0	2	0	1	0	3	4	0	0	6	0	1	0	7	8
18:00	0	0	0	0	1	0	1	2	0	0	0	0	0	0	0	0
18:15	0	0	0	2	1	0	3	4	0	0	0	0	2	0	2	4
18:30	0	0	1	0	2	0	3	5	0	0	1	0	2	0	3	5
18:45	0	0	0	0	2	0	2	4	0	0	0	0	1	0	1	2
Н/ТОТ	0	0	1	2	6	0	9	15	0	0	1	0	5	0	6	11
19:00	0	0	0	0	1	0	1	2	0	0	1	0	2	0	3	5
19:15	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	4
19:30	0	0	0	0	1	0	1	2	0	0	0	0	0	0	0	0
19:45	0	0	0	0	1	0	1	2	0	0	0	0	0	0	0	0
н/тот	0	0	0	0	3	0	3	6	0	0	1	0	4	0	5	9
20:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
20:15	0	0	0	1	0	0	1	1	0	0	0	0	1	0	1	2
20:30	0	0	0	0	1	0	1	2	0	0	0	1	1	0	2	3
20:45	0	0	0	0	1	0	1	2	0	0	0	1	0	0	1	1
Н/ТОТ	0	0	0	1	2	0	3	5	0	0	0	2	2	0	4	6
21:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
21:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
21:30	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
21:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
н/тот	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
22:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
22:15	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
22:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
22:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
н/тот	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
23:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
23:15	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	2
23:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
23:45	0	0	0	0	1	0	1	2	0	0	0	0	0	0	0	0
н/тот	0	0	0	0	1	0	1	2	0	0	2	0	0	0	2	2

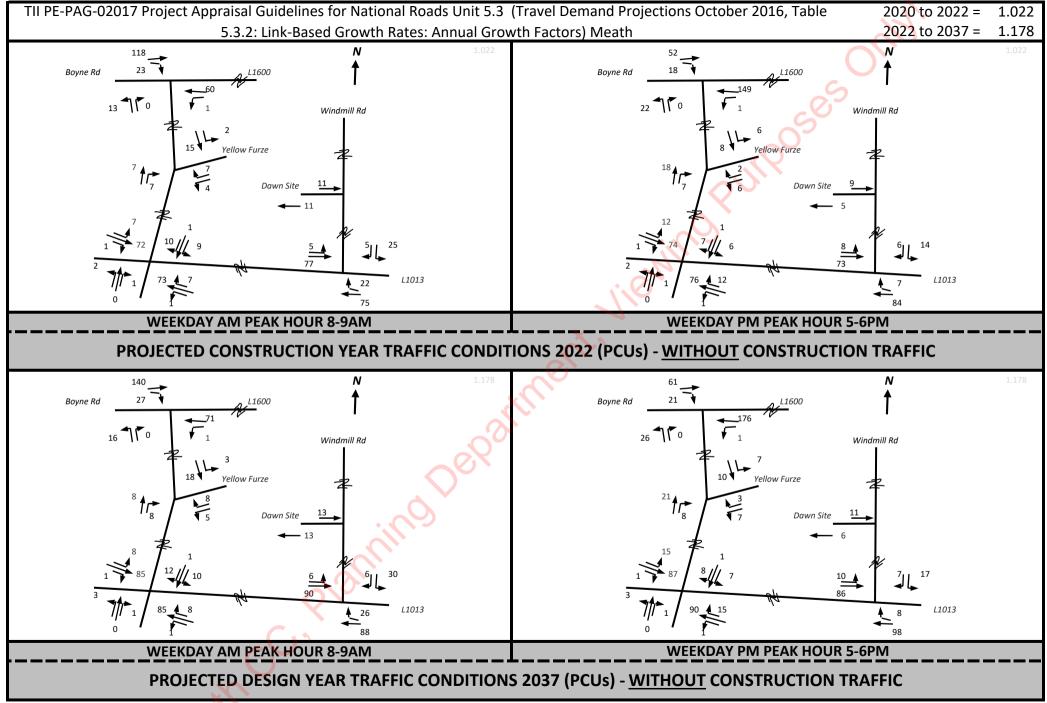


APPENDIX C

Traffic Surveys, Trip Distribution & Network Traffic Flow Diagrams



Page 1 of 8



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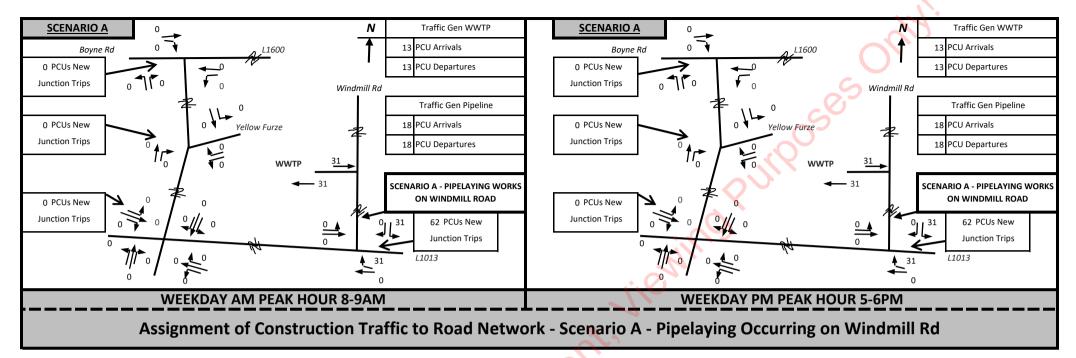
Construction-Activity#	Movements-Generated¤			
	LGV¤	HGV¤	p	
Extension-to-WWTP-(Phase-1)¤	40¤	30¤	n	
Rising-Main-Pipeline [#]	60¤	40¤	D.	

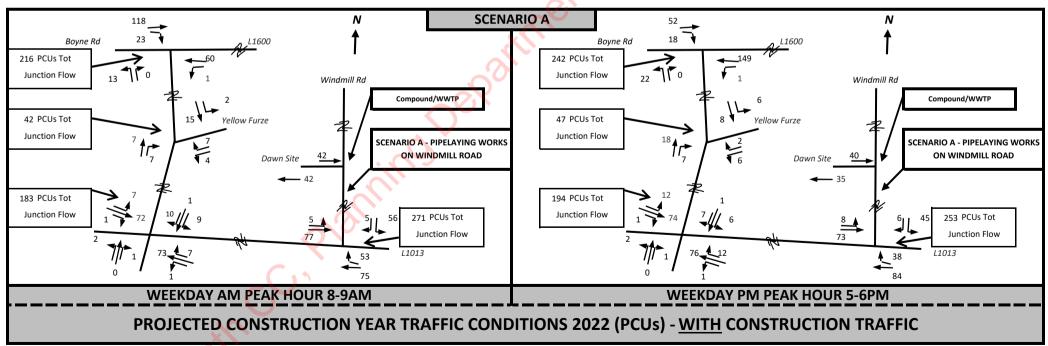
Converting to PCUS & Commuter Peak Flows.....

		<u>-</u>					
WWTP EXTENSION							
Network Period	Arrivals	Departures (PCUs)					
Network Period	(PCUs)						
24 Hr AADT	100	100					
Weekday AM Peak Hr*	13	13					
Weekday PM Peak Hr*	13	13					

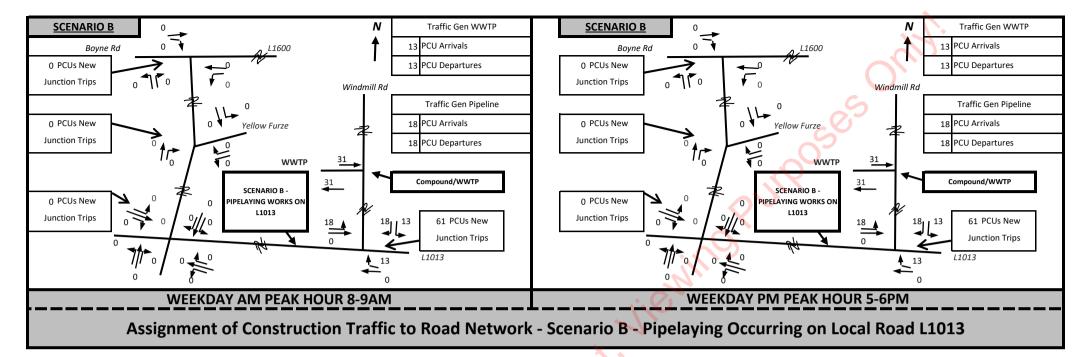
RISING MAIN PIPELINE						
Network Period	Arrivals	Departures				
Network Period	(PCUs)	(PCUs)				
24 Hr AADT	140	140				
Weekday AM Peak Hr*	18	18				
Weekday PM Peak Hr*	18	18				

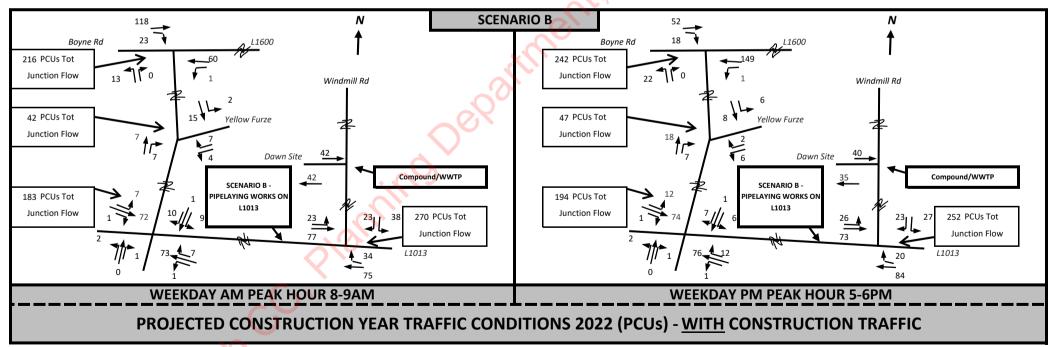
^{*} Industry Standard Method of Calculating Weekday Peak Hr is to Divide 24Hr AADT by 10 - We have Divided by 8 for Robustness, giving higher Peak Hr Flow Volumes



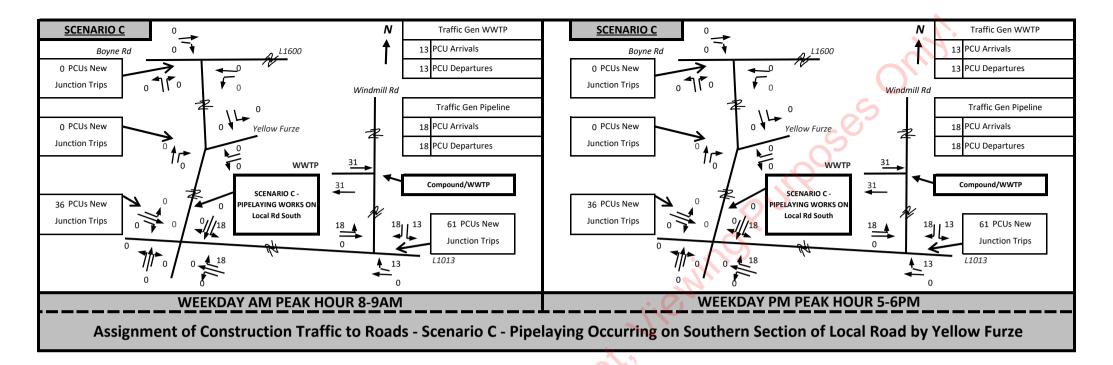


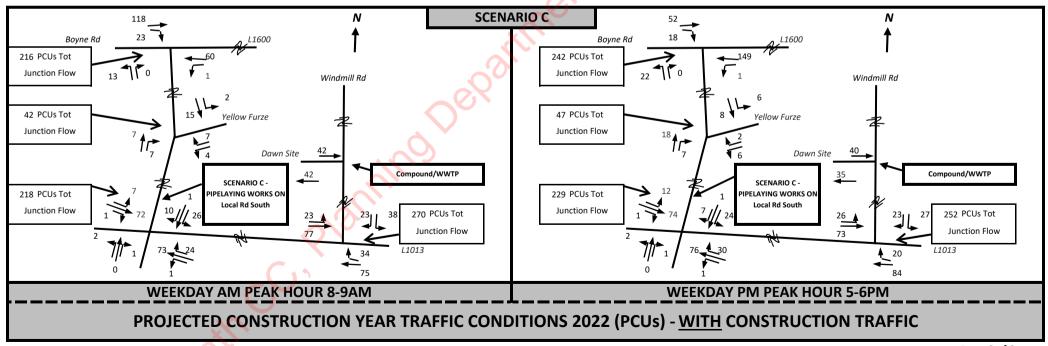
Page 4 of 8



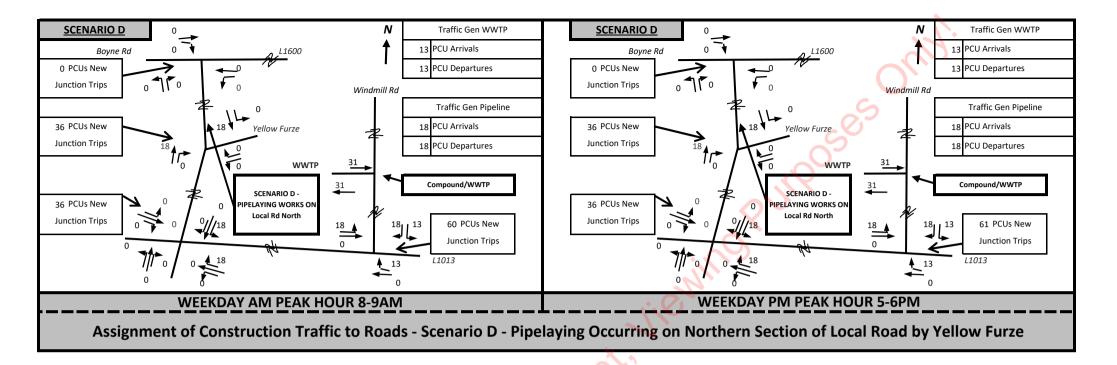


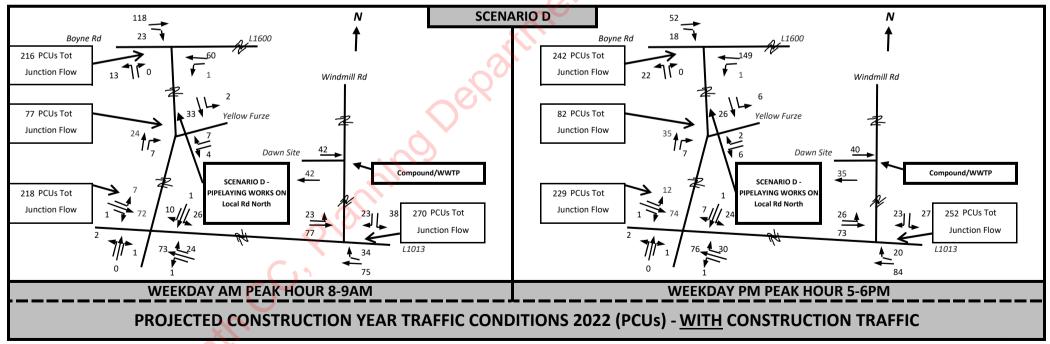
Page 5 of 8



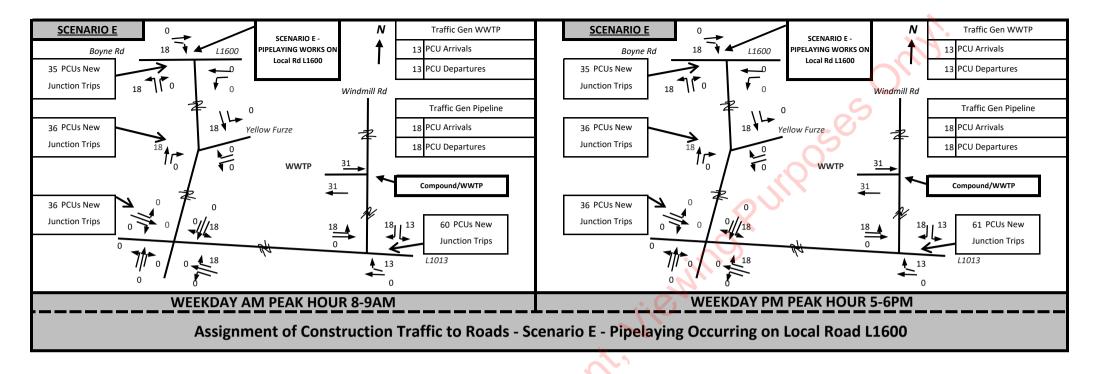


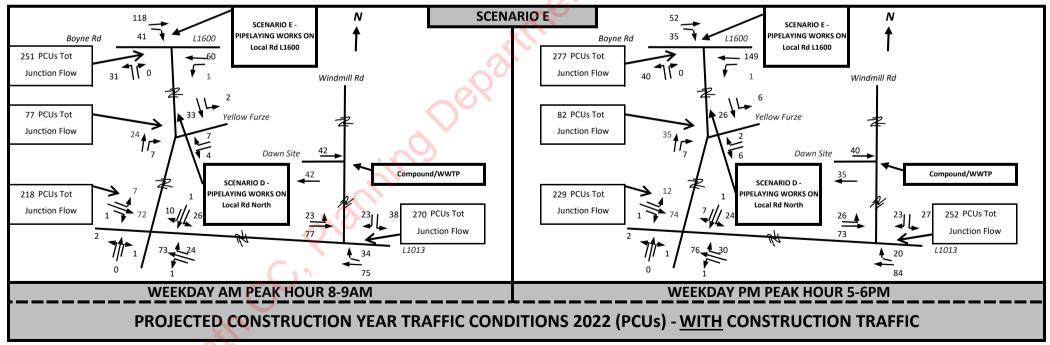
Page 6 of 8





Page 7 of 8





Page 8 of 8



APPENDIX D

,0ses Only

PiCADY Junction Capacity Model Output Existing Windmill Rd/L1013

Established T Junction Leading to Dawn Plant
Summary PICADY Results in Order as included herein
(Robust & Worst Case – Being "Scenario E")
SELECTED CONSTRUCTION YEAR 2022

Modelled	Period Mean Max Q	Period Max
Scenario	(PCUs)	RFC
2022 Construction Year AM Peak	<1	0.11
2022 Construction Year PM Peak	<1	0.10

All Results Above are way below the recommended RFC of 0.85 (85% Capacity) and therefore no problems whatsoever are anticipated at the Junction in terms of Capacity or excessive vehicle Queues

NB Any Small Changes to Selected Construction Year 2022 or indeed significantly higher traffic volumes experienced, will clearly have no significant implications in terms of the conclusions of the Study in Light of the Favourable Results.



Junctions 9

PICADY 9 - Priority Intersection Module

Version: 9.0.1.4646 [] © Copyright TRL Limited, 2020

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Filename: 2022 AM PM Worst Case.j9

Path: C:\Users\Eoin\NRB Consulting Engineers Ltd\NRB Server - Documents\2020\20-070 Dawn Slane\Calculations\Windmill

L1013 Picadys

Report generation date: 13/12/2020 12:32:10

»2022, AM

»2022, PM

Summary of junction performance

	AM			PM				
	Q (PCU)	Delay (s)	RFC	LOS	Q (PCU)	Delay (s)	RFC	LOS
	2022							
Stream B-AC	0.1	7.45	0.11	Α	0.1	7.42	0.10	Α
Stream C-AB	0.1	6.76	0.06	А	0.0	6.60	0.04	Α

Values shown are the highest values encountered over all time segments. Delay is the maximum value of Av. delay per arriving vehicle.

File summary

File Description

Title	(untitled)
Location	
Site number	
Date	09/12/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	NRB-004\Eoin
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Av. delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perHour	S	-Min	perMin

Analysis Options

Calculate Q Percentiles	Calculate residual capacity	RFC Threshold	Av. Delay threshold (s)	Q threshold (PCU)
		0.85	36.00	20.00



ID	mand Set Su	mmary				
	Scenario name	Time Period name		Start time (HH:mm)		Time segment length (min)
D1 D2		AM PM	ONE HOUR	07:45 16:45	09:15 18:15	15 15
_	Network flow so	aling factor (%)				
						aviing Purposes
				epartin		
			Rillia			
		Sha				
(S)		S, Sia				
(Q)		Sia				

ID	Network flow scaling factor (%)
A1	100.000



2022, AM

Data Errors and Warnings

Junction Network

Junctions

	Errors and War	rnings					
Jur	nction Netv	vork					ON
Junc	ctions	. 1	Irmatian Trus	Maiar road direction	lunction Delay (c)	Junction LOS	Ses
Junc 1			Junction Type T-Junction	Major road direction Two-way	Junction Delay (s)	A A	-0,5
Arr	ns					•	0
Arms	S						
	Name	Description	on Arm type				
Arm		•					
Arm A	L1013 West		Major				
		·	Major Minor			110	

Junction Network Options

Driving side	Lighting	
Left	Normal/unknown	

Arms

Arms

Arm	Name	Description	Arm type
Α	L1013 West		Major
В	Windmill Rd to Dawn		Minor
С	L1013 to N2		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
С	7.00			100.0	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

	Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
Γ	В	One lane	3.00	90	90

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	552	0.096	0.243	0.153	0.347
1	B-C	681	0.100	0.252	-	-
1	С-В	632	0.234	0.234	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D1	2022	AM	ONE HOUR	07:45	09:15	15



Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Av. Demand (PCU/hr)	Scaling Factor (%)	
Α		✓	100	100.000	
В		✓	61	100.000	
С		✓	109	100.000	

Origin-Destination Data

Demand (PCU/hr)

		То					
From		Α	В	С			
	Α	0	23	77			
	В	23	0	38			
	С	75	34	0			

Vehicle Mix

HV %s

		То					
From		Α	В	ပ			
	Α	0	8	1			
	В	8	0	8			
	C	1	8	0			

Results

Results Summary for whole modelled period

Stream	Max RFC	Max delay (s)	Max Q (PCU)	Max LOS
B-AC	0.11	7.45	0.1	A
C-AB	0.06	6.76	0.1	A
C-A				
A-B				
A-C			(9)	

Main Results for each time segment

07:45 - 08:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	46	601	0.076	46	0.1	6.998	А
C-AB	26	617	0.042	26	0.0	6.567	А
C-A	56			56			
A-B	17			17			
A-C	58			58			



08:00 - 08:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	55	596	0.092	55	0.1	7.185	А
C-AB	31	615	0.050	31	0.1	6.650	A
C-A	67			67			
A-B	21			21			
A-C	69			69			

08:15 - 08:30

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	67	589	0.114	67	0.1	7.445	A
C-AB	38	612	0.062	38	0.1	6.761	A
C-A	82			82			
A-B	25			25			
A-C	85			85			<i>3</i> O <i>1</i>

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	67	589	0.114	67	0.1	7.448	А
C-AB	38	612	0.062	38	0.1	6.761	Α
C-A	82			82		1	
A-B	25			25	. 0		
A-C	85			85	116		

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	55	596	0.092	55	0.1	7.191	A
C-AB	31	615	0.050	31	0.1	6.651	A
C-A	67			67			
A-B	21			21			
A-C	69			69			

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	Los
B-AC	46	601	0.076	46	0.1	7.008	А
C-AB	26	617	0.042	26	0.0	6.571	А
C-A	56			56			
A-B	17			17			
A-C	58			58			
	رن،	ζ`					
o di	, C _C ,	ζ`					



2022, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Juncti	n Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	Windmill Rd/L1013 T Junct	T-Junction	Two-way	1.99	Α

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ı	D	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mn	n)	Time segment length (min)
E)2	2022	PM	ONE HOUR	16:45	18:15		15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Av. Demand (PCU/hr)	Scaling Factor (%)
Α		✓	99	100.000
В		✓	50	100.000
С		✓	104	100.000

Origin-Destination Data

Demand (PCU/hr)

	То				
		Α	В	၁	
	Α	0	26	73	
From	В	23	0	27	
	U	84	20	0	

Vehicle Mix

HV %s

		То					
X		Α	В	С			
	Α	0	8	1			
From	В	8	0	8			
	U	1	8	0			



Results

Results Summary for whole modelled period

Stream	Max RFC	Max delay (s)	Max Q (PCU)	Max LOS
B-AC	0.10	7.42	0.1	A
C-AB	0.04	6.60	0.0	А
C-A				
A-B				
A-C				

Main Results for each time segment

16:45 - 17:00

0.10	7.42	0.1	A				
0.04	6.60	0.0	A				
							19,
							C
Results for eac	h time segmen	nt				205	
Results for eac	h time segmen	ıt				400	
Results for eac	h time segmen	ıt .				111905	
	Capacity	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	Los	
17:00 Total Demand	Capacity			End queue (PCU)	Delay (s) 7.026	Jibo,	
Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	(PCU/hr)			LOS	
Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC 0.064	(PCU/hr) 37	0.1	7.026	Los A	
Total Demand (PCU/hr) 38	Capacity (PCU/hr)	RFC 0.064	(PCU/hr) 37 15	0.1	7.026	Los A	

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	45	586	0.077	45	0.1	7.188	А
C-AB	18	614	0.029	18	0.0	6.522	А
C-A	75			75			
A-B	23			23			
A-C	66			66			

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	55	579	0.095	55	0.1	7.418	A
C-AB	22	611	0.036	22	0.0	6.602	A
C-A	92			92			
A-B	29			29			
A-C	80			80			

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	55	579	0.095	55	0.1	7.418	A
C-AB	22	611	0.036	22	0.0	6.602	А
C-A	92			92			
A-B	29			29			
A-C	80			80			



17:45 - 18:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	45	586	0.077	45	0.1	7.194	А
C-AB	18	614	0.029	18	0.0	6.522	Α
C-A	75			75			
A-B	23			23			
A-C	66			66			

18:00 - 18:15

	(PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC 38	590	0.064	38	0.1	7.036	Α
C-AB 15	616	0.025	15	0.0	6.463	Α
C-A 63			63			
A-B 20 A-C 55			20 55			
ain	Planni	no Dei	Sarting	snt. Vie	nin ^o	



APPENDIX E

PiCADY Junction Capacity Model Output Existing L1013 Crossroads (Leading to Yellow Furze)

Established L1013 Crossroads Junction Leading to Yellow Furze Summary PICADY Results in Order as included herein (Robust & Worst Case – Being "Scenario E") SELECTED CONSTRUCTION YEAR 2022

Modelled	Period Mean Max Q	Period Max
Scenario	(PCUs)	RFC
2022 Construction Year AM Peak	<1	0.07
2022 Construction Year PM Peak	<1	0.08

All Results Above are way below the recommended RFC of 0.85 (85% Capacity) and therefore no problems whatsoever are anticipated at the Junction in terms of Capacity or excessive vehicle Queues

NB Any Small Changes to Selected Construction Year 2022 or indeed significantly higher traffic volumes experienced, will clearly have no significant implications in terms of the conclusions of the Study in Light of the Favourable Results



Junctions 9

PICADY 9 - Priority Intersection Module

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Filename: 2022 AM PM Worst Case.j9

Path: C:\Users\Eoin\NRB Consulting Engineers Ltd\NRB Server - Documents\2020\20-070 Dawn Slane\Calculations\L1013

Junc Leading YF Picadys

Report generation date: 13/12/2020 12:35:02

»2022, AM

»2022, PM

Summary of junction performance

	AM				PM			
	Q (PCU)	Delay (s)	RFC	LOS	Q (PCU)	Delay (s)	RFC	LOS
		20			022			
Stream B-ACD	0.0	0.00	0.00	Α	0.0	0.00	0.00	Α
Stream A-BCD	0.0	6.61	0.04	Α	0.1	6.84	0.08	Α
Stream D-ABC	0.1	6.74	0.07	Α	0.1	6.66	0.06	Α
Stream C-ABD	0.0	6.04	0.00	Α	0.0	6.13	0.00	A

Values shown are the highest values encountered over all time segments. Delay is the maximum value of Av. delay per arriving vehicle.

File summary

File Description

	/ n
Title	(untitled)
Location	
Site number	
Date	09/12/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	NRB-004\Eoin
Description	
	A

Units

	Dista	nce units	Speed units	Traffic units input	Traffic units results	Flow units	Av. delay units	Total delay units	Rate of delay units
1		m	kph	PCU	PCU	perHour	s	-Min	perMin

Analysis Options

Calculate Q Percentiles	Calculate residual capacity	RFC Threshold	Av. Delay threshold (s)	Q threshold (PCU)
		0.85	36.00	20.00



ID	mand Set Su	ımmary				
-	Scenario name	Time Period name		Start time (HH:mm)		Time segment length (min)
D1 D2		AM PM	ONE HOUR	07:45 16:45	09:15 18:15	15 15
ID	Alysis Set De	aling factor (%)				
A1	100.					awino Purposes
		Plan	ning	epartil		
	C					

ID	Network flow scaling factor (%)
A1	100.000



2022, AM

Data Errors and Warnings

Junction Network

Junctions

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	L1013 Crossroads - Leading to Yellow Furze	Crossroads	Two-way	1.93	А

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description	Arm type
Α	L1013 to Dawn and N2		Major
В	Local Road S		Minor
С	L1013 West Arm		Major
D	Local Road to Yellow Furze		Minor

Major Arm Geometry

	·, / · · · · ·							
	Errors and Warning rors or warnings	gs						
Jur	nction Networ	k						OUIA
	tions			1	,		on LOS	
Junc			Junction Type	 	7		on LOS	0,3
1	L1013 Crossroads - Le	eading to Yellow Furze	Crossroads	Two-way	1.93	А		.0
Arn Arms					lien	ille	>	
Arm	Name	Description Arm typ	ре		. 0			
Α	L1013 to Dawn and N2	Major			1,10			
В	Local Road S	Minor			7			
С	L1013 West Arm	Major			•			
D	Local Road to Yellow Furze	Minor		Ċ				
Majo	or Arm Geometry			(S)				
Arm	Width of carriageway (m)	Has kerbed central re	serve Has rig	ght turn bay Visibilit	y for right turn (m)	Blocks?	Blocking queue (PCU)	
Α	6.00				100.0	✓	1.00	
c	6.00				100.0	✓	1.00	

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

	Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
ĺ	В	One lane	3.00	90	90
ĺ	D	One lane	3.00	90	90

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for AD	Slope for B-A	Slope for B-C	Slope for B-D	Slope for C-A	Slope for C-B	Slope for C-D	Slope for D-A	Slope for D-B	Slope for D-C
1	A-D	632	-		-	-	-	-	0.245	0.350	0.245		-	-
1	B-A	552	0.101	0.254	0.254	-	-	-	0.160	0.363	-	0.254	0.254	0.127
1	B-C	681	0.104	0.264	-	-	-	-	-	-	-	-	-	-
1	B-D, nearside lane	552	0.101	0.254	0.254	-	-	-	0.160	0.363	0.160	-	-	-
1	B-D, offside lane	552	0.101	0.254	0.254	-	-	-	0.160	0.363	0.160	-	-	-
10	С-В	632	0.245	0.245	0.350	-	-	-	-	-	-	-	-	-
1	D-A	681	-	-	-	-	-	-	0.264	-	0.104	-	-	-
1	D-B, nearside lane	552	0.160	0.160	0.363	-	-	-	0.254	0.254	0.101	-	-	-
1	D-B, offside lane	552	0.160	0.160	0.363	-	-	-	0.254	0.254	0.101	-	-	-
1	D-C	552	-	0.160	0.363	0.127	0.254	0.254	0.254	0.254	0.101	-	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.



Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

	ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
П	D1	2022	AM	ONE HOUR	07:45	09:15	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Av. Demand (PCU/hr)	Scaling Factor (%)
Α		✓	98	100.000
В		✓	3	100.000
С		✓	80	100.000
D		✓	37	100.000

Origin-Destination Data

Demand (PCU/hr)

		То						
		Α	В	С	D			
	Α	0	1	73	24			
From	В	1	0	2	0			
	С	72	1	0	7			
	D	26	1	10	0			

Vehicle Mix

HV %s

		То						
		Α	В	С	D			
	Α	0	8	8	8			
From	В	8	0	1	1			
	С	8	1	0	1			
İ	D	8	1	1	0			

Results

Results Summary for whole modelled period

Stream	Max RFC	Max delay (s)	Max Q (PCU)	Max LOS
B-ACD	0.00	0.00	0.0	A
A-BCD	0.04	6.61	0.0	A
A-B				
A-C				
D-ABC	0.07	6.74	0.1	A
C-ABD	0.00	6.04	0.0	A
C-D				
C-A				



Main Results for each time segment

07:45 - 08:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS	
B-ACD	0	552	0.000	0	0.0	0.000	А	
A-BCD	18	619	0.029	18	0.0	6.465	А	
A-B	0.75			0.75				
A-C	55			55				
D-ABC	28	615	0.045	28	0.0	6.479	А	
C-ABD	0.75	612	0.001	0.75	0.0	5.947	А	Ca
C-D	5			5				2
C-A	54			54			C	
8:00 - 0	8:15						,00)
Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	Los	
	0	5.40	0.000	_	0.0	0.000		i

08:00 - 08:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	Los
B-ACD	0	546	0.000	0	0.0	0.000	A
A-BCD	22	617	0.035	22	0.0	6.528	А
A-B	0.90			0.90			
A-C	66			66		. ~~	
D-ABC	33	611	0.054	33	0.1	6.589	А
C-ABD	0.90	608	0.001	0.90	0.0	5.987	A
C-D	6			6			
C-A	65			65			

08:15 - 08:30

			A A				
Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-ACD	0	539	0.000	0	0.0	0.000	А
A-BCD	27	615	0.043	27	0.0	6.611	A
A-B	1			1			
A-C	80			80			
D-ABC	41	606	0.067	41	0.1	6.743	A
C-ABD	1	603	0.002	1	0.0	6.042	А
C-D	8			8			
C-A	79			79			

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-ACD	0	539	0.000	0	0.0	0.000	Α
A-BCD	27	615	0.043	27	0.0	6.611	А
A-B	1			1			
A-C	80			80			
D-ABC	41	606	0.067	41	0.1	6.743	А
C-ABD	(1)	603	0.002	1	0.0	6.042	А
C-D	8			8			
C-A	79			79			



08:45 - 09:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-ACD	0	546	0.000	0	0.0	0.000	A
A-BCD	22	617	0.035	22	0.0	6.529	A
A-B	0.90			0.90			
A-C	66			66			
D-ABC	33	611	0.054	33	0.1	6.590	A
C-ABD	0.90	608	0.001	0.90	0.0	5.987	А
C-D	6			6			
C-A	65			65			

09:00 - 09:15

B-ACD A-BCD	(PCU/hr)	(PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
ABCD	0	552	0.000	0	0.0	0.000	А
ABCD	18	619	0.029	18	0.0	6.468	А
A-B	0.75			0.75			
A-C	55			55			0
D-ABC	28	615	0.045	28	0.1	6.483	А
C-ABD	0.75	612	0.001	0.75	0.0	5.950	A
C-D C-A	5 54			5 54			
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		Slaw					



2022, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	L1013 Crossroads - Leading to Yellow Furze	Crossroads	Two-way	2.16	А

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D2	2022	PM	ONE HOUR	16:45	18:15	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Av. Demand (PCU/hr)	Scaling Factor (%)
Α		✓	120	100.000
В		✓	3	100.000
С		✓	87	100.000
D		✓	32	100.000

Origin-Destination Data

Demand (PCU/hr)

	То					
		Α	В	U	D	
	Α	0	1	76	43	
From	В	1	0	2	0	
	С	74	1	0	12	
	D	24	1	7	0	

Vehicle Mix

HV %s

	То					
		Α	В	С	D	
	Α	0	8	8	8	
From	В	8	0	1	1	
	С	8	1	0	1	
	D	8	1	1	0	



Results

Results Summary for whole modelled period

Stream	Max RFC	Max delay (s)	Max Q (PCU)	Max LOS
B-ACD	0.00	0.00	0.0	А
A-BCD	0.08	6.84	0.1	A
A-B				
A-C				
D-ABC	0.06	6.66	0.1	A
C-ABD	0.00	6.13	0.0	А
C-D				
C-A				

Main Results for each time segment

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-ACD	0	548	0.000	0	0.0	0.000	А
A-BCD	33	620	0.053	32	0.1	6.617	Α
A-B	0.75			0.75			
A-C	57			57			
D-ABC	24	620	0.039	24	0.0	6.407	Α
C-ABD	0.75	606	0.001	0.75	0.0	6.002	A
C-D	9			9			
C-A	56			56			

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-ACD	0	542	0.000	0	0.0	0.000	A
A-BCD	39	618	0.063	39	0.1	6.714	A
A-B	0.89			0.89			
A-C	68			68			
D-ABC	29	616	0.047	29	0.1	6.511	A
C-ABD	0.90	601	0.002	0.90	0.0	6.053	А
C-D	11			11			
C-A	67			67			

17:15 - 17:30

			1	1	1		
Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-ACD	0	533	0.000	0	0.0	0.000	А
A-BCD	48	616	0.078	48	0.1	6.840	А
A-B	1			1			
A-C	83			83			
D-ABC	35	609	0.058	35	0.1	6.656	А
C-ABD	1	595	0.002	1	0.0	6.125	А
C-D	13			13			
C-A	81			81			



17:30 - 17:45

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-ACD	0	533	0.000	0	0.0	0.000	A
A-BCD	48	616	0.078	48	0.1	6.843	А
A-B	1			1			
A-C	83			83			
D-ABC	35	609	0.058	35	0.1	6.656	А
C-ABD	1	595	0.002	1	0.0	6.125	А
C-D	13			13			
C-A	81			81			

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-ACD	0	542	0.000	0	0.0	0.000	A
A-BCD	39	618	0.063	39	0.1	6.716	Α
A-B	0.89			0.89			
A-C	68			68			9
D-ABC	29	616	0.047	29	0.1	6.515	А
C-ABD	0.90	601	0.002	0.90	0.0	6.056	А
C-D	11			11		. ~	
C-A	67			67			

18:00 - 18:15

Stream (PCU/hr) (PCU/hr) RFC (PCU/hr) End queue (PCU) Delay (s) Li		Total Demand	Capacity	556	Throughput	E . I (DO!)	5.1. (.)	, -
ABCD 33 620 0.053 33 0.1 6.626 AB 0.75 0.75 AC 57 57 D-ABC 24 620 0.039 24 0.0 6.414	Stream			RFC		End queue (PCU)	Delay (s)	LO
AB 0.75 AC 57 D-ABC 24 620 0.039 24 0.0 6.414	B-ACD	0	548	0.000	0	0.0	0.000	А
AC 57 57 57 0.0 6.414 0.0 6.414	A-BCD	33	620	0.053	33	0.1	6.626	Α
D-ABC 24 620 0.039 24 0.0 6.414	A-B	0.75			0.75			
	A-C	57			57			
C-ABD 0.75 606 0.001 0.75 0.0 6.003 C-D 9 9	D-ABC	24	620	0.039	24	0.0	6.414	Д
C-D 9 9 56 56 56 56 56 56 56 56 56 56 56 56 56	C-ABD	0.75	606	0.001	0.75	0.0	6.003	Д
C-A 56 56 56	C-D	9			9			
Ath Co, blawing Dek	C-A	56			56			
			an'i	NO				



APPENDIX F

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PiCADY Junction Capacity Model Output Existing T Junction at 'Yellow Furze'

Established T Junction at 'Yellow Furze' Summary PICADY Results in Order as included herein (Robust & Worst Case – Being "Scenario E") SELECTED CONSTRUCTION YEAR 2022

Modelled	Period Mean Max Q	Period Max
Scenario	(PCUs)	RFC
2022 Construction Year AM Peak	<1	0.02
2022 Construction Year PM Peak	<1	0.01

All Results Above are way below the recommended RFC of 0.85 (85% Capacity) and therefore no problems whatsoever are anticipated at the Junction in terms of Capacity or excessive vehicle Queues

NB Any Small Changes to Selected Construction Year 2022 or indeed significantly higher traffic volumes experienced, will clearly have no significant implications in terms of the conclusions of the Study in Light of the Favourable Results.

Meath Co.



Junctions 9

PICADY 9 - Priority Intersection Module

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Filename: 2022 AM PM Worst Case.j9

Path: C:\Users\Eoin\NRB Consulting Engineers Ltd\NRB Server - Documents\2020\20-070 Dawn Slane\Calculations\Yellow

Furze Picadys

Report generation date: 13/12/2020 12:37:29

»2022, AM

»2022, PM

Summary of junction performance

		AM				PM		
	Q (PCU)	Delay (s)	RFC	LOS	Q (PCU)	Delay (s)	RFC	LOS
				20	22			
Stream B-AC	0.0	6.42	0.02	А	0.0	5.83	0.01	Α
Stream C-AB	0.0	5.91	0.01	А	0.0	5.90	0.01	A

Values shown are the highest values encountered over all time segments. Delay is the maximum value of Av. delay per arriving vehicle.

File summary

File Description

Title	(untitled)
Location	
Site number	
Date	09/12/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	NRB-004\Eoin
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Av. delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perHour	S	-Min	perMin

Analysis Options

Calculate Q Percentiles	Calculate residual capacity	RFC Threshold	Av. Delay threshold (s)	Q threshold (PCU)
		0.85	36.00	20.00



	Scenario name	nmary Time Period nan	ne Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	
D1		AM	ONE HOUR	07:45	09:15	15	
D2	1	PM	ONE HOUR	16:45	18:15	15	
ID	Network flow sca	ling factor (%)					
A1	100.0	UU					
						5	
						97.	
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NO CO		bis,					
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ID	Network flow scaling factor (%)
A1	100.000



2022, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	Yellow Furze T Junction at Local Road	T-Junction	Two-way	1.45	А

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description	Arm type
Α	Local Road N		Major
В	Minor Road Approach at YF		Minor
С	Local Road S		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
С	7.00			100.0	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

7	Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
Г	В	One lane	3.00	90	90

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for AB	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	552	0.096	0.243	0.153	0.347
1	B-C	681	0.100	0.252	-	-
1	С-В	632	0.234	0.234	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D1	2022	AM	ONE HOUR	07:45	09:15	15



Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Av. Demand (PCU/hr)	Scaling Factor (%)
Α		✓	35	100.000
В		✓	11	100.000
С		✓	31	100.000

Origin-Destination Data

Demand (PCU/hr)

	То			
		Α	В	С
From	Α	0	2	33
	В	7	0	4
	C	24	7	0

Vehicle Mix

HV %s

	То				
From		Α	В	ပ	
	Α	0	1	8	
	В	1	0	1	
	C	8	1	0	

Results

Results Summary for whole modelled period

Stream	Max RFC	Max delay (s)	Max Q (PCU)	Max LOS
B-AC	0.02	6.42	0.0	A
C-AB	0.01	5.91	0.0	A
C-A				
A-B				
A-C			(9)	

Main Results for each time segment

07:45 - 08:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	8	583	0.014	8	0.0	6.324	А
C-AB	5	626	0.008	5	0.0	5.858	А
C-A	18			18			
A-B	2			2			
A-C	25			25			



08:00 - 08:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	10	581	0.017	10	0.0	6.362	А
C-AB	6	625	0.010	6	0.0	5.878	A
C-A	22			22			
A-B	2			2			
A-C	30			30			

08:15 - 08:30

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	12	579	0.021	12	0.0	6.417	A
C-AB	8	623	0.012	8	0.0	5.907	A
C-A	26			26			
A-B	2			2			
A-C	36			36			30

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	12	579	0.021	12	0.0	6.417	А
C-AB	8	623	0.012	8	0.0	5.907	А
C-A	26			26		7	
A-B	2			2	. 0		
A-C	36			36	116		

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	10	581	0.017	10	0.0	6.365	А
C-AB	6	625	0.010	6	0.0	5.881	A
C-A	22			22			
A-B	2			2			
A-C	30			30			

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	8	583	0.014	8	0.0	6.324	А
C-AB	5	626	0.008	5	0.0	5.860	А
C-A	18			18			
A-B	2			2			
A-C	25			25			
	رن،	Sign				-	
a divina	, CO,	Syca					



2022, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	Yellow Furze T Junction at Local Road	T-Junction	Two-way	1.07	Α

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D2	2022	PM	ONE HOUR	16:45	18:15	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Av. Demand (PCU/hr)	Scaling Factor (%)
Α		✓	32	100.000
В		✓	8	100.000
С		✓	42	100.000

Origin-Destination Data

Demand (PCU/hr)

		7	·o	
		Α	В	С
From	A	0	6	26
	В	2	0	6
	U	35	7	0

Vehicle Mix

HV %s

		T	·o	
X		Α	В	С
	Α	0	1	8
From	В	1	0	1
	U	8	1	0



Results

Results Summary for whole modelled period

Stream	Max RFC	Max delay (s)	Max Q (PCU)	Max LOS
B-AC	0.01	5.83	0.0	А
C-AB	0.01	5.90	0.0	А
C-A				
A-B				
A-C				

Main Results for each time segment

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	6	636	0.009	6	0.0	5.772	А
C-AB	5	626	0.008	5	0.0	5.852	Α
C-A	26			26			
A-B	5			5	4		
A-C	20			20	. 0		

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	7	634	0.011	7	0.0	5.796	A
C-AB	6	625	0.010	6	0.0	5.871	A
C-A	31			31			
A-B	5			5			
A-C	23			23			

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	9	632	0.014	9	0.0	5.830	A
C-AB	8	624	0.012	8	0.0	5.898	А
C-A	39			39			
A-B	7			7			
A-C	29			29			

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	9	632	0.014	9	0.0	5.830	A
C-AB	8	624	0.012	8	0.0	5.898	А
C-A	39			39			
A-B	7			7			
A-C	29			29			



17:45 - 18:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	7	634	0.011	7	0.0	5.799	А
C-AB	6	625	0.010	6	0.0	5.874	А
C-A	31			31			
A-B	5			5			
A-C	23			23			

18:00 - 18:15

B-AC 6 636 0.009 6 0.0 5.772 A C-AB 5 626 0.008 5 0.0 5.852 A C-A 26 26 26 20 20 20 20	C-AB 5 626 0.008 5 0.0 5.852 A C-A 26 26 26 26 26 20
C-A 26 26 26 AB 5 5 AC 20 20	C-A 26
AB 5 5 AC 20 20	AB 5 5 20 20 PHA AC 20 PRINTING DEPARTMENT.
AC 20 20	AC 20 20 INTRO PURE NATIONAL DEPARTMENT.
· NO PULL	aning Department, Viewing Pull
	C Planning



APPENDIX G

PiCADY Junction Capacity Model Output Existing T-Junction at L1600/Road to 'Yellow Furze'

Established T Junction at L1600/ Road to 'Yellow Furze' Summary PICADY Results in Order as included herein (Robust & Worst Case – Being "Scenario E") SELECTED CONSTRUCTION YEAR 2022

Modelled	Period Mean Max Q	Period Max
Scenario	(PCUs)	RFC
2022 Construction Year AM Peak	<1	0.07
2022 Construction Year PM Peak	<1	0.07

All Results Above are way below the recommended RFC of 0.85 (85% Capacity) and therefore no problems whatsoever are anticipated at the Junction in terms of Capacity or excessive vehicle Queues

NB Any Small Changes to Selected Construction Year 2022 or indeed significantly higher traffic volumes experienced, will clearly have no significant implications in terms of the conclusions of the Study in Light of the Favourable Results.



Junctions 9

PICADY 9 - Priority Intersection Module

Version: 9.0.1.4646 [] © Copyright TRL Limited, 2020

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The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the solution

Filename: 2022 AM PM Worst Case.j9

Path: C:\Users\Eoin\NRB Consulting Engineers Ltd\NRB Server - Documents\2020\20-070 Dawn Slane\Calculations\Northern

Yellow F L1600 Picadys

Report generation date: 13/12/2020 12:40:02

»2022, AM

»2022, PM

Summary of junction performance

		AM				PM		
	Q (PCU)	Delay (s)	RFC	LOS	Q (PCU)	Delay (s)	RFC	LOS
	2022							
Stream B-AC	0.1	5.83	0.05	А	0.1	6.17	0.07	Α
Stream C-AB	0.1	6.33	0.07	А	0.1	6.58	0.06	Α

Values shown are the highest values encountered over all time segments. Delay is the maximum value of Av. delay per arriving vehicle.

File summary

File Description

•	
Title	(untitled)
Location	
Site number	
Date	09/12/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	NRB-004\Eoin
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Av. delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perHour	S	-Min	perMin

Analysis Options

Calculate Q Percentiles	Calculate residual capacity	RFC Threshold	Av. Delay threshold (s)	Q threshold (PCU)
		0.85	36.00	20.00



	Scenario name	mmary Time Period nam	ne Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	
D1		AM	ONE HOUR	07:45	09:15	15	
D2		PM	ONE HOUR	16:45	18:15	15	
ID	Network flow sca	lling factor (%)					
A1	100.0	000					C
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Sala), Sla					
S. S), Slo	mino				

ID	Network flow scaling factor (%)
A1	100.000



2022, AM

Data Errors and Warnings

Junction Network

Junctions

	Errors and Warning ors or warnings	gs				
Jun Junct	ction Networ	·k				Only
Junct	ion Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS	3585
1	Yellow Furze T Junct	T-Junction	Two-way	1.77	А	
Arn						ing
Arms	Name	Descri	ption Arm type			\mathcal{U}_{i}
	L1600 East	Descri	Major		11.0	
	Minor Road Leading to Yello	w Furze	Minor			
С	L1600 West		Major			
	r Arm Coometry	I			1	

Junction Network Options

Driving side	Lighting	
Left	Normal/unknown	

Arms

Arms

Arm	Name	Description	Arm type
Α	L1600 East		Major
В	Minor Road Leading to Yellow Furze		Minor
С	L1600 West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
С	7.00			100.0	✓	1.00

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

	Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
Г	В	One lane	3.00	90	90

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for AB	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	552	0.096	0.243	0.153	0.347
1	B-C	681	0.100	0.252	-	-
1	С-В	632	0.234	0.234	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D1	2022	AM	ONE HOUR	07:45	09:15	15

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.



Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Av. Demand (PCU/hr)	Scaling Factor (%)
Α		✓	61	100.000
В		✓	31	100.000
С		✓	159	100.000

Origin-Destination Data

Demand (PCU/hr)

	То				
		Α	В	C	
	Α	0	1	60	
From	В	0	0	31	
	С	118	41	0	

Vehicle Mix

HV %s

	То			
		Α	В	ပ
	Α	0	8	2
From	В	8	0	2
	C	2	2	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max delay (s)	Max Q (PCU)	Max LOS
B-AC	0.05	5.83	0.1	A
C-AB	0.07	6.33	0.1	A
C-A				
A-B				
A-C				

Main Results for each time segment

07:45 - 08:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	23	669	0.035	23	0.0	5.683	А
C-AB	31	626	0.050	31	0.1	6.171	А
C-A	89			89			
A-B	0.75			0.75			
A-C	45			45			



08:00 - 08:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	28	667	0.042	28	0.0	5.745	А
C-AB	37	626	0.060	37	0.1	6.239	A
C-A	106			106			
A-B	0.90			0.90			
A-C	54			54			

08:15 - 08:30

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	34	664	0.051	34	0.1	5.830	Α
C-AB	46	626	0.073	46	0.1	6.327	A
C-A	129			129			
A-B	1			1			
A-C	66			66			30

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	34	664	0.051	34	0.1	5.830	А
C-AB	46	626	0.073	46	0.1	6.329	А
C-A	129			129	4	17	
A-B	1			1	. 0		
A-C	66			66	116		

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	28	667	0.042	28	0.0	5.748	A
C-AB	37	626	0.060	37	0.1	6.243	A
C-A	106			106			
A-B	0.90			0.90			
A-C	54			54			

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	Los
B-AC	23	669	0.035	23	0.0	5.686	А
C-AB	31	626	0.050	31	0.1	6.177	А
C-A	89			89			
Α·Β	0.75			0.75			
A-C	45	. 0		45			
	ر ن د ن	Sign					
SOL SOL	, C _C ,	Sic					



2022, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	Yellow Furze T Junct	T-Junction	Two-way	1.73	А

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D2	2022	PM	ONE HOUR	16:45	18:15	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Av. Demand (PCU/hr)	Scaling Factor (%)
Α		✓	150	100.000
В		✓	40	100.000
С		✓	87	100.000

Origin-Destination Data

Demand (PCU/hr)

		1	Го	
		Α	В	С
	Α	0	1	149
From	В	0	0	40
	U	52	35	0

Vehicle Mix

HV %s

		T	·o	
X		Α	В	С
	Α	0	8	2
From	В	8	0	2
	U	2	2	0



Results

Results Summary for whole modelled period

Stream	Max RFC	Max delay (s)	Max Q (PCU)	Max LOS
B-AC	0.07	6.17	0.1	A
C-AB	0.06	6.58	0.1	А
C-A				
A-B				
A-C				

Main Results for each time segment

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	30	652	0.046	30	0.0	5.899	А
C-AB	26	607	0.044	26	0.0	6.318	Α
C-A	39			39			
A-B	0.75			0.75	4	1	
A-C	112			112	. 0		

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	36	647	0.056	36	0.1	6.011	А
C-AB	32	603	0.052	32	0.1	6.426	A
C-A	47			47			
A-B	0.90			0.90			
A-C	134			134			

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	44	639	0.069	44	0.1	6.170	Α
C-AB	39	597	0.065	39	0.1	6.575	А
C-A	57			57			
A-B	1			1			
A-C	164			164			

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	44	639	0.069	44	0.1	6.170	A
C-AB	39	597	0.065	39	0.1	6.575	A
C-A	57			57			
A-B	1			1			
A-C	164			164			



17:45 - 18:00

Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
B-AC	36	647	0.056	36	0.1	6.012	А
C-AB	32	603	0.052	32	0.1	6.428	A
C-A	47			47			
A-B	0.90			0.90			
A-C	134			134			

18:00 - 18:15

B-AC 30 652 0.046 30 0.0 5.902 A C-AB 26 607 0.044 26 0.0 6.324 A C-A 39 39 39 39 39 39 39 39 39 30 <th>B-AC 30 652 0.046 30 0.0 5.902 A C-AB 26 607 0.044 26 0.0 6.324 A C-A 39 39 39 AB 0.75 AC 112</th> <th>B-AC 30 652 0.046 30 0.0 5.902 A C-AB 26 607 0.044 26 0.0 6.324 A C-A 39 39 0.75</th> <th>B-AC 30 652 0.046 30 0.0 5,902 A C-AB 26 607 0.044 26 0.0 6.324 A C-A 39 39 0.75 A AC 112 112</th> <th>Stream</th> <th>Total Demand (PCU/hr)</th> <th>Capacity (PCU/hr)</th> <th>RFC</th> <th>Throughput (PCU/hr)</th> <th>End queue (PCU)</th> <th>Delay (s)</th> <th>LOS</th>	B-AC 30 652 0.046 30 0.0 5.902 A C-AB 26 607 0.044 26 0.0 6.324 A C-A 39 39 39 AB 0.75 AC 112	B-AC 30 652 0.046 30 0.0 5.902 A C-AB 26 607 0.044 26 0.0 6.324 A C-A 39 39 0.75	B-AC 30 652 0.046 30 0.0 5,902 A C-AB 26 607 0.044 26 0.0 6.324 A C-A 39 39 0.75 A AC 112 112	Stream	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	End queue (PCU)	Delay (s)	LOS
C-A 39 AB 0.75 AC 112	C-A 39 AB 0.75 AC 112	C-A 39 39 39 0.75 0.75 0.75 0.75 0.75 0.75 0.75 0.75	C-A 39 39 39 0.75 0.75 0.75 0.75 0.75 0.75 0.75 0.75	B-AC	· · · · · ·	` ′	0.046	· · · · · ·	0.0	5.902	Α
AB 0.75 AC 112 112	AB 0.75	AB 0.75 0.75 0.75 0.75 0.75 0.75 0.75 0.75	AB 0.75 0.75 0.75 0.75 0.75 0.75 0.75 0.75	C-AB	26	607	0.044	26	0.0	6.324	Α
AC 112 112 112	AC 112 112	AC 112 112 112 Niewing Pull	AC 112 112 112 III III III III III III III	C-A							
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