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FAHY BEG WIND FARM

GEOTECHNICAL ASSESSMENT REPORT

Prepared for: RWE Renewables Ireland Ltd.



Date: August 2022

Unit 6, Bagenalstown Industrial Park, Bagenalstown, Co. Carlow, R21 XW81, Ireland T: +353 59 9723800 | E: info@ftco.ie CORK | DUBLIN | CARLOW www.fehilytimoney.ie Jare Planno Authority. Inspection Purposes Only



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1. INTRODUCTION

1.1 General

RWE Renewables Ireland (RWE) are applying to Clare County Council for consent for the proposed Fahy Beg Wind Farm in County Clare. The proposed development is located approximately 5km northwest of O'Briensbridge and 14km northeast of Limerick city.

1.2 Details of Proposed Works

The project primarily consists of a proposed wind farm site (the Site) of up to 8 no. wind turbine generators (WTG), a substation compound, grid connection route (GCR) and ancillary civil and electrical infrastructure.

The GCR will consist of a 38kV underground cable connecting the proposed Fahy Beg Wind Farm substation to the existing Ardnacrusha 110kV substation located approximately 10km southwest of the site.

The proposed wind farm site and grid connection includes lands in the townlands of Fahy More North, Ballymoloney, Ballykavin, Ballyquin More, Woodpark, Leitrim, Ballybrack, Fahy More South, Aharinaghmore, Tooreen, Aharinaghbeg, Knockdonagh, Roo East, Blackwater, Rosmadda West, Parkroe, Lackyle and Ballykeelaun.

An overview of the proposed project is presented in Figure 1.1 in Appendix 1.

1.3 Scope of Works and Proposed Objectives

Fehily Timoney and Company (FT) were engaged by RWE to undertake a geotechnical assessment of the proposed wind farm site with respect to slope stability.

This study is carried out in accordance with Eurocode 7: Part 2 (NSAI, 2007).

This report includes the following information:

- Site details including location, present use, proposed use etc.
- Site geology (bedrock, superficial deposits and made ground).
- Site hydrogeology.

Site hydrology.

- Any site-specific requirements.
- A summary of the intrusive site investigations completed at the site.



This report includes the following interpretative elements:

- Interpretation of the findings of the site walkovers and intrusive ground investigations. •
- Details of site constraints which may affect proposed site layout and engineering options. •
- estober estober coare pranning List of potential hazards at the site arranged into a Design Risk Register which will highlight any • topographic, geological or man-made hazards in the area and potential mitigation measures to be taken



Prior to undertaking the site walkover and intrusive ground investigations, a desk study was undertaken to provide relevant background information for the site. The desk study involved an examination of the following sources of information:

- OSI (2022), current and historic Ordnance Survey Ireland mapping and ortho-photography.
- Taluntais (1980), General Soil Map of Ireland.
- Geological Survey of Ireland (2022) GSI Public Data Viewer (www.spatial.dcenr.gov.ie).
- Environmental Protection Agency (2022) EPA online mapping (http://gis.epa.ie/Envision).
- Proposed layout of the development.

To determine the existing hydrogeological regime within the study area the following EPA and GSI online datasets and mapping from the sources outlined above were reviewed:

- Catchment & Management Units;
- Groundwater Bodies Status and Risk;
- Drinking Water Protection Areas;
- Groundwater Resources (Aquifers);
- Groundwater Wells and Springs;
- Karst Features; and
- Groundwater Vulnerability

2.1 Geology

2.1.1 Quaternary Deposits

The Quaternary deposits underlying the site and GCR is discussed below and presented in Figure 2.1.

The Quaternary deposits documented within the proposed development areas include:

- Till derived from Lower Palaeozoic sandstones and shales (TLPSsS),
- Bedrock outcrop or sub-crop (Rck),
- Gravels derived from Lower Palaeozoic and Devonian Sandstones (GLPDSs),
- Fen peat (FenPt),
- Till derived from Devonian Sandstones (TDSs), and
- Alluvium (A).

As shown in Figure 2.1 the majority of turbine locations and associated infrastructure are located within areas classified as Till derived from Lower Palaeozoic sandstones and shales with limited located within Gravels derived from Lower Palaeozoic and Devonian sandstones and bedrock sub-crop or outcrop.





The majority of the proposed grid connection route is underlain by Till derived from Devonian Sandstones with limited areas of bedrock sub-crop or outcrop, Till derived from Lower Palaeozoic sandstones and shales, Gravel derived from Lower Palaeozoic and Devonian sandstones and alluvium indicated along the proposed route.

During site walkovers area of shallow Peaty Topsoil deposits were noted across the site but these were generally very thin (0.1 to 0.2m thick) and were not considered to constitute Peat Deposits but rather a highly organic Topsoil with Peaty appearance.

The information above is taken from the Geological Survey of Ireland (GSI) online mapping - Quaternary Geology of Ireland (1:50,000 scale).

2.1.2 Solid Geology

The bedrock geology underlying the site and GCR is discussed below and presented in Figure 2.2.

The bedrock formations documented within the proposed development areas include:

- Carboniferous Lower Limestone Shale (LLS)
- Carboniferous Ballysteen Formation (BA)
- Carboniferous Waulsortian Limestones (WA)
- Upper Devonian Old Red Sandstone (ORS)
- Silurian The Broadford formation (BF)
- Silurian Cratloes Formation (CR)

The majority of the site is underlain by Old Red Sandstone which is described as red mudstones, siltstones and sandstones, and poorly sorted, polymict pebble conglomerates and breccia. The northern part of the site is underlain by Broadford formation, which is described as dominated by grey banded mudstones also containing abundant arenaceous horizons on the northern limb. On the southern limb of the Slieve Bernagh syncline, the formation is predominantly argillaceous in character (60% of outcrop). A fault line and unconformity exist between the two formations. The fault trends in an east-north-east to west-south-west direction.

The proposed grid connection route traverses the Old Red Sandstone Formation, as described above, at various sections of the route. The remainder of the route passes through (from north to south) Cratloes Formation, Lower Limestone Shale, Ballysteen Formation and Waulsortian Limestones (WA).

The information above is taken from the Geological Survey of Ireland (GSI) online mapping - 1:100,000 scale bedrock geology map.



2.2 Hydrogeology

2.2.1 Groundwater Vulnerability

The groundwater vulnerability within the site ranges from 'H – High' to 'X – rock at or near surface'. Along the proposed grid connection, the vulnerability classification ranges from 'Moderate' to 'Extreme' with localised areas of exposed bedrock (X).

The information above is taken from the Geological Survey of Ireland (GSI) online mapping – groundwater data viewer.

2.2.1 Groundwater Bodies Description

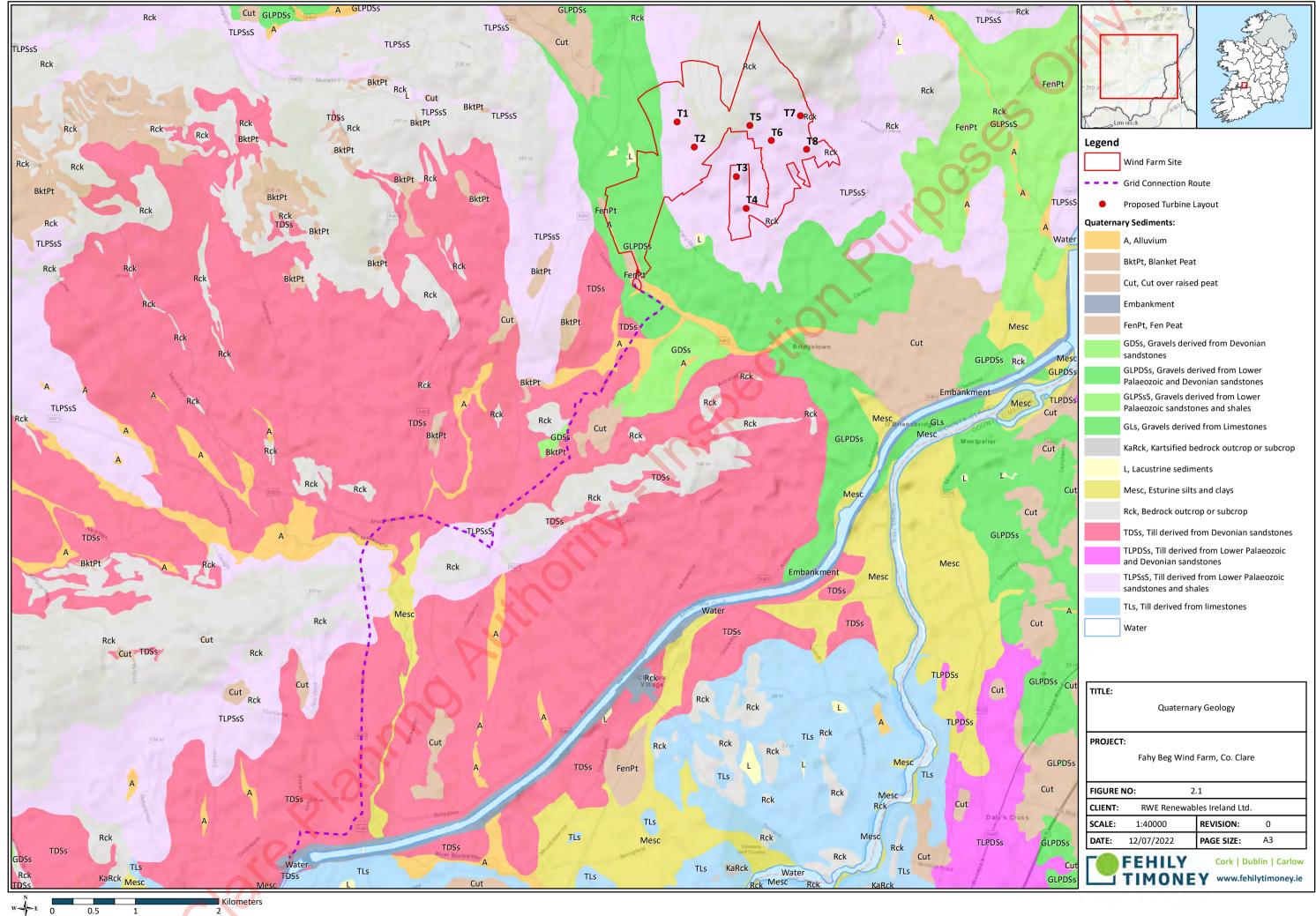
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The site is located within three groundwater bodies namely the Lough Graney, Tulla-Newmarket-on-Fergus, and Broadford Gravels. The GRC is underlain predominant by the Lough Graney GWB, with the northern area of the route underlain by Broadford Gravels GWB and the southern area of the route underlain by Ardnacrusha GWB.

The information above is taken from the Geological Survey of Ireland (GSI) – Groundwater Body Description sheets.

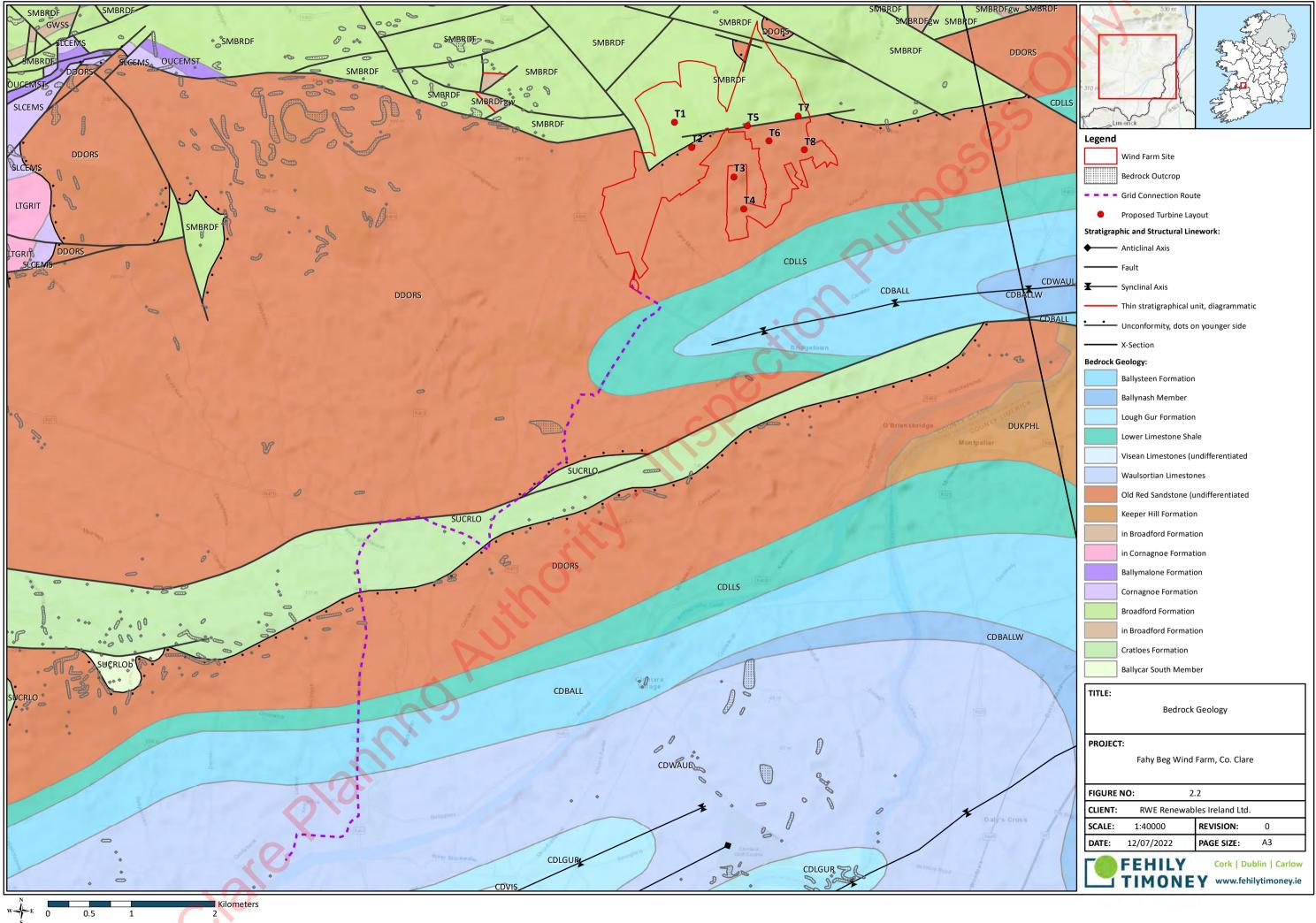
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3. SITE WALKOVER

As part of the geotechnical assessment site walkovers were carried out by FT during August 2021 and June 2022. The objective of the site walkovers was to determine the baseline characteristics of the proposed wind farm site. This included the recording of salient geomorphological features with respect to the wind farm development. The method adopted for carrying out the site walkover relied on practitioners carrying out a visual assessment of the site supplemented with slope inclination measurement.

The survey covered the proposed locations for the turbine bases, substation, met mast, construction compounds, existing and proposed new access roads and all associated infrastructure.

The method adopted for carrying out the site walkover relied on practitioners carrying out a visual assessment of the site supplemented with measurement of slope inclinations.

3.1 General

As outlined above, site walkovers were carried out by FT during August 2021 and June 2022. The method adopted for carrying out the site walkovers relied on practitioners carrying out a visual assessment of the site supplemented with recording of slope inclinations.

The assessment included a series of hand-held probes and hand shear vanes at proposed infrastructure locations to determine the presence/depth of peat within the proposed development site. Visual observations were also made to assess the stability of other soil slopes and rock exposures across the site.

The main findings of the site walkovers within the wind farm site are as follows:

The slopes of the proposed development site are characterised by elevated lands with typical elevations of between 120m to 350m AOD.

Slopes at proposed turbine locations in the development range from 4 to 12 degrees.

Peaty Topsoil deposits were noted throughout the site, but these were generally very thin (0.1 to 0.2m thick) and were not considered to constitute Peat Deposits but rather a highly organic Topsoil with Peaty appearance.

No evidence of past failures or any signs of peat instability were noted on site.

No evidence of slope instability in other soil or rock slopes was observed at the site and there are no historical records of landslide activity within the site boundary or 5km of the site, on the GSI database.

The forested areas have been planted predominantly with conifers with some deciduous in other areas of the forestry plantations. Ground conditions within the forested areas typically comprise thin cover of soft organic Topsoil over Mineral Soil and Glacial Till.

From site walkovers completed by FT it was noted majority of existing access tracks on site have been constructed using a founded construction method based on observations made during site walkovers. The access tracks for the proposed development will comprise upgrading of existing founded access tracks and construction of new proposed access tracks using excavate and replace construction techniques.

A summary of the information obtained during the field assessments is provided below in Table 4.4.

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4. GROUND INVESTIGATIONS

Intrusive ground investigations were undertaken by Irish Drilling Ltd (IDL) under the part-time supervision of an Engineering from FT in February, May, and July 2022.

The scope of the ground investigations is summarised below:

February 2022

- Advancement of 10 No. trial pits to a maximum depth of 3.0m below ground level (BGL) at proposed turbine locations and various infrastructure locations.
- Collection of samples for environmental and geotechnical testing.

<u>May 2022</u>

- Advancement of 5 No. trial pits to a maximum depth of 4.50m BGL at revised substation location.
- Collection of samples for environmental and geotechnical testing.

July 2022

• Advancement of 8 No. trial pits to a maximum depth of 4.50m BGL at revised substation locations.

The ground investigations were carried out in accordance with the principles in BS 5930:2015 and Eurocode 7 Part 2. A ground investigation location plan showing all trial pit locations is included as Figure 4.1 in Appendix 2 of this report.

4.1 Summary of Ground Conditions Encountered

The following section describes the ground conditions encountered during ground investigation completed at selected proposed turbine locations and the proposed substation locations.

4.1.1 Proposed Turbine Locations

Trial pits were excavated at all of the turbine locations. Geotechnical samples were collected from trial pits with the results described in Section 4.2 of this report. Refusal was recorded at 0.40m bgl for T3, 1.80m bgl for T6, 2.30m bgl for T4 & T7, 2.50m bgl for T2 & T8, 2.70m bgl for T5 and 3.00m bgl for T1. At T5 the rock was recorded as possible slate.

4.1.2 <u>Proposed Substation Location</u>

A total no. of 13 trial pits were dug around the 3 possible substation locations (5 at the first two substations and 3 at the third), to a maximum depth of 4.5m bgl. Geotechnical samples were collected from 5 of the 13 trial pits with the results described in Section 4.2.



4.2 **Groundwater Encountered**

Groundwater was recorded in multiple locations shown in Table 4.1 below.

Borehole/Trial Pit ID	Groundwater Strike (m bgl)
TP-T001	1.80
TP-T003	Ground level to 1.0
TP-T004	1.80
TP-001	2.30
TP-002	3.40
TP-003	4.10
TP-004	1.40
TP08	3.90

Table 4.1: **Summary of Groundwater Encountered**

4.3 **Geotechnical Laboratory Testing**

Following completion of intrusive site investigations by IDL laboratory testing was scheduled by FT. Soil testing was carried out in accordance with BS1377 (1990) – Methods of Test for Soils for Civil Engineering Purposes in their own designated Materials Laboratory, accredited in accordance with the Irish National Accreditation Board (INAB).

The samples of the overburden material were analysed for a range of parameters which included Particle Size Distribution (PSD), Moisture Content and Atterberg Limits. Chemical testing was also undertaken to determine Concrete Classification from the derived Sulphate Class for buried concrete.

The results are summarised in Table 4.2 & Table 4.3.

Table 4.2: Laboratory Testing (Feb 2020)

Туре	N	Min	Max	Remarks
Natural Moisture Content (%)	3	13	34	-
Particle Size Distribution	8			Typically described as Brown silty clayey very sandy GRAVEL with occasional fine rootlets & Brown slightly sandy gravelly silty CLAY
Hydrometer	3			-
Water soluble sulphate (2:1 water/soil extract)	3	0.021	0.032	-
рН	3	7.82	7.85	-



Table 4.3: Laboratory Testing (May 2022)

Natural Moisture Content (%) Atterberg Limits Particle Size Distribution Hydrometer Organic Content Water soluble sulphate (2:1 water/soil extract) pH	5 3 4 3 1 3 4 4	17 0.0069 7.76	47 0.0279 8.65	- Liquid Limit, LL 31% to 44% Plastic Limit, PL 22% to 26% Plasticity Index, PI 9% to 18% Typically described as Brown sandy SILT & Brown silty SAND. - Only one test carried out. Result – 0.795 -
Particle Size Distribution Hydrometer Organic Content Water soluble sulphate (2:1 water/soil extract)	4 3 1 3			Plastic Limit, PL 22% to 26% Plasticity Index, PI 9% to 18% Typically described as Brown sandy SILT & Brown silty SAND.
Distribution Hydrometer Organic Content Water soluble sulphate (2:1 water/soil extract)	3 1 3			
Organic Content Water soluble sulphate (2:1 water/soil extract)	1			- Only one test carried out. Result – 0.795 -
Water soluble sulphate (2:1 water/soil extract)	3			Only one test carried out. Result – 0.795 -
sulphate (2:1 water/soil extract)				
рН	4	7.76	8.65	ýQ,
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	Depth to Groundwater (Intrusive 1) Investigation)	1.80					
	Depth to Expected Bedrock (Intrusive Investigation)	3.00	2.50	0.40	2.30	2.70	1.80
	Ground Conditions (Intrusive Investigation)	Topsoil over firm Silt and weathered bedrock	Firm Silt over sandy Gravel and weathered bedrock	Topsoil over possible bedrock	Firm gravelly Silt over very sandy Gravel over gravelly Clay	Topsoil over gravelly Silt over silty Clay over sandy Gravel	Firm Silt over very sandy Gravel over Cobles and Boulders
	Groundwater Vulnerability (GSI Online Mapping)	H – High	H – High	X – Rock at or near surface	E – Extreme	H-High	н-Нigh
	Bedrock Geology (GSI Online Mapping)	Broadford Formation	Old Red Sandstone	Broadford Formation	Broadford Formation	Old Red Sandstone	Old Red Sandstone
	Quaternary Deposits (GSI Online Mapping)	Till derived from Lower Palaeozoic sandstones and shales	Till derived from Lower Palaeozoic sandstones and shales	Bedrock outcrop or sub-crop	Till derived from Lower Palaeozoic sandstones and shales	Till derived from Lower Palaeozoic sandstones and shales	Till derived from Lower Palaeozoic sandstones and shales
	Average Peat Depth (m)	0	0 0	0	0	0	0
	Slope (Site Walkover)	A8	ø	12	7	Q	S
\mathbf{i}	Land Use (Site) Walkover)	Agriculture	Agriculture	Forestry	Forestry	Forestry	Agriculture
	Exploratory Hole ID	ТР-Т001	ТР-Т002	TP-T005	ТР-ТОО7	ТР-ТОО6	ТР-Т008
	Proposed Infrastructure	T01	Т02	Т03	Т04	T05	T06

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Proposed Infrastructure	Exploratory Hole ID	Land Use (Site Walkover)	Slope (Site Walkover)	Average Peat Depth (m)	Quaternary Deposits (GSI Online Mapping)	Bedrock Geology (GSI Online Mapping)	Groundwater Vulnerability (GSI Online Mapping)	Ground Conditions (Intrusive Investigation)	Depth to Expected Bedrock (Intrusive Investigation)	Depth to Groundwater (Intrusive Investigation)
Τ07	TP-T004	Agriculture	04	o	Till derived from Lower Palaeozoic sandstones and shales	Old Red Sandstone	H – High	Topsoil over gravelly Silt over very sandy Gravel	2.30	1.80
T08	TP-T003	Agriculture	6	0	Till derived from Lower Palaeozoic Sandstones and shales	Old Red Sandstone	H – High	Topsoil over firm Silt over stiff gravelly Silt/Clay	2.50	g/l to 1.00
Compound	TP-C1	Overgrown area in Quarry	3	0	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H – High	Very soft organic silty Clay		1.50
Construction Compound	TP-S2	Overgrown area in Quarry	m	0	Fen Peat Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H – High	Very soft silty Clay over Boulders and Cobbles		1.70
Possible Substation (1)	TP-001	Overgrown area in Quarry	3	0	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H - High	Fine Sand over sandy silty Clay		2.30
Possible Substation (1)	ТР-002	Overgrown area in Quarry	3	0	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H – High	Fine Sandy over clayey Silt		3.40
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	5					Bedrock			Depth to	Domth to
	Exploratory Hole ID	Land Use (Site Walkover)	Slope (Site Walkover)	Average Peat Depth (m)	Quaternary Deposits (GSI Online Mapping)	Geology (GSI Online Mapping)	Groundwater Vulnerability (GSI Online Mapping)	Ground Conditions (Intrusive Investigation)	Expected Bedrock (Intrusive Investigation)	Depth to Groundwater (Intrusive Investigation)
Possible Substation (1)	ТР-003	Overgrown area in Quarry	03	0	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	h – High	Firm Clay with rootlets over very soft Clay	I	4.10
Possible Substation (1)	ТР-004	Overgrown area in Quarry	m	10	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H – High	Fine Sand with layer of soft Clay	I	1.40
Possible Substation (1)	ТР-005	Overgrown area in Quarry	ĸ	0	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H – High	Fine Sand with layer of soft Clay		I
Possible Substation (2)	TP01	Agriculture	6 – 16	0	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H – High	Topsoil over gravelly Sand	I	ı
Possible Substation (2)	TP02	Agriculture	6 – 16	0	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H - High	Topsoil over clayey Sand over gravelly Sand		ı
Possible Substation (2)	TP03	Agriculture	6 - 16	0	Gravels derived from Lower Palaeozoic and Devonian sandstones	Old Red Sandstone	H – High	Topsoil over gravelly Sand over Cobbles and Boulders		

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				3.90
Topsoil over gravelly Sand	Topsoil over clayey gravelly Sand	Gravelly Sand	Sand and Gravel over gravelly Sand	Topsoil over sandy Gravel over gravelly Sand
H – High	H – High	H – High	H – High	H – Hgh
Old Red Sandstone	Old Red Sandstone	Old Red Sandstone	Old Red Sandstone	Old Red Sandstone
Gravels derived from Lower Palaeozoic and Devonian sandstones	Gravels derived from Lower Palaeozoic and Devonian	Gravels derived from Lower Palaeozoic and Devonian sandstones	Gravels derived from Lower Palaeozoic and Devonian sandstones	Gravels derived from Lower Palaeozoic and Devonian sandstones
0		0	0	0
6-16	6 – 16	ĸ	ε	m
Agriculture	Agriculture	Overgrown area in Quarry	Overgrown area in Quarry	Overgrown area in Quarry
TP04	TP05	TP06	TP07	TP08
Possible Substation (2)	Possible Substation (2)	Possible Substation (3)	Possible Substation (3)	Possible Substation (3)
	TP04 Agriculture 6-16 0 Palaeozoic and Devonian Old Red Sandstone H - High	TP04 Agriculture 6 – 16 0 Palaeozoic and Palaeozoic and Bevonian Old Red Sandstone H – High TP05 Agriculture 6 – 16 0 Palaeozoic and Palaeozoic and from Lower Old Red Bevonian H – High TP05 Agriculture 6 – 16 0 Palaeozoic and Palaeozoic and Bevonian Old Red Bevonian H – High	TP04 Agriculture 6-16 0 Palaeozoic and Palaeozoic and Devonian Old Red Sandstones H – High TP05 Agriculture 6 – 16 0 Palaeozoic and Devonian Old Red Sandstones H – High TP05 Agriculture 6 – 16 0 Palaeozoic and Devonian Sandstones H – High TP05 Agriculture 6 – 16 0 Palaeozoic and From Lower Old Red From Lower H – High TP06 Overgrown 3 0 Palaeozoic and Devonian Sandstones H – High TP06 Overgrown 3 0 Palaeozoic and Devonian Sandstone H – High	TP04Agriculture6-160Palaeozoic and from Lower BevonianOld Red SandstonesH - High BevonianTP05Agriculture6 - 160Palaeozoic and DevonianOld Red SandstonesH - HighTP05Agriculture6 - 160Palaeozoic and DevonianSandstone SandstonesH - HighTP05Agriculture6 - 160Palaeozoic and DevonianSandstone SandstoneH - HighTP06Overgrown30Palaeozoic and DevonianSandstone SandstonesH - HighTP06Overgrown30Palaeozoic and DevonianSandstone SandstonesH - HighTP07Overgrown30Palaeozoic and DevonianPalaeozoic and SandstonesPalaeozoic and Sandstones

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The site walkover and ground investigations have generally confirmed the anticipated geology described in the Desk Study. A summary of the geological strata encountered during the ground investigations is summarised in Table 5.1 and Table 5.2 below.

Table 5.1: **Summary of Geology Encountered at Turbine Locations**

Strata	General Description	Depth to Top Range (m bgl)	Depth to Bottom Range (m bgl)
Topsoil	Firm grey slightly sandy gravelly SILT - TOPSOIL with rootlets	0.00	0.20 - 0.50
Glacial Till	cial Till Firm to stiff grey gravelly sandy SILT/CLAY		1.00 - 3.00
Gravel	Brown silty clayey very sandy GRAVEL with low boulder content	0.50 - 2.10	1.00 - 2.70
Weathered Bedrock	Recovered as flat and angular sandstone clasts with orange sandy gravel infill	0.2 - 1.0	0.4 - 2.5
Bedrock	Possible Slate rock	0.4 - 3.00	N/A

Table 5.2: Summary of Geology Encountered at Substation Locations

Strata	General Description	Depth to Top Range (m bgl)	Depth to Bottom Range (m bgl)
Made Ground	Soft wet greyish brown sandy SILT/CLAY	0.00	0.60 - 4.50
Topsoil	Firm brown slightly sandy CLAY	0.00	0.25 - 0.40
Gravel	Brown very sandy GRAVEL	0.25	0.60
Sand	Brown/orange clayey gravelly SAND	0.00 - 1.30	0.90 - 4.50
Glacial Till	Reddish slightly gravelly sandy CLAY	0.90	1.30
Cobbles & Boulders	Cobbles and Boulders (sub-angular)	3.30	3.50
Bedrock	Extracted boulders described as SILTSTONE with iron staining. Also described as probable SANDSTONE bedrock.	3.50 - 4.10	N/A

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6.1 Factors Controlling the Stability of Slopes

The factors controlling the stability of slopes are:

- Slope geometry
- Geology
- Properties of the slope material
- Groundwater levels
- Surcharge.

From a review of the GSI Landslide Susceptibility database the proposed development and proposed infrastructure locations are generally located within areas of 'Low' and 'Moderately High' susceptibility.

Slopes at these proposed turbine locations were recorded during site walkover to be moderate/steep with maximum slope angles of 12 degrees at turbine T05. This location was selected for slope stability assessment in accordance with the principles of Eurocode 7 (IS EN 1997-1).

6.2 Eurocode 7 and Partial Factors

In accordance with the principles of Eurocode 7 (IS EN 1997-1), rather than using a global factor of safety as per previous design codes, the factors of safety (termed partial factors) are applied to the chosen characteristic values to obtain design values. Actions (influences) are multiplied by the safety factor, while resistances are divided by the safety factor.

In accordance with Eurocode 7 (IS EN 1997-1), geotechnical checks must be carried out to ensure that the resistance preventing a slide are greater than or equal to the actions which cause a slide, i.e.:

 $E_d \le R_d$

Where:

 E_d = Sum of design actions R_d = Sum of design resistances

By adopting the methods of analysis given in Eurocode 7 (IS EN 1997-1), the factor of safety against failure is *included* in the partial factors (ranging from 1.0 to 1.3 for various parameters) applied to the analysis rather than to the end result. In order to verify that this condition is met, the resulting "safety ratio" must be equal or greater than 1.0 in order to verify that the above condition is met. i.e.: An in-situ "safety ratio" of less than 1.0 indicates that the slope currently has an inadequate factor of safety against failure and therefore is potentially unstable. Ratios greater than 1.0 indicate an adequate factor of safety against failure and are considered stable in both short and long term.

Table 6.1: Partial Factors used to Derive Design Parameters

Partial	Factor	Parameter	
γc'	1.25	Effective cohesion	
γ ≊'	1.25	Effective angle of friction	
γγ	1	Soil density	
γα	1.3	Loading (unfavourable)	
γ R;e	1	Earth resistance	

Table 6.1 shows the partial factors which have been applied to the characteristic values to give the derived parameters in Table 6.2 and 6.3 used during the slope stability analyses. The design parameter is derived by multiplying or dividing the characteristic values by the associated partial factor, i.e. tan $15(\phi') / 1.25(\gamma_{\phi'}) = 12.1^{\circ}$.

6.3 Slope Stability Analysis Method

Slope/W software of Geo Studio International was used to assess the stability of proposed slopes at turbine locations TO2 & TO3. Slope/W is a general software tool for the slope stability analysis of earth structures.

It uses the limit equilibrium method of analysis by using the idea of dissecting a potential sliding mass into vertical slices. It assesses the factor of safety for both moment and force equilibrium based on various methods, including Bishop, Janbu and Morgenstern-Price.

Using this software, it is possible to deal with complex stratigraphy, highly irregular pore-water pressure conditions, a variety of linear and nonlinear shear strength models, virtually any kind of slip surface shape, concentrated loads and pressure lines. Limit equilibrium formulations based on the method of slices are also being applied more and more to the stability analysis of structures such as tieback walls, nail or fabric reinforced slopes, and even the sliding stability of structures subjected to high horizontal loading arising.

Traditionally, the factor of safety is defined as that factor by which the shear strength of the soil must be reduced in order to bring the mass of soil into a state of limiting equilibrium along a selected slip surface. The results of the analysis show the overall stability of the embankment expressed as a factor of safety.

The definition of factor of safety used within SLOPE/W is:

 $F = \frac{\text{Available restoring moment (or forces)}}{\text{Total disturbing moment (or forces)}}$

Design values for use in the slope stability analysis have been derived using Eurocode 7 (IS EN 1997-1) Design Approach 1 Combination 2. This design approach is considered to be the most logical approach for slope stability analysis as it includes partial factors for both material properties and variable loads (for example traffic loads).



6.4 Limitations of Slope Stability Analysis

The application of traditional stability analysis such as this can be misleading as they assume a circular slip surface is the ultimate limit state. In reality, the ultimate limit state is likely to be non-circular in nature and as such these models may not be strictly modelling the critical limit state. Slope/W allows for some optimization of the slip surface within its analysis which reduces this limitation to some extent.

Despite the limitations outlined above, this method of slope analysis is still considered to provide a conservative analysis of the ultimate limit state and its use is in accordance with current industry best practice.

6.5 Material Properties

Table 6.2 below shows the typical parameters used for the Glacial Till and bedrock encountered beneath the turbine locations TO2 & TO3.

Material	Glacial Till (Cohesive)	Granular Fill (Class 1)	Gravel	Bedrock (Weathered)
Cohesion, c', kN/m ²	0	0	0	100
Effective Friction angle, ϕ' ,	30	38	30	30
Bulk unit weight, γ , kN/m ³	21	21	20	21
Undrained Cohesion c _u , kN/m ²	75		-	-

Table 6.2: Characteristic Parameters for Materials

6.6 Loading

A modelled loading of 250kN/m² was conservatively applied to the slopes during the analyses to simulate an Outrigger Pad either side of the lifting crane on the slopes. These Outrigger pads were placed a minimum of 2.5m from the slope edge. The distance between each Outrigger was assumed to be 3.0m.

For the purposes of the slope stability modelling all shallow soft deposits have been removed from the proposed location of the turbine foundation.

6.7 Slope Stability Analysis Models

A Slope/W model has been presented to reflect the proposed slopes at the turbine locations as outlined below:

- Model 1 Turbine T02
- Model 2 Turbine T03

The results of this analysis are summarised in Table 6.4 with safety ratios calculated for the Bishop method.



6.8 Slope Stability Analysis Results

The lowest safety ratio for potential slope failures (Table 6.4) was 1.027. Analyses were undertaken for both deep-seated (rotational) type slips and shallow (translational) type although the shallow translations failures within the overburden deposits gave the lower safety ratios.

Table 6.3: Slope Analysis results

have planno hutch

Model Name	Location	FoS – Drained	FoS - Undrained
1	T02	1.041	1.027
2	T03	3.105	2.456

The safety ratio for potential slope failure for drained conditions was 1.041 at proposed turbine location T02. By adopting the methods of analysis given in IS EN 1997-1, the factor of safety (FoS) against failure is included in the partial factors applied to the analysis rather than to the end result. A safety ratio of greater than 1.0 indicates that the slope is considered stable in the long-term drained conditions.

In order to maintain the safety of the slopes during the foundation and hardstands excavation works and associated cut and fill activities groundwater and surface water drainage should be maintained to mitigate the potential instability of the slopes. It is also recommended that surcharging loads i.e. construction traffic is limited to 10 kN/m² and a 0.5m exclusion zone from the edge of the crest of constructed slopes is maintained to prevent surface failures or shoulder failure at the crest of the slope.

In addition, it is recommended that the slopes are inspected after extended periods of heavy rain for any signs of instability such as tension cracks at the top of the slopes or bulging near the toe of slopes.



7. GEOTECHNICAL CONSIDERATIONS

7.1 **Turbine Foundations**

Based on the findings of the site investigations undertaken to date, a preliminary assessment of the likely foundation types found that a gravity foundation construction (founded) would be suitable for all of the proposed turbine foundations.

At the underside of the turbine foundation, a layer of structural up-fill (class 6N/6P - in accordance with TII requirements) will be required.

It should be noted that at detailed design stage a confirmatory ground investigation will be carried out at each proposed turbine locations to confirm the turbine foundation type. The ground investigation will be in the form of a borehole with in-situ SPT testing at 1.0m intervals in the overburden and follow-on rotary core through bedrock.

A summary of turbine foundation types estimated depth and founding stratum is provided below in Table 7.1.

Table 7.1: Turbine Foundation Summary

Proposed Infrastructure	Quaternary Deposits (GSI)	Ground Conditions	Slope (degrees)	Depth to Bedrock	Foundation Recommendation
		Encountered	C,		
T01	Till derived from Lower Palaeozoic sandstones and shales	Topsoil over firm Silt and weathered bedrock	8	3.00	Gravity foundation up to 3.0m bgl.
Т02	Till derived from Lower Palaeozoic sandstones and shales	Firm Silt over sandy Gravel and weathered bedrock	8	2.50	Gravity foundation up to 3.0m bgl
т03	Bedrock outcrop or subcrop	Topsoil over possible bedrock	12	0.40	Gravity foundation up to 3.0m bgl
T04	Till derived from Lower Palaeozoic sandstones and shales	Firm gravelly Silt over very sandy Gravel over gravelly Clay	7	2.30	Gravity foundation up to 3.0m bgl
T05	Till derived from Lower Palaeozoic sandstones and shales	Topsoil over gravelly Silt over silty Clay over sandy Gravel	6	2.70	Gravity foundation up to 3.0m bgl
T06	Till derived from Lower Palaeozoic sandstones and shales	Firm Silt over very sandy Gravel over Cobles and Boulders	5	1.80	Gravity foundation up to 3.0m bgl
т07	Till derived from Lower Palaeozoic sandstones and shales	Topsoil over gravelly Silt over very sandy Gravel	4	2.30	Gravity foundation up to 3.0m bgl
Т08	Till derived from Lower Palaeozoic sandstones and shales	Topsoil over firm Silt over stiff gravelly Silt/Clay	6	2.50	Gravity foundation up to 3.0m bgl



7.2 Access Tracks

It is considered all newly constructed access road will be of the founded type. Existing access road infrastructure will be incorporated into the design or improved upon through the use of widening and strengthening.

Founded roads are used in areas where competent ground is encountered at shallow depth. These roads are constructed by excavating until competent strata is encountered and then filling with a compacted 6F2 granular fill to road level. A layer of Class 804 material (in accordance with 800 series of the Specification for Road Works) is then used as a surfacing layer.

Tracks shall be observed during earthworks operations, if excessive rutting occurs, the pavement depth shall be increased.

Stone fill of suitable Class 6F2 material will be placed and compacted in accordance with the TII Specification for Road Works.

Where bearing stratum has slope greater than 1:1.5, benching should be carried out. Benches to be 0.5m Vertical & 1.0m Horizontal, with maximum crossfall of 2% on Horizontal section.

7.3 Crane Hardstands

Crane hardstands will all be founded. Crane hardstands are generally constructed using compacted Class 1/6F material on a suitable sub-formation to achieve the required bearing resistance. The hardstands will be designed for the most critical loading combinations from the crane. The founding levels for the hardstands may be variable across the site and will be determined during confirmatory ground investigation/detailed design stage.

The typical make-up of the hardstands would include up to 1.0m of compacted Class 1/6F material with geotextile and/or geogrid layers incorporated as required during detailed design stage.

7.4 Substation Foundations and Platforms

The substation platforms will be constructed using the founded technique. The substation foundations may comprise strip/raft foundations under the main footprint of the building with possibly a basement/pit for cable connections. Substation platforms are generally constructed using compacted Class 1/6F material with a suitable sub-formation to achieve the required bearing resistance.

Given the ground conditions present at the proposed substations, it is envisaged that the foundations will require to be founded on Glacial Till deposits. The typical make-up of the substation platform may include up to 750mm of granular stone fill with possibly a layer of geotextile and/or geogrid. At the underside of the substation foundations, a layer of structural up-fill (class 6N/6P) will likely be required.



7.5 Temporary Construction Compound Platforms

The construction compound platforms will be constructed using founded techniques. The construction compound platforms are generally constructed using compacted Class 1/6F material on a suitable sub-formation to achieve the required bearing resistance.

ed Case and or general second The typical make-up of the construction compound platform would include up to 500mm compacted Class 1/6F



Fehily Timoney & Company (FT) were retained by RWE Renewables Ireland Ltd. to undertake a geotechnical site assessment at the proposed Fahy Beg Wind Farm located in County Clare.

The slopes of the proposed development site are characterised by elevated lands with typical elevations of between 120m to 350m AOD.

A review of the published GSI datasets for the site indicated that the majority of turbine locations and associated infrastructure are located within areas classified as Till derived from Lower Palaeozoic sandstones and shales with limited areas located within Gravels derived from Lower Palaeozoic and Devonian sandstones and bedrock sub-crop or outcrop. The findings of the intrusive site investigations confirm the geological profiles outlined by the GSI mapping and datasets.

Maximum slope angles of 12 degrees at turbine T03 were recorded. This location has the proposed largest cut within the development and therefore was selected for slope stability assessment. T02 has the largest fill within the development and was also selected for slope stability assessment. A safety ratio for potential slope failures for drained conditions of 3.105 and 1.041 at proposed turbine locations T03 and T02, respectively. A safety ratio of greater than 1.0 indicates that the slope is considered stable in the long-term drained conditions.

Despite the development site having an acceptable margin of safety with respect to slope stability a number of mitigation/control measures are proposed to ensure that all works adhere to an acceptable standard of safety for work in upland site conditions. Mitigation/control measures identified for each of the infrastructure elements in the risk assessment should be taken into account and implemented throughout design and construction works.

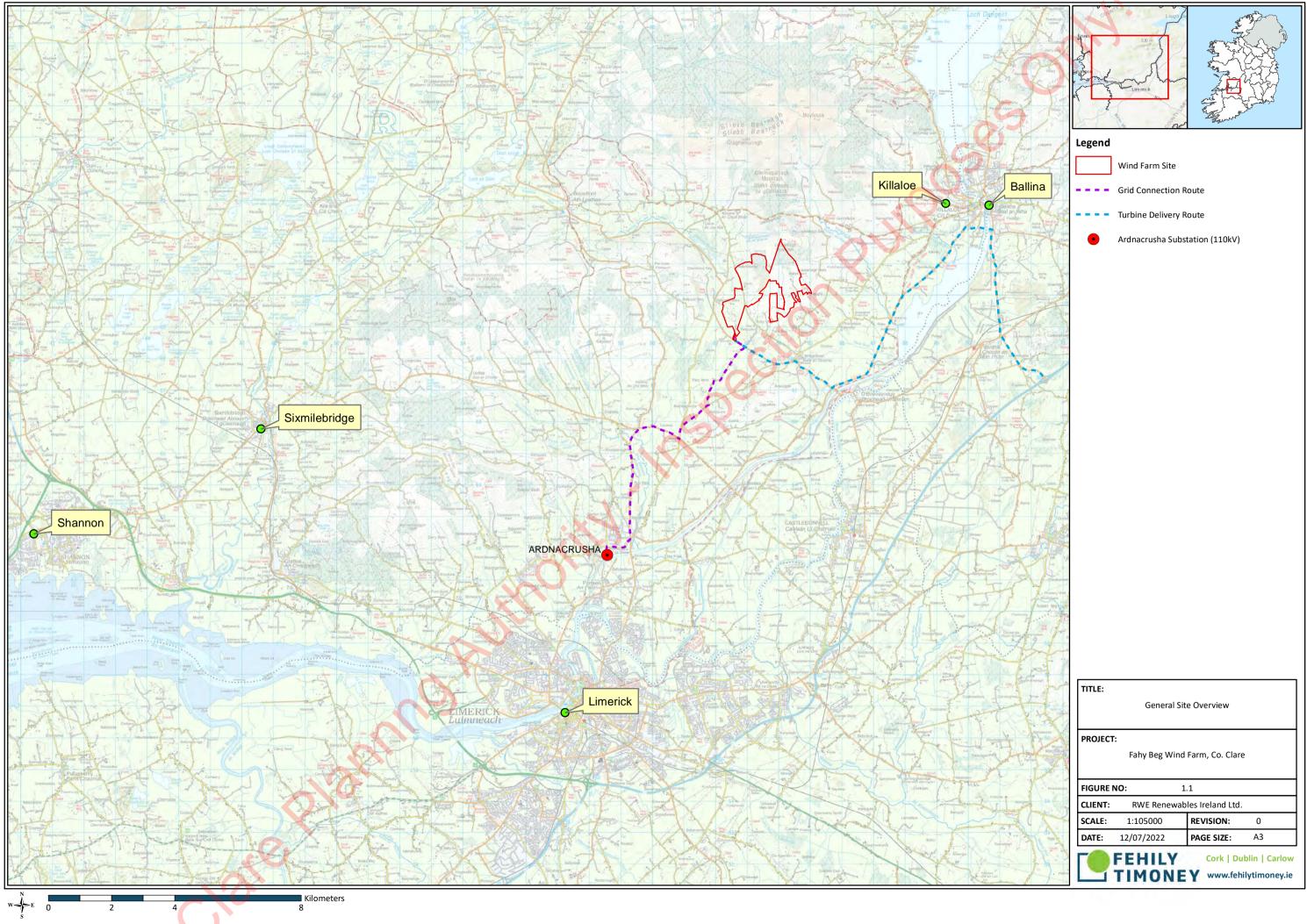
There is a risk of water ingress during excavation for the footings above the level of bedrock at the site. As such, provisions should be made for sump pumping should water ingress occur. Should foundations be required to advance below bedrock dewatering infrastructure should be considered prior to construction commencing.



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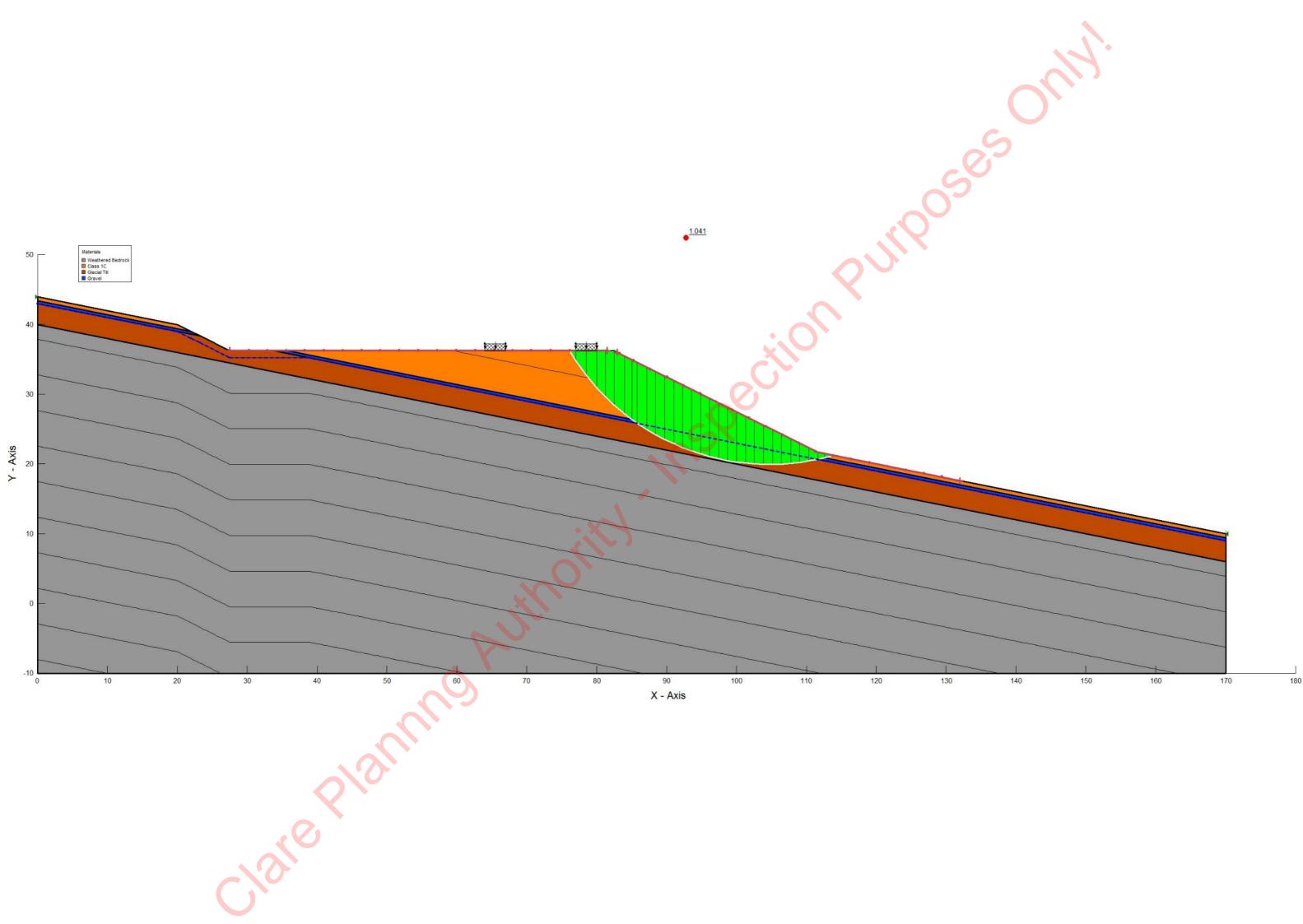
APPENDIX 1

sk heber clare Slope Stability Assessment



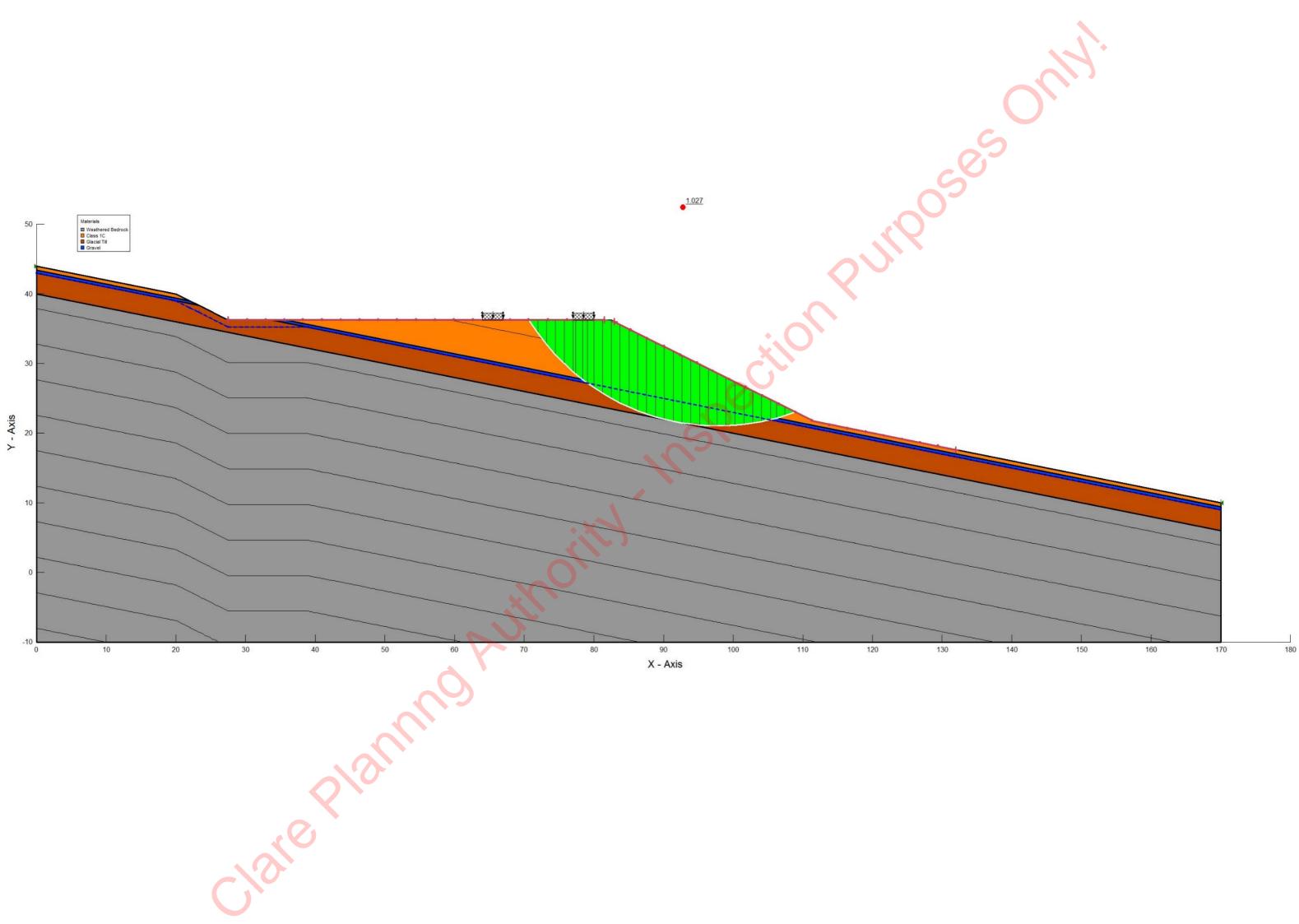
Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS Use Is Attribution 4.0 International (CC BY 4.0) licence https://creativecommons.org/licenses/by/4.0/, ESB 2017; If Applicable: Mapping Reproduced Under Licence from the Ordnance Survey Ireland Licence No. EN 0001219 © Governm





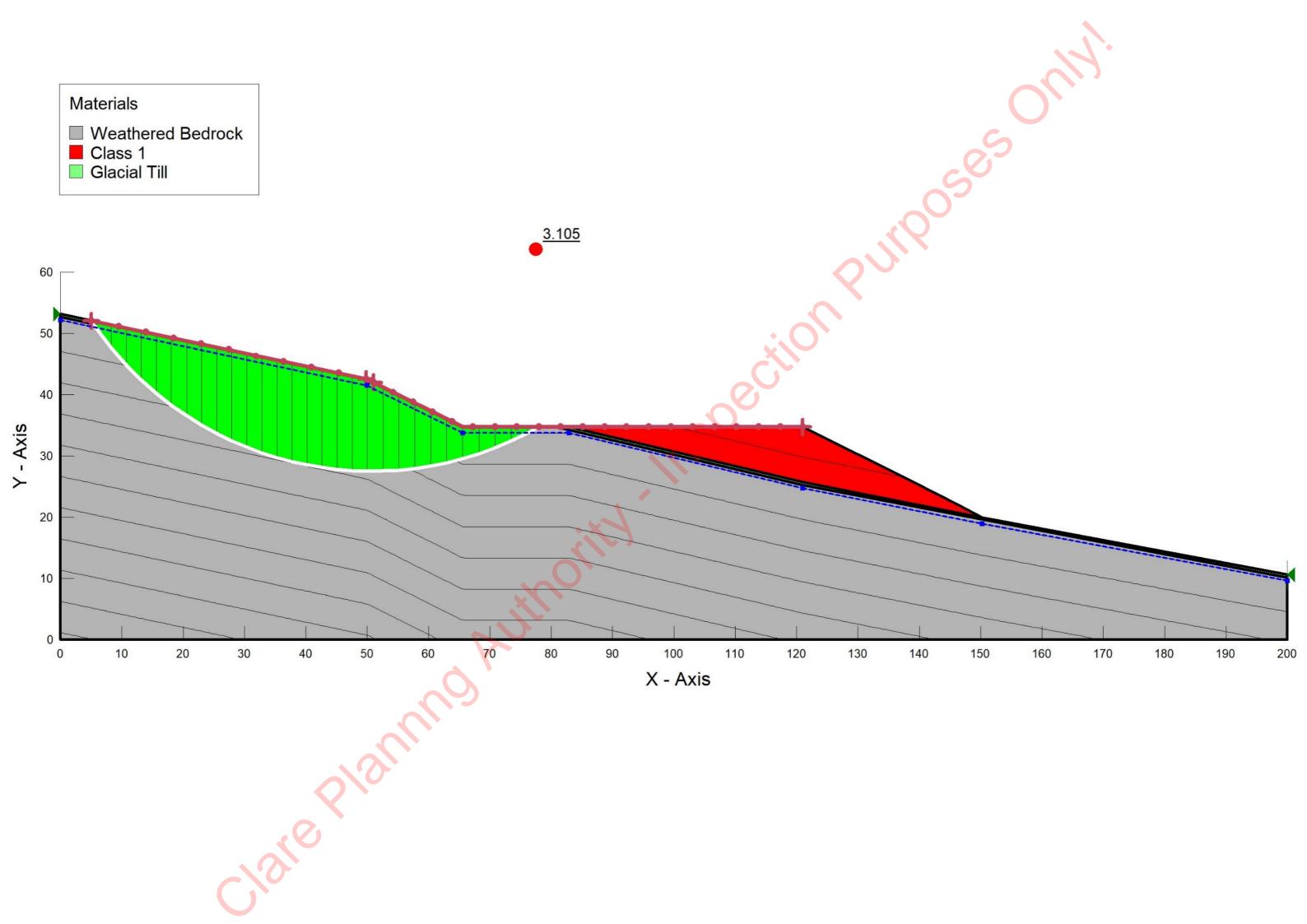




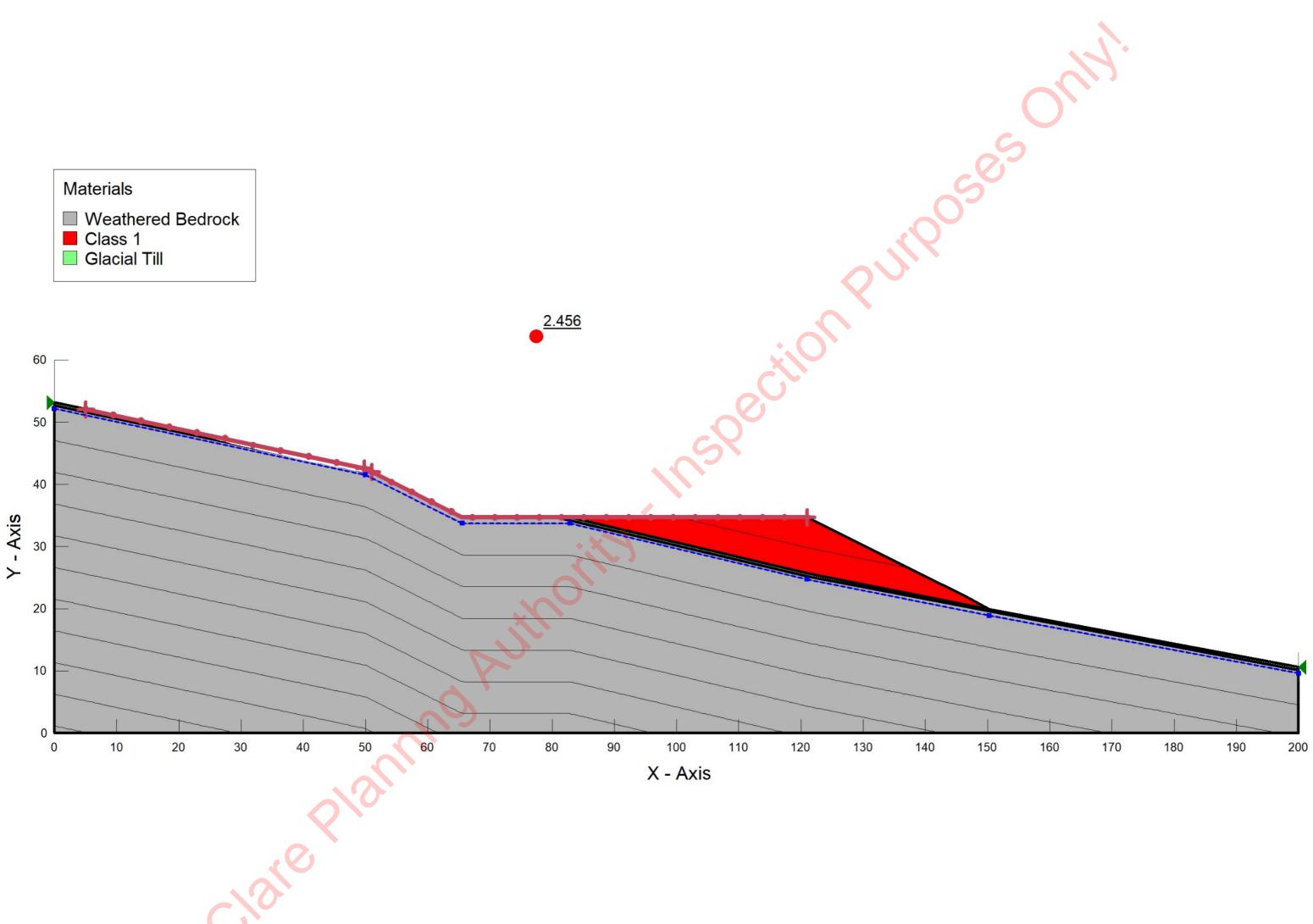


















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APPENDIX 2

Site Investigation Plan & Ground Investigation Factual Gare Planno Authority Report



Legend Wind Farm Site Grid Connection Route Proposed Turbine Layout Trial Pits TITLE: Ground Investigation Location Plan PROJECT: Fahy Beg Wind Farm, Co. Clare

FIGURE	NO: 4	1.1	
CLIENT:	RWE Renewa	oles Ireland Ltd	
SCALE:	1:14000	REVISION:	0
DATE:	12/07/2022	PAGE SIZE:	A3
	FEHILY TIMONE	1.0	ublin Carlow hilytimoney.ie



IRISH DRILLING LIMITED



LOUGHREA, CO. GALWAY, IRELAND

CONTRACT DRILLING SITE INVESTIGATION

Phone: (091) 841 274 Fax: (091) 847 687

email:

info@irishdrilling.ie

FAHY BEG WIND FARM

SITE INVESTIGATION CONTRACT FACTUAL REPORT

RWE, RWE Platz 1, 45141, Essen. Fehily Timoney & Company, Consulting Engineers, Singleton's Lane, Bagenalstown, Carlow.

X	Prepared by	Approved by	Rev. Issue Date:	Revision No.
	Ronan Killeen	Declan Joyce	8 th August 2022	21_CE_102/03
<u>Signature</u>				

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FOREWORD

The trial pit records have been compiled from an examination of the samples by a Geotechnical Engineer and from the Drillers' descriptions.

The report presents an opinion on the configuration of the strata within the site based on the trial pit results.

The fieldwork was carried out in accordance with IS EN 1997-2 and BS5930, 2015 Code of Practice for Site



urposes

Contents:

1.0 2.0 3.0 4.0	Introduction The Site & Geology Fieldwork Laboratory Testing
Book 1 of 1	
Appendix 1	Trial Pit Records
A second se	Trial Dit Deserve (Dhese)

- Appendix 1a Trial Pit Records (Phase 2)
- Appendix 2 Laboratory Test Results
- Appendix 2a Laboratory Test Results (Phase 2)
- Appendix 3 Site Plan
- Appendix 4 Trial Pit Photographs (Phase 2)
- Appendix 5 AGS Data



1.0 Introduction.

Irish Drilling Ltd. (IDL) was instructed by Fehily Timoney & Partners, Consulting Engineers, on behalf of RWE Ireland, to carry out a site investigation at the site of the proposed Fahey Beg Wind Farm Project.

This site investigation was carried out to provide detailed factual geotechnical information of the underlying ground conditions at the location of the proposed works.

Phase 1 fieldwork commenced on February 3rd 2022 and was completed on February 7rd 2022.

Phase 2 fieldwork was carried out on May 13th 2022.

2.0 Site & Geology

The site is located southwest of Killaloe, County Clare.

The site is agricultural in nature and the fieldwork was carried out predominantly on agricultural lands.

Weather conditions in general were quite variable with the majority of the fieldwork carried out over a typical winter period in Ireland.

Geological Survey maps of the area indicate that the site is underlain by Limestone and Sandstone Rock Formations.

A Site Plan, prepared by the client's representatives to show approximate fieldwork locations, is included with this report.

3.0 Fieldwork.

The following plant was mobilised to site to carry out fieldwork operations:

Phase 1 Fieldwork:

1nr Kobelco 7T Tracked Excavator.

Fieldwork carried out to date has included the following:

Ten trial pits were excavated on site using a tracked excavator.

The pits were logged and photographed by an Engineer with observations made on ground conditions, pit stability, water ingress and services encountered.

Small and bulk disturbed soil samples were recovered at each change in strata and returned to the laboratory and presented for testing.

The pits were excavated to depths ranging from 0.40m to 3.00m below ground level.

Phase 2 Fieldwork:

1nr Hitachi 13T Tracked Excavator.

Fieldwork carried out to date has included the following:

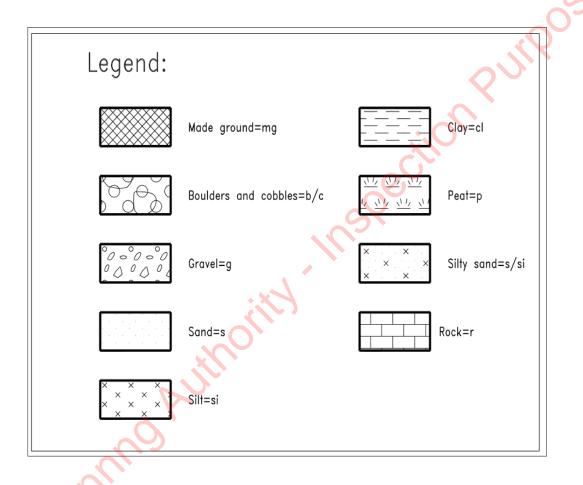
Five trial pits were excavated on site using a tracked excavator.

The pits were logged and photographed by an Engineer with observations made on ground conditions, pit stability, water ingress and services encountered.

Small and bulk disturbed soil samples were recovered at each change in strata and returned to the laboratory and presented for testing.

The pits were excavated to depths ranging from 2.30m to 3.40m below ground level.

The following Key Legend Table details the symbology used on the engineering logs to describe ground conditions encountered:



Ground conditions encountered during the completion of the fieldwork were typical and as expected for this region and predominantly consisted of Glacial Tills overlying bedrock.

The Glacial Tills in general consisted of grey and brown slightly gravelly sandy silt with cobbles and boulders and/or silty clayey sandy gravel with cobbles and boulders.

Possible weathered bedrock was also encountered at trial pit T005 while a number of trial pits encountered 'refusal' on possible bedrock at relatively shallow depths.

Phase 2 fieldwork operations encountered possible made ground to depths of up to 3.40m and predominantly consisted of silty sands interbedded with silt/clay. It is possible that the area where the trial pits were excavated was previously used as a settlement pond for an adjacent quarry.



For detailed descriptions of the ground conditions encountered please refer to the engineering logs included as Appendix 1 and Appendix 1a of this report.

The fieldwork was carried out in accordance with IS EN 1997-2 and BS5930, 2015 Code of Practice for Site Investigations with precedence given to IS EN 1997-2 where applicable.

The fieldwork locations were set out on site using a Trimble CU Bluetooth GPS Surveying Unit and the co-ordinates are included on the logs presented in the appendices. All fieldwork co-ordinates are reported to Irish Transverse Mercator (ITM) with Reduced Levels recorded relative to Malin Head Datum and with an accuracy level of + or -0.10m.

4.0 Laboratory Testing

Representative samples recovered from the boreholes and trial pits were scheduled for testing in the laboratory.

The test schedules were prepared by the Client's Representative and included some or all of the following tests on disturbed soil samples:

- * Moisture Content.
- * Atterberg Limits.
- * Particle Size Distribution.
- * Sedimentation.
- * Chemical (pH, Sulphate).
- * Organic Content.

The records of these laboratory tests results are included as Appendix 2 and Appendix 2a of this factual report.

The soil descriptions as noted on the trial pit logs are in general visual descriptions as observed and logged by our Engineers and are described in accordance with IS EN 1997-2 and BS5930, 2015 Code of Practice for Site Investigations.

Soils descriptions (cohesive or otherwise) are also initially assessed based on the texture and 'feel' of the soil materials as witnessed by our Geotechnical Engineers and in accordance with IS EN 1997-2 and BS5930.

Where laboratory classification tests have been carried out on soil or rock samples then these visual descriptions have been amended accordingly to take into account the results of these classification tests.

The records of all fieldwork, laboratory test results and photographs are included in the appendices of this Factual Report.

Ronan Killeen Chartered Engineer Irish Drilling Limited July 19th 2022



tare planned without inspection purposes on

PROJECT: LOCATIO									TRIALPIT: TP-C1 Sheet 1 of 1
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Ground level: GROUNDV Water strikes: 1st: 1.50m 2nd: 3rd:	VATE	O.D. R e to after:			PIT I PIT I LOG	DIREC DIME GED 1	CTION NSION BY:	990-270 A .10 * 2.20 D MM C	DATE: 3.2.22 Shoring/Support: N/A Stability: Pit unstable. Sidewall 1.10 collapse.
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LOCATION	: Killalo	eg Wind Farm De, Co. Clare						TRIALPIT: TP-S1 Sheet 1 of 1
CLIENT: R ENGINEER:		Timoney					Co-ordinates: E 562,539.6 N 668,774.4	Rig: 7T Tracked Kobelco Rev: FINAL
Ground level: 4	5.19m O.I							DATE: 3.2.22
GROUNDWA Water strikes: 1st: 1.70m 2nd: 3rd:	ATER Rose to :	after:		PIT 1	DIREC DIMEI GED I	NSION	090-270 1.10 * 2.30 D C	H Shoring/Support: N/A Stability: Pit unstable. B ↓ 1.10
Depth (m) Date	Water	Samples Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)	DESC	CRIPTION
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D	ngress of v	water at 1.70m bgl.	TP back	filled w	ith arisir	igs.		Scale:
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		-	-	'ind Farm Co. Clare						TRIALPIT: TP-T001 Sheet 1 of 1
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	GINEER:			oney		1			E 563,039.4 N 670,613.	
GRC	nd level: 1 DUNDW: r strikes: 1.80m	ATE				PIT	DIREC DIME GED 1	NSION	090-270 → 3.40 1.00 * 3.40m D MM C	DATE: 3.2.22 → Shoring/Support: N/A Stability: Pit stable. B 1.00
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						ו×× × •× *•× *•×	•	0.55	Firm orangish grey slightly gravelly sand subrounded fine to medium.	y SILT. Sand is medium. Gravel is rounded to
1						× • × • × • × • × • × • × • × • × • × •	114.46	0.90	Stiff damp grey slightly gravelly sandy Si shale.	ILT. Gravel is angular to rounded of siltstone and
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3						$\frac{1}{8}$	112.36	3.00	TP terminated at 3.00m bgl. Unable to m	ake progress - hard digging.
4			0							
3										
3 4 5 Rem										
5 Rem	arks: ^h	ngress	of water	at 1.80m bgl.	TP back	filled w	ith arisir	ngs.		Scale:
									drilling LTD	1:25

	DJECT: 1 CATION:	-	-	ind Farm								TRIALPIT: T] Sheet 1 of 1	P-T002
CLII	ENT: R	WE								Co-ordinates: E 563,245.5 N 670,310		Rig: 7T Tracked Rev: FINAL	Kobelco
	GINEER: nd level: 1			oney						L 505,245.5 IN 070,310		Kev: FINAL DATE: 3.2.22	
GRC	DUNDWA r strikes: dry	АТЕ				PIT	DIREC DIME GED 1	NSION	: 000-1 : 1.00 MM	80 → 3.00 A * 3.00m D C		Shoring/Support: Stability: Pit stabl	N/A e.
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DE	SCRIP	TION	<u> </u>
0						×o × × × ×			Grass o	ver firm brown gravelly SILT. G	ravel is a	ngular to subangular fir	e to medium.
						× × ×	142.02	0.30	Drown	sh orange gravelly SILT.			
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			B 1	0.60-1.00			141.72	0.60	Brown	silty clayey very sandy GRAVEL is fine to medium.	with trac	ces of rootlets.	
			100000			\$ 1.8			Gravel	is mile to medium.	0	<i>), ,</i>	
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													1:25

	DJECT:	•		ind Farm								TRIALPIT: TP-T003 Sheet 1 of 1
CLI	ENT: R	WE								Co-ordinates:		Rig: 7T Tracked Kobelco
	GINEER:		•	oney		1				E 563,748.2 N 669,95	9.3	Rev: FINAL
GRO	nd level: 1 DUNDWA r strikes: 0.00m	ATE				PIT 1	DIREC DIMEI GED I	NSION	: 000-1 : 1.10 MM	80 → 3.40 → 3.40 → A → A → A → A → A → A → A → A → A →		DATE: 4.2.22 Shoring/Support: N/A Stability: 1.10
Depth (m)	Date	♦ Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		D	ESCRII	PTION
0		_					153.67	0.40	Water s	IL: Grass over firm brown slig eepage from GL to 1.00m dept	h observed	
· 1											2	JIP
							150.45	1 (0)	1.00m 1	o 1.60m: large boulder within s	southern fa	ace of 1P.
2			в 1 100000000000	1.60-2.00			152.47	1.60	Stiff blu rounded	uish grey slightly sandy gravelly I to subangular fine to medium.	y SILT/CL	AY with low cobble content. Gravel is
							151.57	2.50	TP term	ninated at 2.50m bgl. Obstruction	on as poss	ible rock or boulder.
3					8	Š						
			N.									
4	Ke K											
5 Rem	narks: h	ngress	of water :	from g/l to 1.0	00m bgl.	TP bac	kfilled w	ith arisir	ngs.			Scale:
								Tutal	J	ing LTD		1:25

)JECT: CATION	-	-	ind Farm Co. Clare								TRIALPIT: TP-T004 Sheet 1 of 1
CLI	ENT: R	WE								Co-ordinates:		Rig: 7T Tracked Kobelco
	GINEER: nd level: 1			oney						E 563,872.0 N 669,577.		Rev: FINAL DATE: 4.2.22
GRC	DUNDW r strikes: 1.80m	ATE				PIT	DIREC DIME GED 1	NSION	: 000-1 [: 1.10 MM	80 → 3.00 → 3.00 → A → 3.00 → 3		Shoring/Support: N/A Stability: Pit stable.
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DE	SCRIP	TION
0							-		TOPSO	DIL: Grass over firm brown SILT.		6
				0.70.1.00		— — ×	125.12			rown gravelly SILT. Gravel is ang	-	100SC
1			<mark>6</mark> 81 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	0.70-1.00					Firm g angular sandsto	rey mottled orange slightly sandy to subrounded of shale and sand me.	gravelly Istone. Co	SILT with low cobble content. Gravel is obbles are rounded to subangular of
		Į Ţ	B 2	1.70-2.30			123.62	1.70	subang	a brown silty very sandy GRAVE ular fine to medium. water seepage from pit side walls		w boulder content. Gravel is rounded to
2			neveneranea 1			\$0000000000000000000000000000000000000	123.02	2.30	Slight	water seepage from pit side walls	observed	1 at 1.80m depth.
3	Q1	2				END			1P terr	ninated at 2.30m bgl. Unable to n	nake proş	gress - hard digging.
5 Rem	arks: F	ngress	of water a	at 1.80m bgl.	TP back	filled w	ith arisir	gs.				Scale:
m		- -			- 5408			o				1:25
1								Irist	dril	ing LTD		Ph. Fax

	DJECT: 1 CATION:	-	-	ind Farm o. Clare							TRIALPIT: T Sheet 1 of 1	Р-Т005
CLII	ENT: RV GINEER:	WE								ordinates: 3,791.9 N 670,570.4	Rig: 7T Tracked Rev: FINAL	Kobelco
Grou	nd level: m DUNDWA	1 O.D.		v					1		DATE: 7.2.22	
	r strikes: dry		N e to after:			PIT	DIREC DIME GED I	NSION	090-270 2.00 * 3.30r MM	A	Shoring/Support: Stability: Pit stab	N/A lle.
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DESC	CRIPTION	oni
) -								0.20		s roots and briars over firm bunded of sandstone.	brown gravelly SILT with b	oulder. Boulders are
				0.40				0.40		•	asts with orange sandy grave	l infill.
1						END				t 0.40m bgl. Obstruction as	RUIR	
3				no	4	<u>z</u> e	0	N.				
4			2									
5 Rem	arks: T	P dry	on excava	tion. TP back	filled wi	 ith arisi	ngs.	•		with a putting.		Scale:
	U	nable	to survey represent	due to dense	toliage -	 locatio 	n set out	t using h	dhelkd GPS and	with co-ordinates as receiv	ved from	1:25

	DJECT: 1 CATION:	-	-	ind Farm								TRIALPIT: TP-T006 Sheet 1 of 1
CLI	ENT: RV	WE								Co-ordinates: E 564,167.0 N 670,388]	Rig: 7T Tracked Kobelco Rev: FINAL
	JINEER: nd level: n			oney						E 304,107.0 IN 070,388		Rev: FINAL DATE: 7.2.22
GRO	DUNDWA strikes: dry	ATE				PIT	DIREC DIME GED 1	NSION	: 000-1 : 1.00 MM	80 → 3.2		Shoring/Support: N/A Stability:
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DE	SCRIP	TION
) -								0.38		IL: Grass and briars over firm da	-	n silty CLAY.
l						* • × • × * • × • ×			to med	um of shale.		SILT. Graver is angular to subangular line
			B 1	1.20-1.80				1.20	Stiff da siltston	mp blue mottled grey slightly san e and shale.	ndy gravel	lly silty CLAY. Gravel is fine to coarse of
2			3 2 200000000000000000000000000000000000	2.10-2.70				2.10		grey brown silty sandy GRAVE is fine to coarse, angular to subar		
3			5.	0,00	6	END			TP tern	ninated at 2.70m bgl. Obstruction	n as possib	ole slate rock.
	ie i		G									
5 Rema	arks: T	P dry o Inable	on excava to record	ation. TP back reduced level	filled wi	ith arisin foliage.	ngs.					Scale:
										ing LTD		1:25

			ind Farm Co. Clare								TRIALPIT: TP-T007 Sheet 1 of 1
CLIENT	: RWE								Co-ordinates: E 564,519.2 N 670,704.		Rig: 7T Tracked Kobelco Rev: FINAL
ENGINE Ground lev			ioney						2 301,317.2 11 070,704.	-	DATE: 7.2.22
GROUN Water strike 1st: dr 2nd: 3rd:	DWATI s: R				PIT 1	DIREC DIME GED 1	NSION	: 320-1 : 1.10 MM	40 3.30 A ★ 3.30 D C		Shoring/Support: N/A Stability:
Depth (m)	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DES	SCRI	PTION
)					× × × × × × × × × × × × × × × × × × ×		1.00			2	ge slightly sandy sightly gravelly SILT.
		ана 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1.00-1.50		₩ 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		1.50		mottled grey silty clayey very same is fine to coarse.	•	
2									with high boulder content. Bould		CLAY. Gravel is flat and angular to rounded to subrounded of sandstone and
	_						2.30				
3 4		21				Or			ninated at 2.30m bgl. Obstruction	ar bosz	
5 Remarks	: TP dr	y on excave	ation. TP back	filled wi	th arisi	ngs.					Scale:
	Unabi	e to record	ation. TP back reduced level	due to f	foliage.	.					1:25
Day							Irish	dril	ling LTD		Ph. Fax

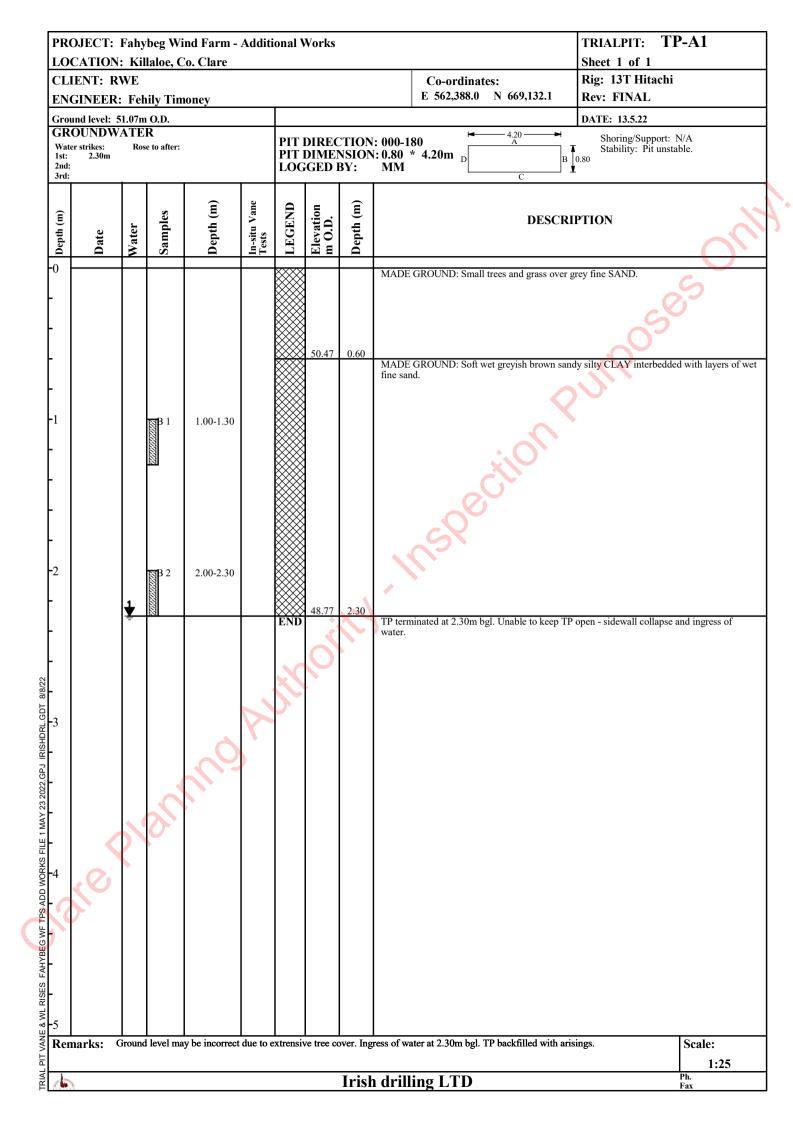
LOC	CATION	Kil	-	ind Farm Co. Clare							Sheet 1 of 1	P-T008
	ENT: RV GINEER:		nily Tim	oney						Co-ordinates: E 564,593.3 N 670,291.8	Rig: 7T Tracked Rev: FINAL	Kobelco
Grou	nd level: 1 DUNDW	57.16	m O.D.	v							DATE: 7.2.22	
	r strikes: dry		n after:			PIT	DIREC DIME GED 1	NSION	: 040-2 : 1.00 MM	$\begin{array}{c c} 330 & 320 \\ 330 & \mathbf{320m} \\ 5 & \mathbf{320m} \\ 5 & 5 \\ 5 \\ 5 & 5 \\ 5 & 5 \\ $	Shoring/Support: Stability: Pit stabl	N/A e.
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DESC	RIPTION	00
0							156.96	0.20		DIL: Grass over firm brown gravelly S		S
			ज्ञ B 1	0.50-1.00			156.66	0.50			S	
				0.50-1.00					Pinkisl boulde Traces	n grey brown silty clayey very sandy or r content. Gravel is angular to subang of rootlets.Gravel is fine to coarse.	GRAVEL with low cobble co gular of shale and siltstone.	ntent and medium
1			22							ion		
							155.66	1.50	Blocky BOUL	and angular to subrounded COBBLI DERS with a silty gravelly infill.	ES and blocky and angular to	subrounded
							155.36	1.80				
2						END			I P teri	ninated at 1.80m bgl. Obstruction as	possible rock.	
							Ś					
						X						
3				A								
				200								
			0									
4	.0											
2												
5												
Rem	narks: T	P dry	on excava	ation. TP back	filled w	ith arisi	ngs.					Scale: 1:25
1								Irisł	dril	ling LTD		Ph. Fax

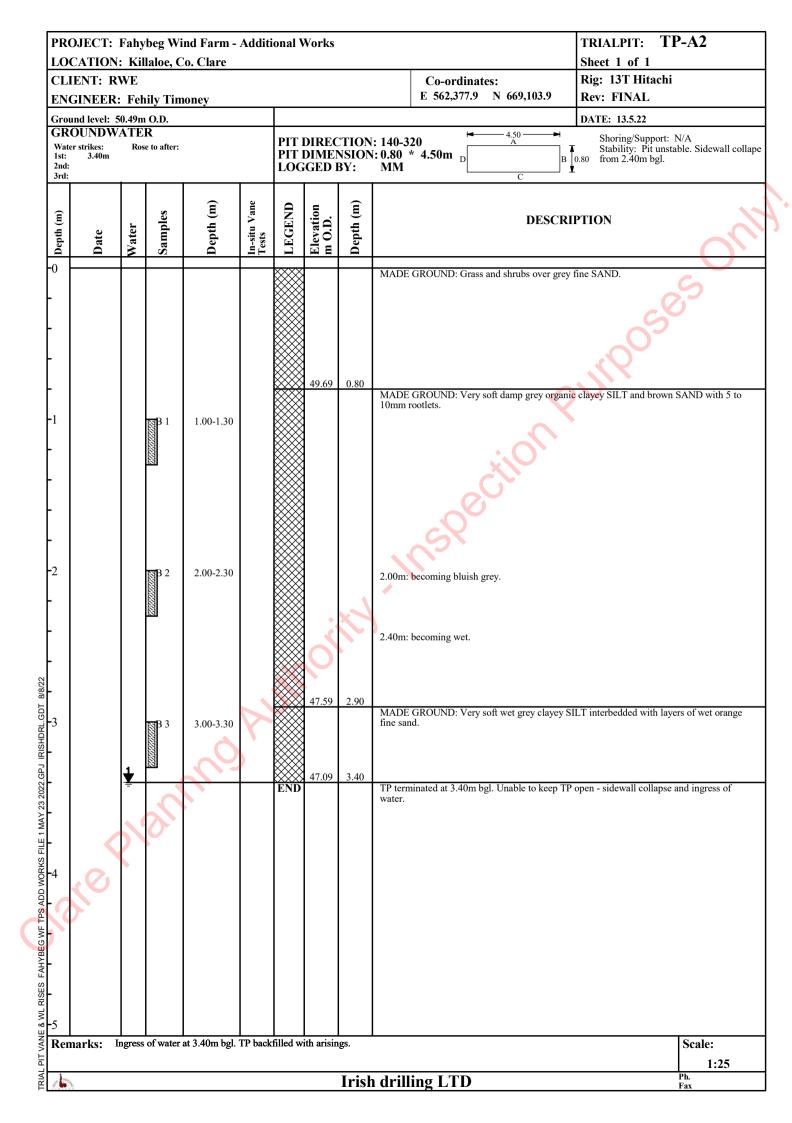




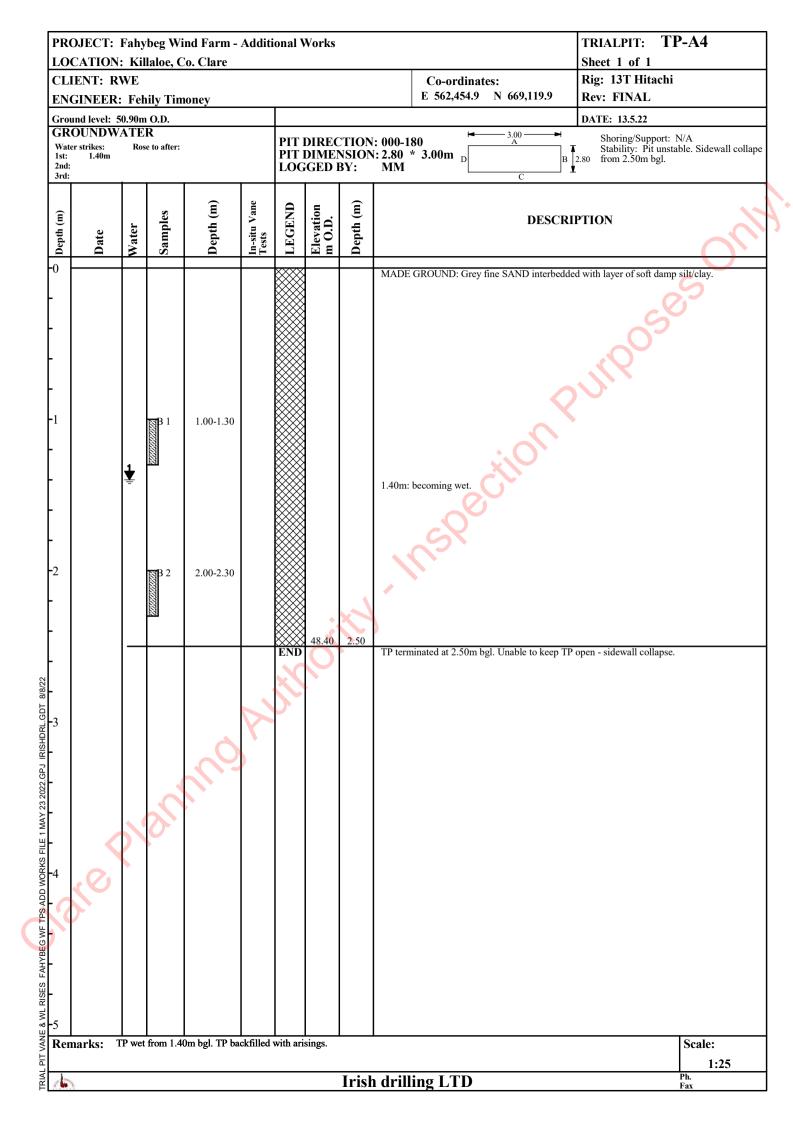
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LOCATION: CLIENT: RV ENGINEER: Ground level: 5 GROUNDWA Water strikes: 1st: 4.10m 2nd: 3rd:	WE <u>Fehi</u> 0.35m ATEF	ily_Tim O.D.							Sheet 1 of 1
Ground level: 5 GROUNDWA Water strikes: 1st: 4.10m 2nd:	0.35m ATEF	O.D.	oney					Co-ordinates:	Rig: 13T Hitachi
GROUNDWA Water strikes: 1st: 4.10m 2nd:	ATEF							E 562,413.7 N 669,079.4	Rev: FINAL DATE: 13.5.22
	·	K to after:			PIT I PIT I LOG	DIREC DIMEI GED I	CTION NSION BY:		Shoring/Support: N/A Stability: Pit unstable. Sidewall collape 0.80 from 2.50m bgl.
Depth (m) Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)	DESCH	RIPTION
0						49.85	0.50	ADE GROUND: Grass and shrubs over fir	605
1		83 I	1.00-1.20					ADE GROUND: Very soft wet reddish bro	wn SILT/CLAY with rootlets.
2		32 300000	2.00-2.20			47.85	2.50	30m: becoming greyish brown. ADE GROUND: Very soft greyish brown S edium sand.	SILT/CLAY interbedded with layers of wet grey
3		B ³³	3.00-3.20						
4	↓	19 34	4.10-4.40		END	45.85	4.50	terminated at 4.50m bgl.	
5 Remarks: Ir	ngress	of water a	at 4.10m bgl. '	TP back	filled wi	th arisin	ıgs.		Scale: 1:25



CLIENT: RWE ENGINEER: Fehily TimoneyCo-ordinates: E 562,449.5 N 669,163.8Rig: 13T H Rev: FINA DATE: 13.5.7Ground level: 51.86m O.D. GROUNDWATER Water strikes: Rose to after:DATE: 13.5.7 PIT DIRECTION: 000-180 A Shoring/S Stability:Shoring/S Stability:	RIALPIT: TP-A5 eet 1 of 1	I			Vorks	onal V	Additi	nd Farm -	-	-		
Subscription Date: Date: Ground level: Rose to after: PIT DIRECTION: 000-180 Numer order: Rose to after: PIT DIRECTION: 000-180 Numer order: Rose to after: PIT DIRECTION: 000-180 Numer order: Stability: PIT DIRECTION: 000-180 Numer order: Stability: PIT DIRECTION: 000-180 Numer order: Stability: PIT DIRECTION: 000-180 Stability: Stability: PIT DIRECTION: 000-180 Stability: Stability: PIT DIRECTION: PIT DIRECTION: Stability: Stability: Stability: <t< th=""><th>g: 13T Hitachi</th><th>-</th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th>VE</th><th>ENT: RV</th><th>CLI</th></t<>	g: 13T Hitachi	-								VE	ENT: RV	CLI
GROUNDWATER Inter tridue and: Rose to after: PIT DIRECTION: 000-180 PIT DIMENSION: 1.90 * 3.30m Image: Stability: Image: Stability: Imag			E 562,449.5 N 669,163					oney				
0 B1 0.00-1.50 MADE GROUND: Brown fine SAND interbedded with layer of the sand interbedded with layer of solution interbedded withedded withedded with layer of solution interbedded with	Shoring/Support: N/A Stability: Pit unstable. Sidewall co		* 3.30m _D	DN: 1.90	DIMEN	PIT			R	ATEI	OUNDWA er strikes:	GRO Wate 1st: 2nd:
0 3 3 0.00-1.50 MADE GROUND: Brown fine SAND interbedded with layer of the sand state of the sand sta	ON	DESCRIP	DF		Elevation m O.D.	TEGEND	In-situ Vane Tests	Depth (m)	Samples	Water	Date	Depth (m)
- MADE GROUND: Damp grey fine SAND interbedded with lay brown clay.	ith layer of soft brown silt/clay.	on P'	ctio	0	50.36							- - - - - - -
			clay.	0 MADE		END	2	3.00-3.40	toppon			· · ·
									3		Xe X	4
-5 Remarks: TP dry on excavation. TP backfilled with arisings.	Scale:				ıgs.	th arisi	filled wi	ation. TP back	on excava	P dry o	narks: T	
	1:25											

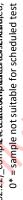
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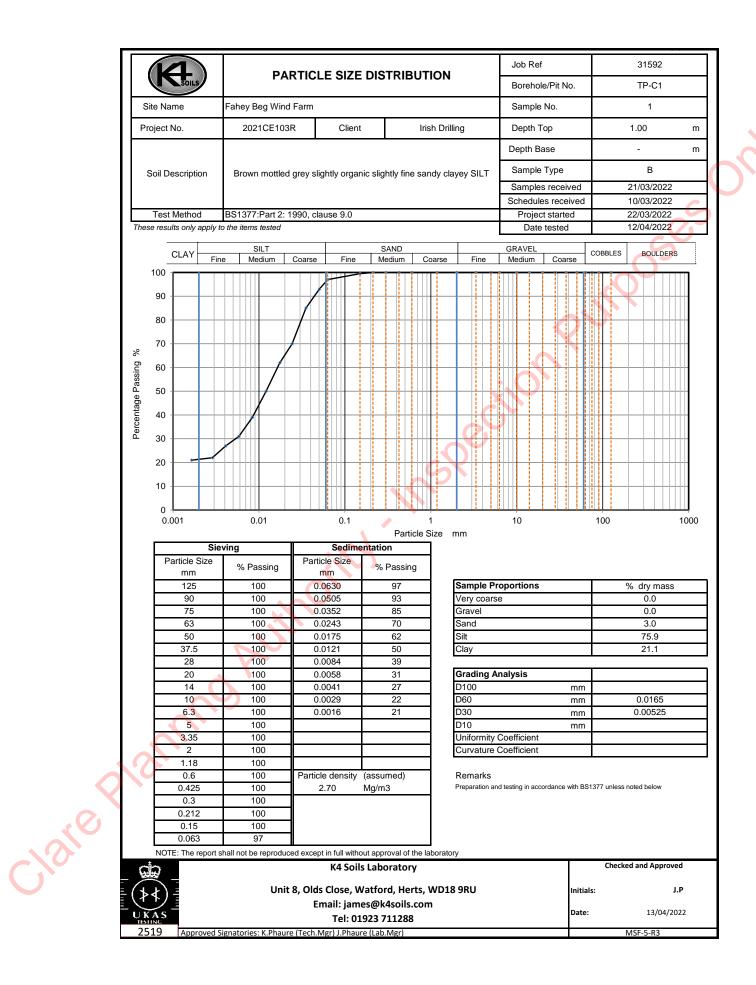
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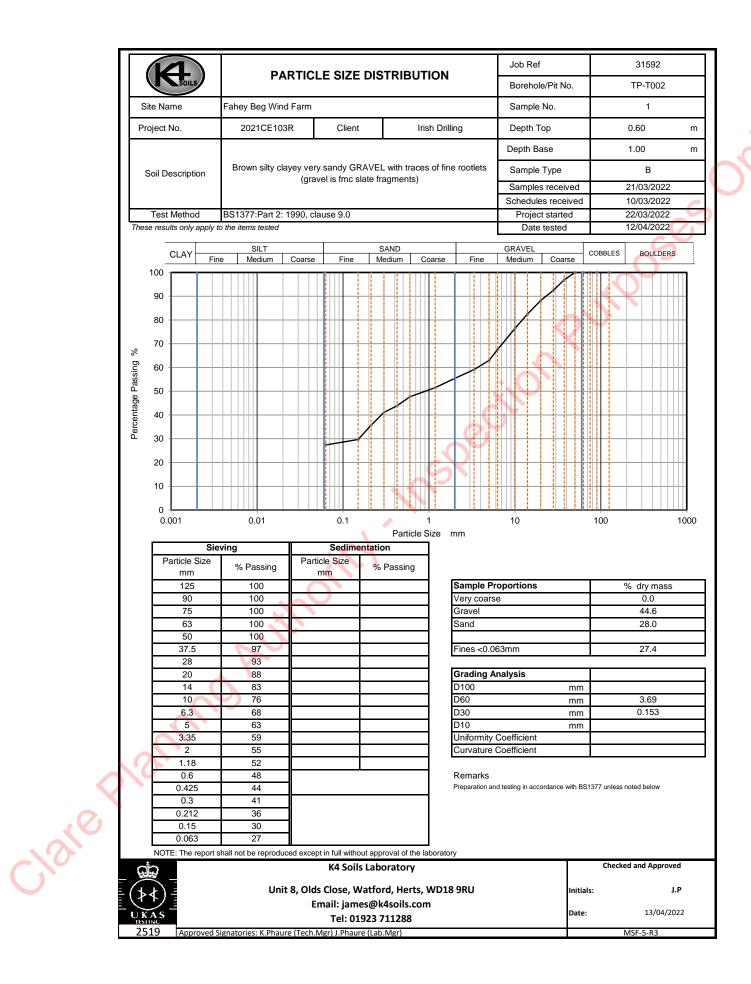


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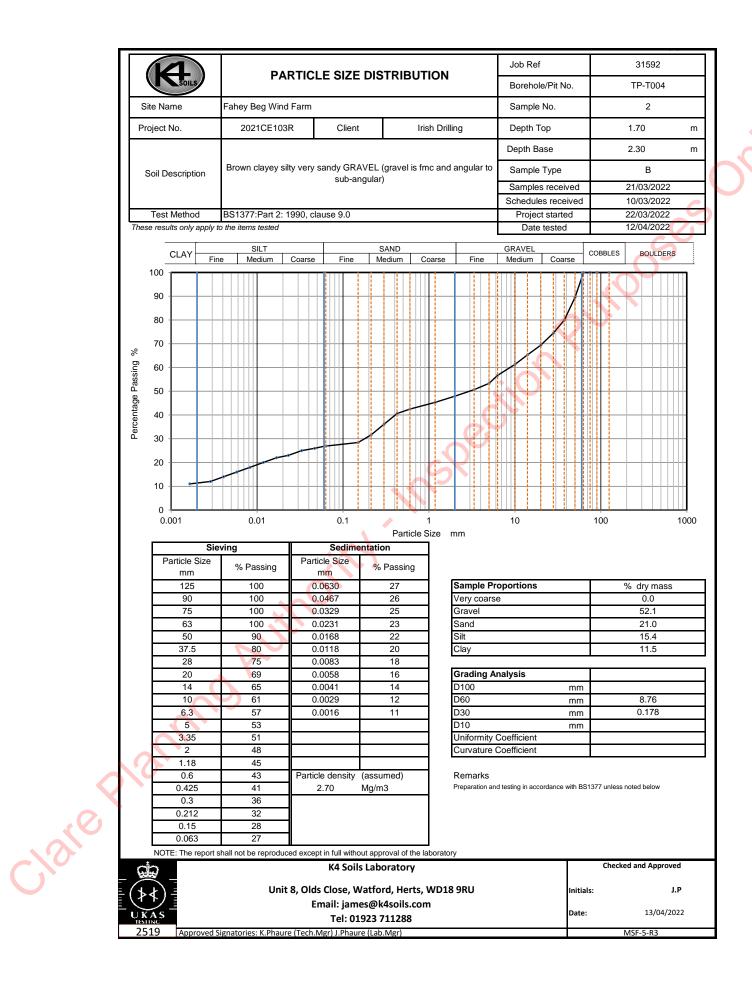
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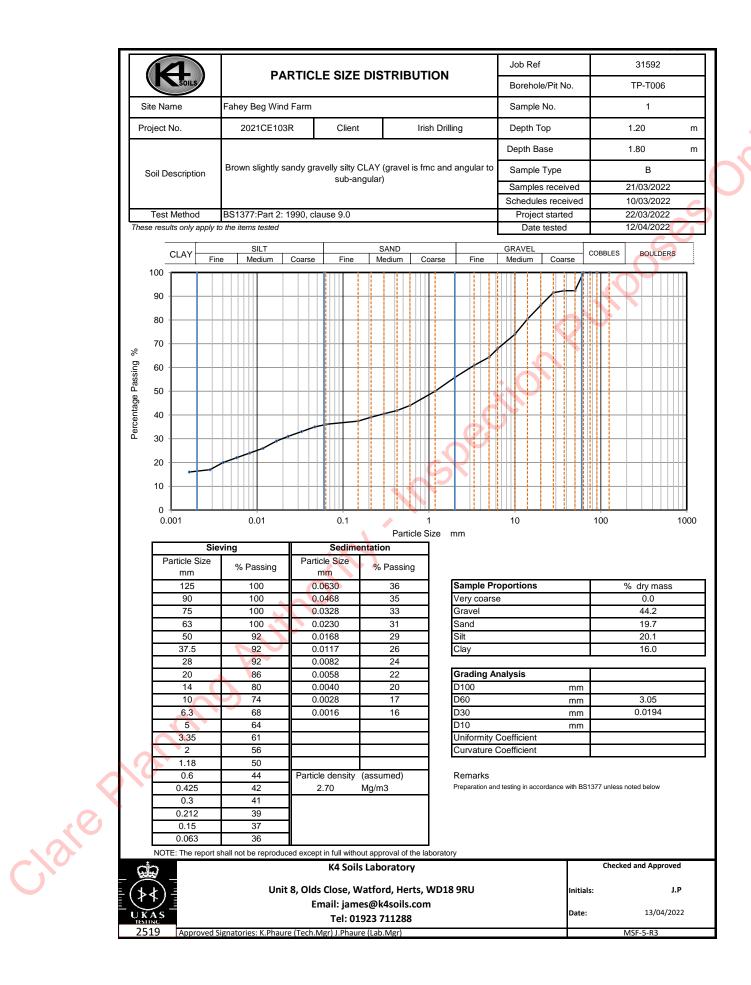
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2021CE 103R Inish Drilling Testing Started Hole No. Sample Soil Description NMC Pairing Started PL PI Remarks TP-C1 1 1.00 - B Brown mottled grey slightly organic slightly fire sandy clayey SLT 34 - - - NMC Only TP-C1 1 0.60 1.00 B Brown mottled grey slightly organic slightly fire sandy clayey SLT 34 - - - NMC Only TP-T002 1 0.60 1.00 B brown slightly sandy gravely slightly organic slate fragments) 34 - - - NMC Only TP-T002 1 1.20 1.80 B Brown slightly sandy gravely slightly CLAY (gravel is fire collets (gravel is fire. 14 - - - NMC Only TP-T008 1 1.20 1.80 B Brown slightly sandy gravely slightly fire - - NMC Only TP-T008 1 1.20 1.80 B Brown slightly sandy gravely slightly fire -					Beg W	ind Farm				Schedule	received	10/03/2022
Sample Parameter of the par												22/03/2022
Hole No. Ref Top Base Type Soil Description NMC Passing W LL PL PI Remats TP-C1 1 1.00 - B Brown motiled grey slightly organic slightly line sandy clayey SLT 34 - - - NMC Only TP-C1 1 0.60 1.00 B Brown motiled grey slightly organic slightly fine sandy clayey very sandy GRAVEL with traces of the rootets (gravelis fine slight regenetis) 14 - - - NMC Only TP-T002 1 0.60 1.00 B Brown slightly sandy gravely slip CLAY angular) 14 - - - NMC Only TP-T006 1 1.20 1.80 B Gravelis fine and angular to sub- angular) 13 - - - NMC Only TP-T006 1 1.20 1.8 I Image: I	2021	CE103	R	Irish Dri	illing					Testing S	tarted	
Reif Top Base Type Addition St. St. <th< th=""><th>Hole No</th><th></th><th>San</th><th>nple</th><th></th><th>Soil Description</th><th>NMC</th><th></th><th>LL</th><th>PL</th><th>PI</th><th>Pemarke</th></th<>	Hole No		San	nple		Soil Description	NMC		LL	PL	PI	Pemarke
TP-C1 1 1.00 · B Brown motiled grey slightly organic slightly organic slightly fine sandy clayey SiLT 34 · · · · NMC Only TP-T002 1 0.60 1.00 B Brown silty clayey very sandy GRAVEL state fragments) 14 · · · · NMC Only TP-T002 1 0.60 1.00 B Brown silty clayey very sandy GRAVEL state fragments) 14 · · · · NMC Only TP-T006 1 1.20 1.80 B Brown silty sandy gravely silty CLAY angular to sub-angular to sub-angular) 13 · · · · NMC Only TP-T006 1 1.20 1.80 B Brown silty fine sandy gravely silty CLAY angular to sub-angular to sub-angular) 13 · · · · NMC Only T 1 1 I I I I ·	Hole No.	Ref			Туре	Soli Description	9/		9/	9/	9/	Kellidiks
TP-T002 1 0.80 1.00 B with traces of fine rootiets (gravel is fmc later fragments) 14 - - - NMC Only TP-T006 1 1.20 1.80 B Brown slightly sandy gravelly slity CLAY gravells fmc and angular to sub-angular) 13 - - - NMC Only TP-T006 1 1.20 1.80 B Brown slightly sandy gravelly slity CLAY gravells fmc and angular to sub-angular) 13 - - - NMC Only TP-T006 1 1.20 1.80 B Brown slightly sandy gravelly slity CLAY gravells fmc and angular to sub-angular) 13 - - - NMC Only TP-T006 1 1.80 B <td>TP-C1</td> <td>1</td> <td></td> <td></td> <td>в</td> <td>Brown mottled grey slightly organic slightly fine sandy clayey SILT</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>NMC Only</td>	TP-C1	1			в	Brown mottled grey slightly organic slightly fine sandy clayey SILT						NMC Only
TP-T006 1 1.20 1.80 B Brown slightly sandy gravelly slity CLAY (graveli is fine and angular to sub-angular) 13 . . NMC Only TP-T006 1 1.20 1.80 B Graveli is fine and angular to sub-angular to sub-angular) 13 . . . NMC Only Image: Strength of the strengt of the strength of the strength of the strengend of the strengen	TP-T002	1	0.60	1.00	в	with traces of fine rootlets (gravel is fm		-	-		S.C	NMC Only
	TP-T006	1	1.20	1.80	В	(gravel is fmc and angular to sub-		-	~	-	-	NMC Only
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	â	Natural	Moisture	Content	: clause	e 3.2 Te	st Report by	K4 SOILS	LABOR	ATORY		Checked and Approved
Test Methods: BS1377: Part 2: 1990: Natural Moisture Content : clause 3.2 Checked and Test Report by K4 SOILS LABORATORY Checked and Approved	$\overline{\mathbf{A}}$.0	Unit 8 Olds	Close Old	ls Appro	ach		Initials J.P
Test Methods: BS1377: Part 2: 1990: Natural Moisture Content : clause 3.2 Test Report by K4 SOILS LABORATORY Checked and Approved Atterberg Limits: clause 4.3 and 5.0 Unit 8 Olds Close Olds Approach Initials J.P		NOTE:	The repo	t shall no	t be rep	produced except in full	Tel:	01923 71 [.]	1 288			Date: 13/04/202
Natural Moisture Content : clause 3.2 Atterberg Limits: clause 4.3 and 5.0 These results only apply to the items tested NOTE: The report shall not be repreduced event in full NOTE: The report shall not be repreduced event in full NOTE: The report shall not be repreduced event in full NOTE: The report shall not be repreduced event in full Note: 01923 711 288 Note: 01923 711 288 Note: 01923 711 288	UKAS TESTING 2519			of the lab		re (Tech.Mgr) J.Phaure (Lab.Mgr)	Email: Ja	mes@k4	soils.coi	n		MSF-5-R1(b)



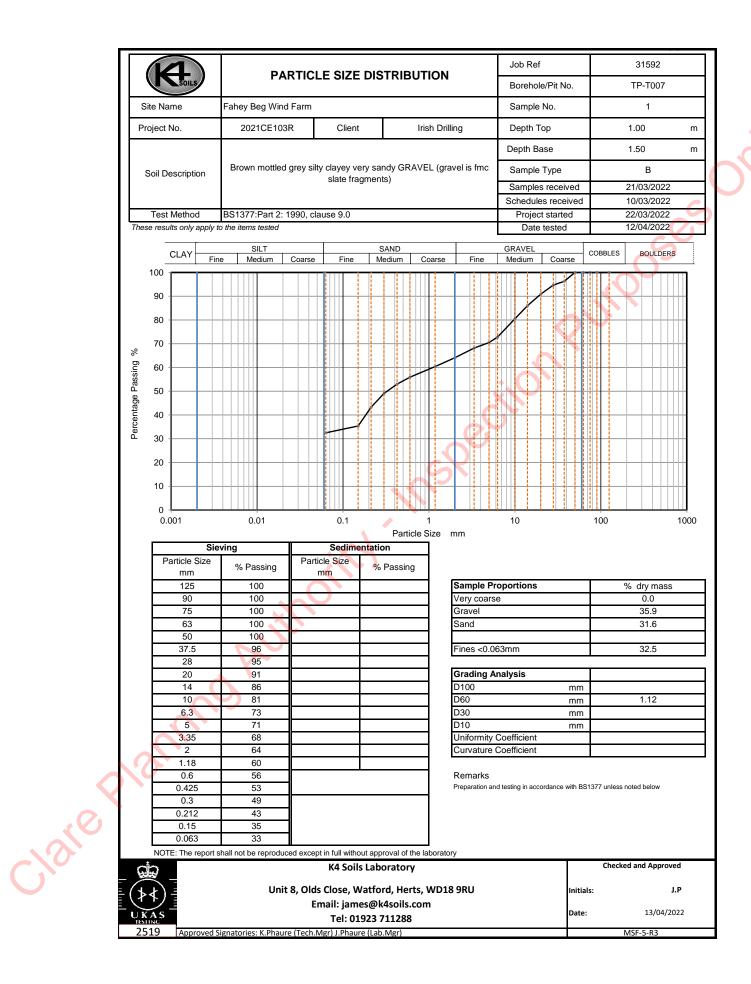


										Job Ref		31592	
	SOIL		PA	RTIC	LE SIZ	ZE DI	STRI	BUTIO	N	Borehole/Pit N	No.	TP-T003	
		-		_									
Si	ite Name		Fahey Beg Wind	Farm						Sample No.		1	
Pro	oject No.		2021CE103	R	Cli	ent		Irish D	rilling	Depth Top		1.60	m
										Depth Base		2.00	m
			Brown slightly me	ottled g	rey sligh	itly sand	dy grav	elly silty C	LAY (gravel is	Sample Type		В	
5	Soil Descript	ion	0,	fmc a	nd angu	ular to s	ub-rour	nded)	10	Samples recei		21/03/2022	,
										Schedules rece		10/03/2022	
	Test Metho	d	BS1377:Part 2: 1	990, cl	ause 9.0)				Project starte		22/03/2022	
Thes	e results only	apply t	o the items tested							Date tested	d	12/04/2022	
			SILT				SAND			GRAVEL	0	OBBLES BOULDER	
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$\langle Q \rangle$	1.18	3	60										
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25		iovod c	ignatorios K Phase										
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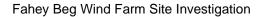
			DA	DTIC	LE SIZ		топ		NI		Job Re	f		31592	
	SOILS		F 7					SUTIC	'IN		Boreho	ole/Pit No.		TP-T006	
Site N	ame		Fahey Beg Win	d Farm							Sample	e No.		2	
Project	t No.		2021CE10	3R	Clie	nt		Irish I	Drilling		Depth	Тор		2.10	m
					-						Depth E	Base		2.70	m
Soil [Descriptio	'n	Brown silty clay						rootlets (gravel	Sample	е Туре		В	
	·			is fmo	c and angu	lar to s	sub-an	jular)				es received		21/03/2022	
Tes	t Method		BS1377:Part 2:	1990 c	lause 9.0							les received ct started	-	10/03/2022 22/03/2022	
			the items tested									e tested		12/04/2022	
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					K4 So	ils Lab	orato	ry					Che	cked and Approv	red
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				E	Email: jaı Tel· (nes@)1923						Date	:	13/04/2	022
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			DA	DTIC	LE SIZ		трі		M		Job Ref		31	592	
	SOILS	/	F <i>F</i>			E DIS		50110	N		Borehole/Pit N	No.	TP-	T008	
Sit	te Name	F	⁻ ahey Beg Win	d Farm							Sample No.			1	
Pro	oject No.		2021CE10	3R	Clie	nt			Depth Top		0.50		m		
											Depth Base		1.00		m
s	oil Descriptio	on	Brown silty cla		y very sandy GRAVEL with occasiona					ets	Sample Type			В	
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	Test Method		BS1377:Part 2: the items tested	1990, c	clause 9.0						Project start Date teste			3/2022 1/2022	-
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				I	Email: jai							Date:	:	13/04/2022	2
1511 251	NC 19 Appr	nved Sig	natories: K.Phau	re (Tech		01923 7		0					MSF-5	-R3	
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V	SO	ILS			Tested in accordance with BS1377 : Part 3 : 20		se 7.0 a	Clause		
Job No.			Project N					Samples r	Programme received 21/03/2022	
31592			Fahey B	Beg Wind	Farm			Samples r Schedule i		
Project No	o.		Client			Project started 22/03/202				
2021CE1	03R		Irish Drill	ling				Testing S	Started 11/04/2022	
		S	ample			Dry Mass		──		
Hole No.	Ref	Тор	Base	Туре	Soil description	passing 2mm	SO4 Content	рН	Remarks	
	\vdash	m	m	 		%	mg/l			
TP-T002	1	0.60	1.00	В	Brown silty clayey very sandy GRAVEL with traces of fine rootlets (gravel is fmc slate fragments)	55	320	7.85		
TP-T006	1	1.20	1.80	В	Brown slightly sandy gravelly silty CLAY (gravel is fmc and angular to sub-angular)	56	210	7.82		
TP-T007	1	1.00	1.50	В	Brown mottled grey silty clayey very sandy GRAVEL (gravel is fmc slate fragments)	64	270	7.83		
					SP					
					" NON.					
				5						
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0	3									
			<u> </u>	L	Test Report by K4 SOILS LABORATORY Unit 8 Olds Close Olds Approach Watford Herts WD18 9RU Tel: 01923 711 288 Email: James@k4soils.com These results only apply to the items tested NOTE: The report shall not be reproduced except in full without authority of the labor.		<u> </u>	<u> </u>	Checked and Approved Initials J.P Date: 13/04/202	

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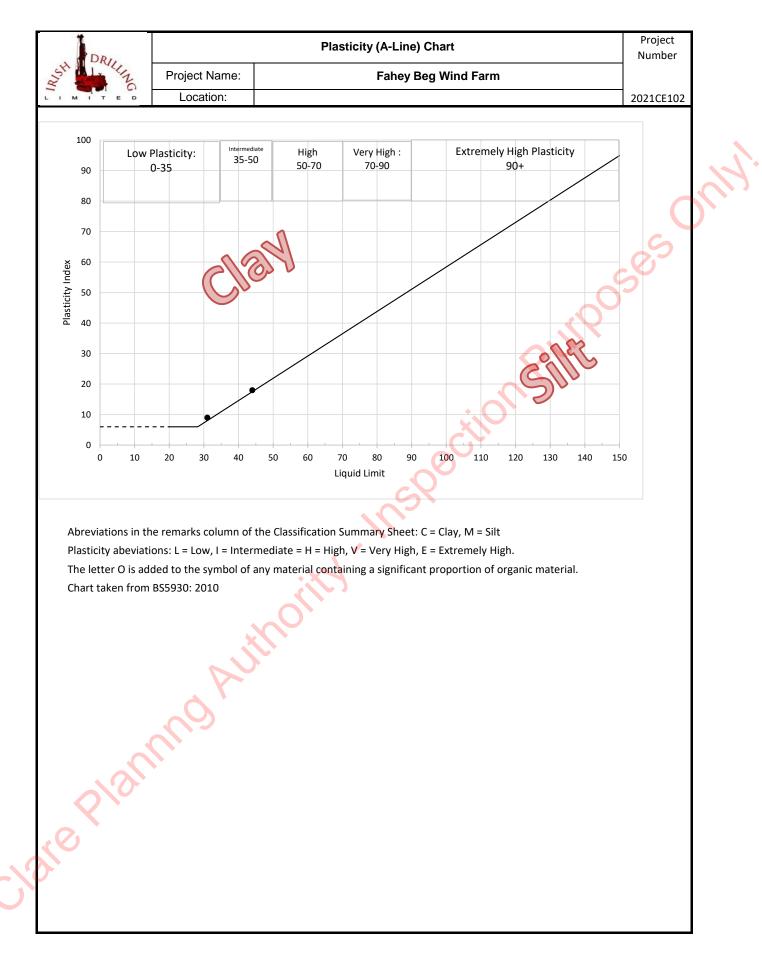
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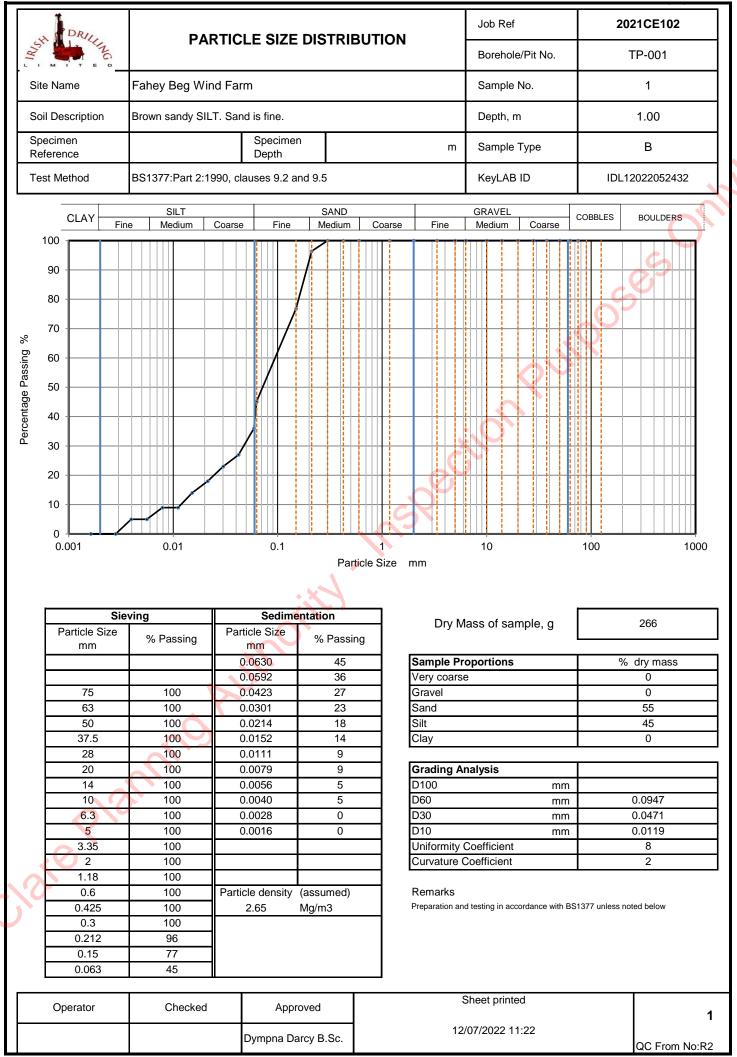
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roject No.			Project	Name											
2021	CE102				1				g Wind	Farm		1	-		
Hole No.	Ref	Sar Top	nple Base	Туре		Soil Description	Dens bulk	ity dry	W	Passing 425µm	LL	PL	PI	Particle density	Remarks
		100	Dusc	Type			Mg/n	n3	%	%	%	%	%	Mg/m3	
TP-001	1	1.00	1.30	В		Brown sandy SILT. Sand is fine.			25.0	100					
TP-002	2	2.00	2.30	В		Brown SAND.			37.0	100					NP
TP-003	2	2.00	2.20	В		Brown SILT			44.0	100	44	26	18		М
TP-003	4	4.10	4.40	в		Brown slightly sandy SILT. Sand is fine.			30.0	100	31	22	9		CL
TP-004	1	1.00	1.30	в		Brown very silty fine and medium SAND.			17.0	98					C)
TP-005	1	0.00	1.50	в		Brown slightly sandy SILT.			47.0	99				0	
													~	$\mathbf{\Sigma}$	
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									0)					
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l tests perf	ormed	in acco	rdance v	with BS	61377:	1990 unless specified othe	rwise								
						= Plastic Limit, PI = Plastic		x	Date F	Printed		Appr	oved	Ву	Table
Density	test	ment unles		Liquid I		Particle	e density nall pyknom			12/2022	00.00				1
		acement		1pt - si					017	, _ \ \ \ Z	30.00				sheet

Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39 Approved Signatures: Dympna Darcy (DCD) Lab Manager, Declan Joyce (DJ) Chartered Geotechnical Engineer, Ronan Killeen (RK) Quality Manager.

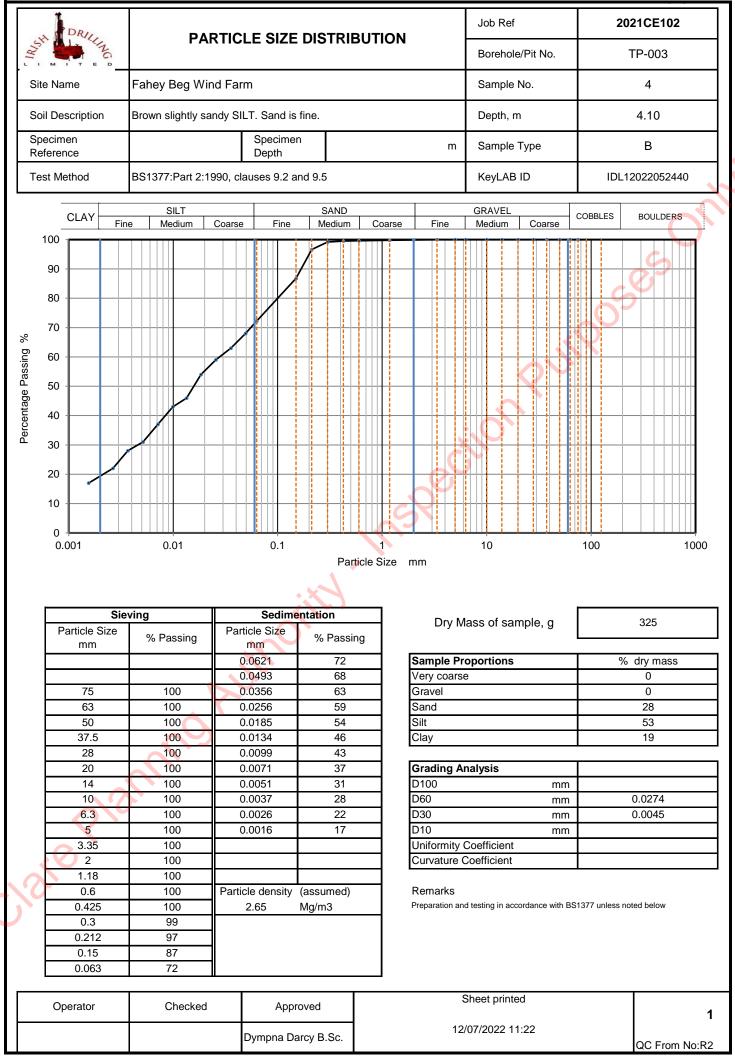


QC Form: R1



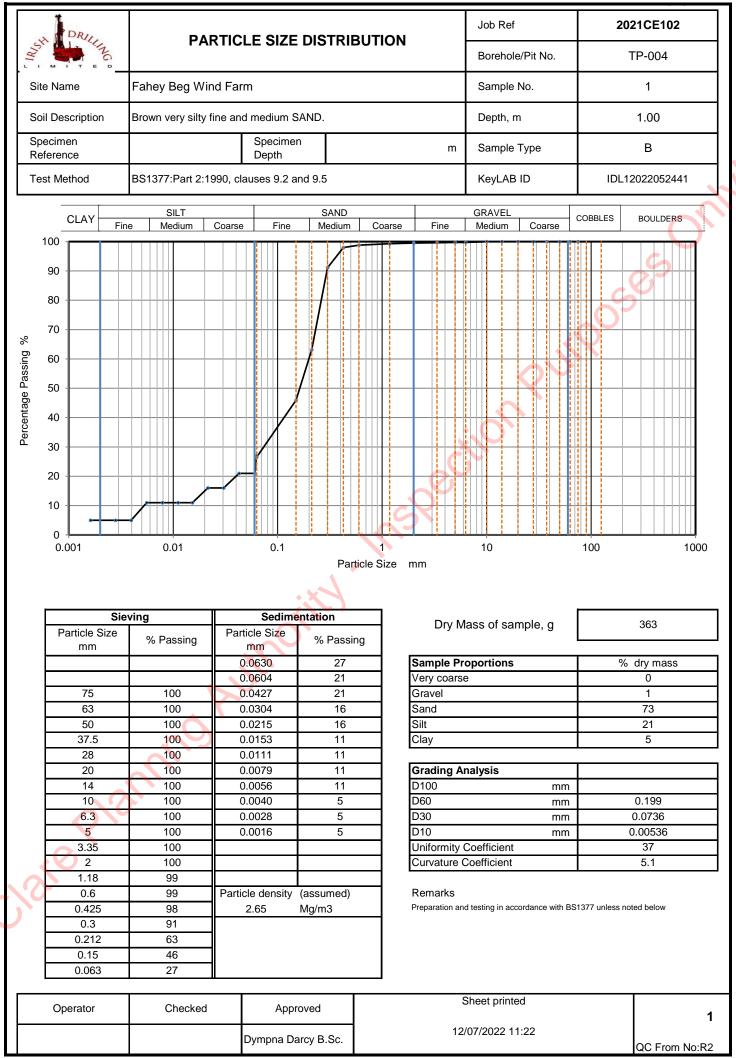
Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39

Approved Signatures: Dympna Darcy (DCD) Lab Manager, Declan Joyce (DJ) Chartered Geotechnical Engineer, Ronan Killeen (RK) Quality Manager.



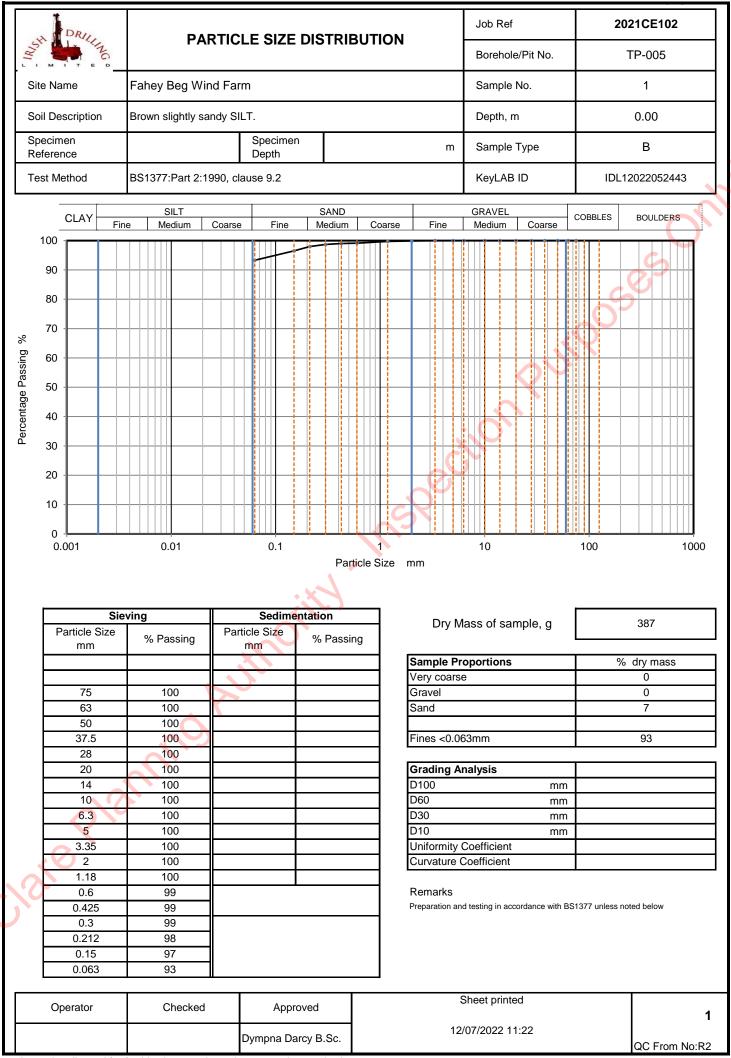
Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39

Approved Signatures: Dympna Darcy (DCD) Lab Manager, Declan Joyce (DJ) Chartered Geotechnical Engineer, Ronan Killeen (RK) Quality Manager.



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Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39

Jare Planno Authority. Inspection Purposes Only



Irish Drilling Limited Old Galway Road Loughrea Co. Galway

Attention: Dympna Darcy

Unit 7-8 Hawarden Business Park Manor Road (off Manor Lane) Hawarden Deeside CH5 3US Tel: (01244) 528777 email: hawardencustomerservices@alsglobal.com Website: www.alsenvironmental.co.uk

urposes

CERTIFICATE OF ANALYSIS

Date of report Generation: Customer: Sample Delivery Group (SDG): Your Reference: Location: Report No: Order Number: 13 July 2022 Irish Drilling Limited 220706-92 2021CE102 Faheybeg WF 654229 11407

We received 4 samples on Wednesday July 06, 2022 and 4 of these samples were scheduled for analysis which was completed on Wednesday July 13, 2022. Accredited laboratory tests are defined within the report, but opinions, interpretations and on-site data expressed herein are outside the scope of ISO 17025 accreditation.

Should this report require incorporation into client reports, it must be used in its entirety and not simply with the data sections alone.

Chemical testing (unless subcontracted) performed at ALS Life Sciences Ltd Hawarden.

maput

All sample data is provided by the customer. The reported results relate to the sample supplied, and on the basis that this data is correct.

Incorrect sampling dates and/or sample information will affect the validity of results. The customer is not permitted to reproduce this report except in full without the approval of the laboratory.

Approved By:

Sonia McWhan Operations Manager



ALS Life Sciences Limited. Registered Office: Units 7 & 8 Hawarden Business Park, Manor Road, Hawarden, Deeside, CH5 3US. Registered in England and Wales No. 4057291. Version: 3.3 Version Issued: 13/07/2022 Jare Planmo Authority. Inspection Punposes Only

CERTIFICATE OF ANALYSIS



SDG: 220706-92 Client Ref.: 2021CE102

Report Number: 654229 Location: Faheybeg WF Superseded Report:

Validated

Received Sample Overview

b Sample No(s)	Received S Customer Sample Ref.	AGS Ref.	Depth (m)	Sampled Date
26546501	TP001	B1	1.00 - 1.30	13/05/2022
26546507	TP002	B2	2.00 - 2.30	13/05/2022
26546511	TP003	B4	4.10 - 4.40	13/05/2022
26546514	TP004	B1	1.00 - 1.30	13/05/2022
<pre>/ received samples which</pre>	have had analysis scheduled will b	be shown on the following pa	ges.	
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CERTIFICATE OF ANALYSIS Report Number: 654229

Location: Faheybeg WF

SDG: 220706-92

Client Ref.: 2021CE102

Results Legend

Page 3 of 8

Validated

Superseded Report:

CERTIFICATE OF ANALYSIS

SDG: 220706-92 Client Ref.: 2021CE102 Report Number: 654229 Location: Faheybeg WF Superseded Report:

Validated

Sample Descriptions

very fine <0.0	063mm fine 0.	063mm - 0.1mm	medium 0.1m	m - 2mm coa	arse 2mm - 1	0mm very coars	e >10m
Lab Sample No(s)	Customer Sample Ref.	Depth (m)	Colour	Description	Inclusions	Inclusions 2	
26546501	TP001	1.00 - 1.30	Light Brown	Loamy Sand	Vegetation	None	
26546507	TP002	2.00 - 2.30	Light Brown	Silty Clay	None	None	
26546511	TP003	4.10 - 4.40	Light Brown	Silt Loam	None	None	4
26546514	TP004	1.00 - 1.30	Light Brown	Sand	Stones	None	

These descriptions are only intended to act as a cross check if sample identities are questioned, and to provide a log of sample matrices with respect to MCERTS validation. They are not intended as full geological descriptions.

We are accredited to MCERTS for sand, clay and loam/topsoil, or any of these materials - whether these are derived from naturally ocurring soil profiles, or from fill/made ground, as long as these materials constitute the major part of the sample.

Other coarse granular materials such as concrete, gravel and brick are not accredited if they comprise the major part of the

CERTIFICATE OF ANALYSIS

Validated

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				FICATE OF A							
	SDG: 220706- ent Ref.: 2021CE1			Report Number: 6 Location: F	654229 Faheybeg WF	Superseded Report:					
Results Legend # ISO17025 accredited.	C	ustomer Sample Ref.	TP001	TP002	TP003	TP004					
M mCERTS accredited. aq Aqueous / settled sample. diss.filt Dissolved / filtered sample. tot.unfilt Total / unfiltered sample. * Subcontracted - refer to subcontractor	- report for	Depth (m) Sample Type	1.00 - 1.30 Soil/Solid (S)	2.00 - 2.30 Soil/Solid (S)	4.10 - 4.40 Soil/Solid (S)	1.00 - 1.30 Soil/Solid (S)					
accreditation status. ** % recovery of the surrogate standard to efficiency of the method. The results of	to check the f individual	Date Sampled Sample Time Date Received	13/05/2022 06/07/2022	13/05/2022 06/07/2022	13/05/2022 06/07/2022	13/05/2022 06/07/2022					
compounds within samples aren't corre recovery	ected for the	SDG Ref Lab Sample No.(s)	220706-92 26546501	220706-92 26546507	220706-92 26546511	220706-92 26546514					
(F) Trigger breach confirmed 1-4+§@ Sample deviation (see appendix)		AGS Reference	B1	B2	B4	B1					
omponent	LOD/Units	Method									
isture Content Ratio (% of as eived sample)	%	PM024	19	22	23	15					
Organic Matter (SOM)	<0.35 %	TM132		0.795 @#							
	1 pH Units	TM133	8.65 @ M	7.76 @ M	8.41 @ M	8.3 @ M					
ter Soluble Sulphate as SO4 2:1 ract	<0.004 g/l	TM243	0.01 @ M	0.0279 @ M	0.0141 @ M	0.0069 @ M	C	2			
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CERTIFICATE OF ANALYSIS

Report Number: 654229 Location: Faheybeg WF Superseded Report:

Table of Results - Appendix

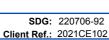
Method No PM024 TM132 TM133 TM243 not applicable. cal testing (unless	Reference Modified BS 1377 In - house Method BS 1377: Part 3 1990;BS 6068-2.5	Description Soil preparation including homogenisation, moisture screens of soils for Asbestos Containing Material ELTRA CS800 Operators Guide Determination of pH in Soil and Water using the GLpH pH Meter
TM132 TM133 TM243 not applicable.	In - house Method	Material ELTRA CS800 Operators Guide
TM133 TM243 not applicable.		
TM243 not applicable.	55 Torrin and 1990,05 0000-2.0	Potentination of DELIT COLLARG WALL USING THE GEDT DE WEEK
not applicable.		Mixed Anions In Soils By Kone
	subcontracted) performed at ALS Life Sciences Ltd Hawa	
	Plannohutn	stite
Jare		

SDG: 220706-92

Client Ref.: 2021CE102

CERTIFICATE OF ANALYSIS Report Number: 654229

Superseded Report:



Test Completion Dates

	3: 220706-9 :: 2021CE10				Number: 654 .ocation: Fat	
			Tes	st Com	pletion	Dates
Lab Sam	ple No(s)	26546501	26546507	26546511	26546514	
Customer Sar	mple Ref.	TP001	TP002	TP003	TP004	
	AGS Ref.	B1	B2	B4	B1	
-	Depth	1.00 - 1.30	2.00 - 2.30	4.10 - 4.40	1.00 - 1.30	
	Туре	Soil/Solid (S)	Soil/Solid (S)	Soil/Solid (S)	Soil/Solid (S)	
ons by Kone (soil)		13-Jul-2022 11-Jul-2022	12-Jul-2022 11-Jul-2022	13-Jul-2022 08-Jul-2022	13-Jul-2022 11-Jul-2022	
mple description		07-Jul-2022	07-Jul-2022	07-Jul-2022	07-Jul-2022	
al Organic Carbon			13-Jul-2022			
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						purposes only
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CERTIFICATE OF ANALYSIS



220706-92 2021CE102 Report Number: 654229 Location: Faheybeg WF Superseded Report:

Appendix

General

1. Results are expressed on a dry weight basis (dried at 35°C) for all soil analyses except for the following: NRA and CEN Leach tests, flash point LOI, pH, ammonium as NH4 by the BRE method, VOC TICs and SVOC TICs.

2. If sufficient sample is received a sub sample will be retained free of charge for 30 days after analysis is completed (e-mailed) for all sample types unless the sample is destroyed on testing. The prepared soil sub sample that is analysed for asbestos will be retained for a period of 6 months after the analysis date. All bulk samples will be retained for a period of 6 months after the analysis date. All samples received and not scheduled will be disposed of one month after the date of receipt unless we are instructed to the contrary. Once the initial period has expired, a storage charge will be applied for each month or part thereof until the client cancels the request for sample storage. ALS reserve the right to charge for samples received and stored but not analysed.

3. With respect to turnaround, we will always endeavour to meet client requirements wherever possible, but turnaround times cannot be absolutely guaranteed due to so many variables beyond our control.

4. We take responsibility for any test performed by sub-contractors (marked with an asterisk). We endeavour to use UKAS/MCERTS Accredited Laboratories, who either complete a quality questionnaire or are audited by ourselves. For some determinands there are no UKAS/MCERTS Accredited Laboratories, in this instance a laboratory with a known track record will be utilised.

5. If no separate volatile sample is supplied by the client, or if a headspace or sediment is present in the volatile sample, the integrity of the data may be compromised. This will be flagged up as an invalid VOC on the test schedule and the result marked as deviating on the test certificate.

6. NDP - No determination possible due to insufficient/unsuitable sample.

7. Results relate only to the items tested.

8. LoDs (Limit of Detection) for wet tests reported on a dry weight basis are not corrected for moisture content.

9. **Surrogate recoveries** - Surrogates are added to your sample to monitor recovery of the test requested. A % recovery is reported, results are not corrected for the recovery measured. Typical recoveries for organics tests are 70-130%. Recoveries in soils are affected by organic rich or clay rich matrices. Waters can be affected by remediation fluids or high amounts of sediment. Test results are only ever reported if all of the associated quality checks pass; it is assumed that all recoveries outside of the values above are due to matrix affect.

10. Stones/debris are not routinely removed. We always endeavour to take a representative sub sample from the received sample.

11. In certain circumstances the method detection limit may be elevated due to the sample being outside the calibration range. Other factors that may contribute to this include possible interferences. In both cases the sample would be diluted which would cause the method detection limit to be raised.

12. For dried and crushed preparations of soils volatile loss may occur e.g volatile mercury.

13. For leachate preparations other than Zero Headspace Extraction (ZHE) volatile loss may occur.

14. For the BSEN 12457-3 two batch process to allow the cumulative release to be calculated, the volume of the leachate produced is measured and filtered for all tests. We therefore cannot carry out any unfiltered analysis. The tests affected include volatiles GCFID/GCMS and all subcontracted analysis.

15. Analysis and identification of specific compounds using GCFID is by retention time only, and we routinely calibrate and quantify for benzene, toluene, ethylbenzenes and xylenes (BTEX). For total volatiles in the C5-C12 range, the total area of the chromatogram is integrated and expressed as ug/kg or ug/l. Although this analysis is commonly used for the quantification of gasoline range organics (GRO), the system will also detect other compounds such as chlorinated solvents, and this may lead to a falsely high result with respect to hydrocarbons only. It is not possible to specifically identify these non-hydrocarbons, as standards are not routinely run for any other compounds, and for more definitive identification, volatiles by GCMS should be utilised.

16. We are accredited to MCERTS for sand, clay and loam/topsoil, or any of these materials - whether these are derived from naturally occurring soil profiles, or from fill/made ground, as long as these materials constitute the major part of the sample. Other coarse granular material such as concrete, gravel and brick are not accredited if they comprise the major part of the sample.

17 Data retention. All records, communications and reports pertaining to the analysis are archived for seven years from the date of issue of the final report.

18. Tentatively Identified Compounds (TICs) are non-target peaks in VOC and SVOC analysis. All non-target peaks detected with a concentration above the LoD are subjected to a mass spectral library search. Non-target peaks with a library search confidence of >75% are reported based on the best mass spectral library match. When a non-target peak with a library search confidence of <75% is detected it is reported as "mixed hydrocarbons". Non-target compounds identified from the scan data are semi-quantified relative to one of the deuterated internal standards, under the same chromatographic conditions as the target compounds. This result is reported as a semi-quantitative value and reported as Tentatively Identified Compounds (TICs). TICs are outside the scope of UKAS accreditation and are not moisture corrected.</p>

19. Sample Deviations

If a sample is classed as deviated then the associated results may be compromised.

	Container with Headspace provided for volatiles analysis
2	Incorrect container received
3	Deviation from method
4	Matrix interference
•	Sample holding time exceeded in laboratory
(\mathbf{a})	Sample holding time exceeded due to late arrival of instructions or samples
§	Sampled on date not provided

20. Asbestos

When requested, the individual sub sample scheduled will be analysed in house for the presence of asbestos fibres and asbestos containing material by our documented in house method TM048 based on HSG 248 (2021), which is accredited to ISO17025. If a specific asbestos fibre type is not found this will be reported as "Not detected". If no asbestos fibre types are found all will be reported as "Not detected" and the sub sample analysed deemed to be clear of asbestos. If an asbestos fibre type is found it will be reported as detected (for each fibre type found). Testing can be carried out on asbestos positive samples, but, due to Health and Safety considerations, may be replaced by alternative tests or reported as No Determination Possible (NDP). The quantity of asbestos present is not determined unless specifically requested.

Identification of Asbestos in Bulk Materials & Soils

The results for identification of asbestos in bulk materials and soils are obtained from supplied bulk materials and soils which have been examined to determine the presence of asbestos fibres using ALS (Hawarden) in-house method of transmitted/polarised light microscopy and central stop dispersion staining, based on HSG 248 (2021).

The results for identification of asbestos in soils are obtained from a homogenised sub sample which has been examined to determine the presence of asbestos fibres using ALS (Hawarden) in-house method of transmitted/polarised light microscopy and central stop dispersion staining.

Asbe stos Type	Common Name
Chrysof le	WhiteAsbestos
Amosite	Brow n Asbestos
Cio d dolite	Blue Asbe stos
Fibrous Act nolite	-
Fibious Anthophyllite	-
Fibrous Tremolite	-

Visual Estimation Of Fibre Content

Estimation of fibre content is not permitted as part of our UKAS accredited test other than: - Trace - Where only one or two asbestos fibres were identified.

Respirable Fibres

Respirable fibres are defined as fibres of <3 μ m diameter, longer than 5 μ m and with aspect ratios of at least 3:1 that can be inhaled into the lower regions of the lung and are generally acknowledged to be most important predictor of hazard and risk for cancers of the lung.

Further guidance on typical asbestos fibre content of manufactured products can be found in HSG 264.

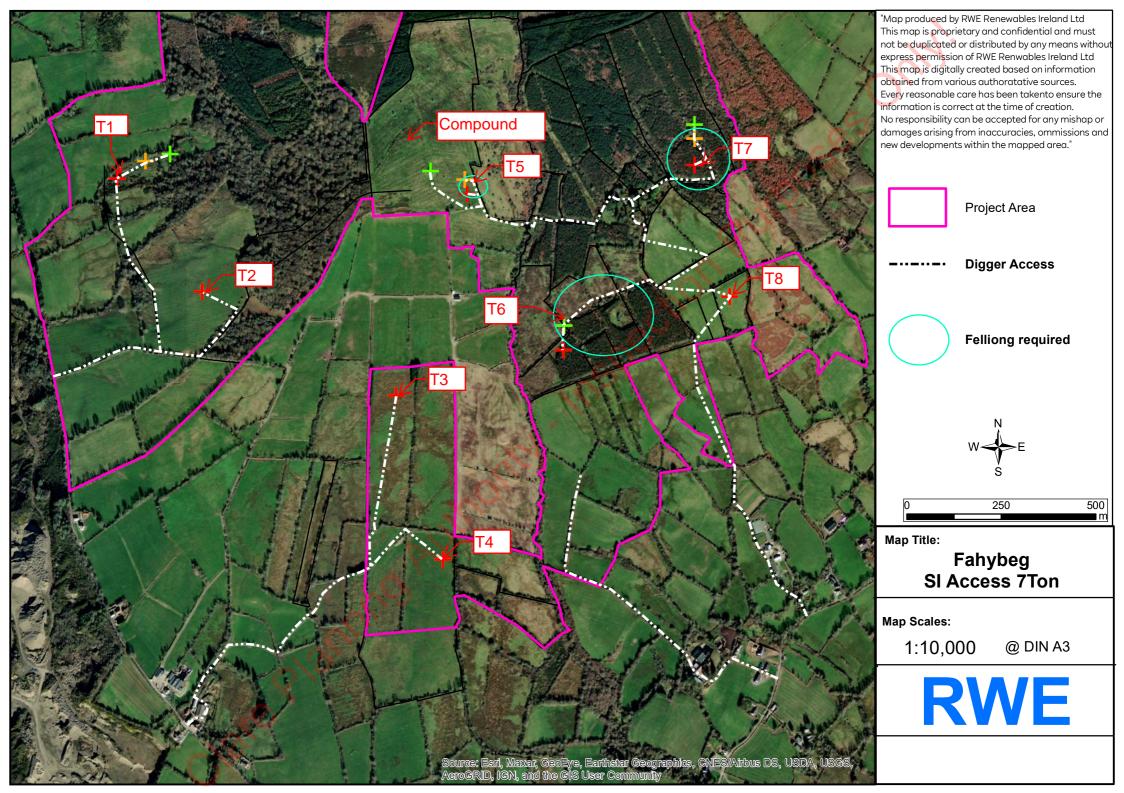
The identification of asbestos containing materials and soils falls within our schedule of tests for which we hold UKAS accreditation, however opinions, interpretations and all other information contained in the report are outside the scope of UKAS accreditation.

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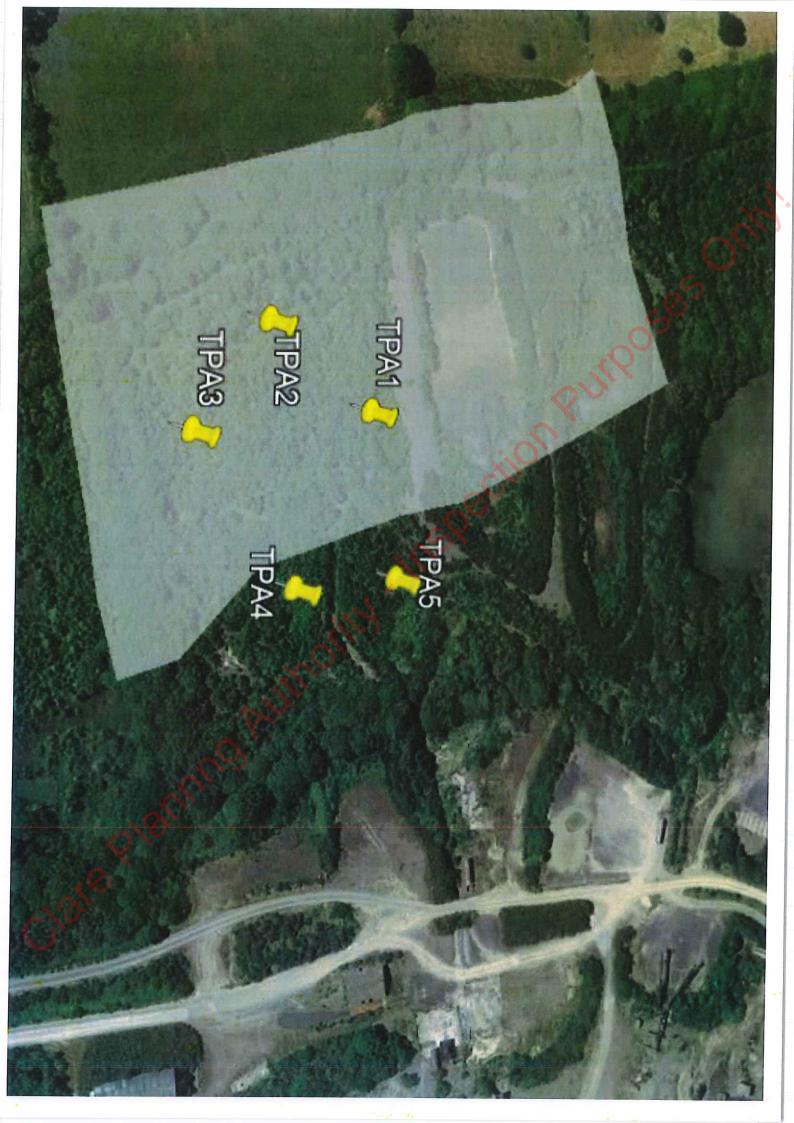


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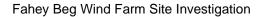
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Figure 1 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-1 (1).JPG



Figure 2 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-1 (2).JPG



Figure 3 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-2 (1).JPG



Figure 4 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-2 (2).JPG



Figure 5 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-3 (1).JPG



Figure 6 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-3 (2).JPG

Irish Drilling Ltd: Trial Pit Photos:



Figure 7 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-4 (1).JPG



Figure 10 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-5 (2).JPG



Figure 8 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-4 (2).JPG

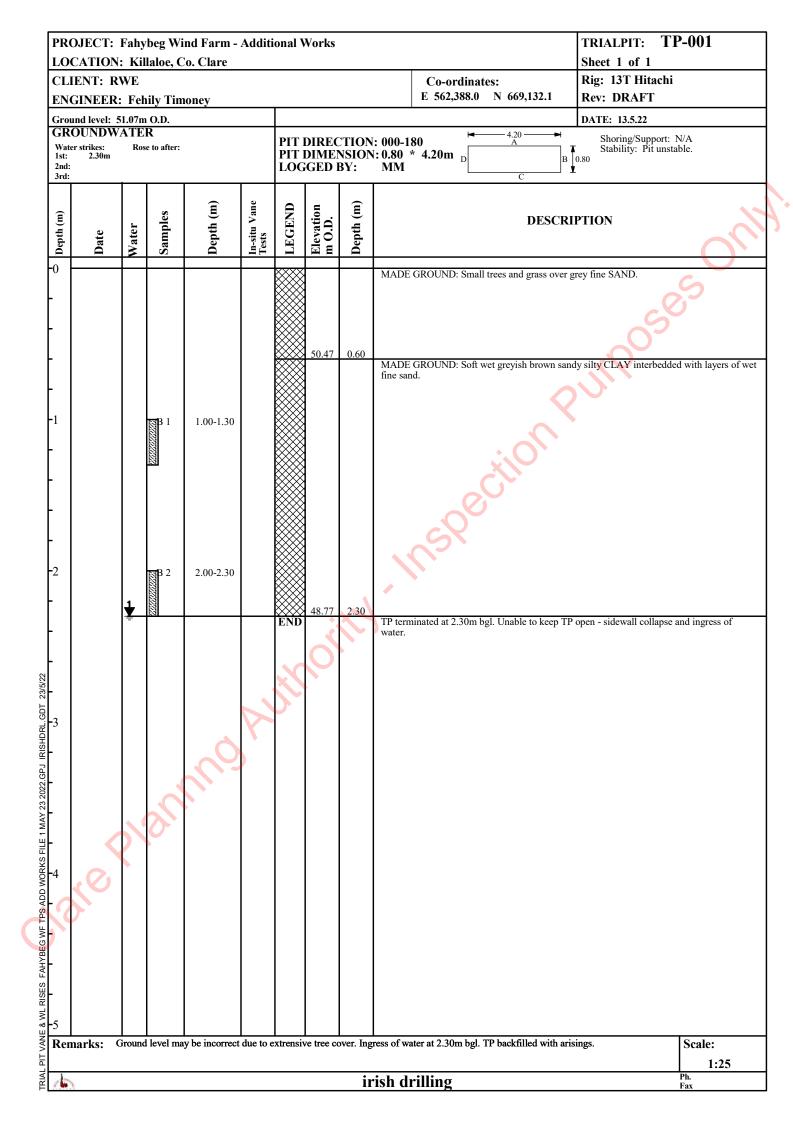


Figure 9 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-5 (1).JPG



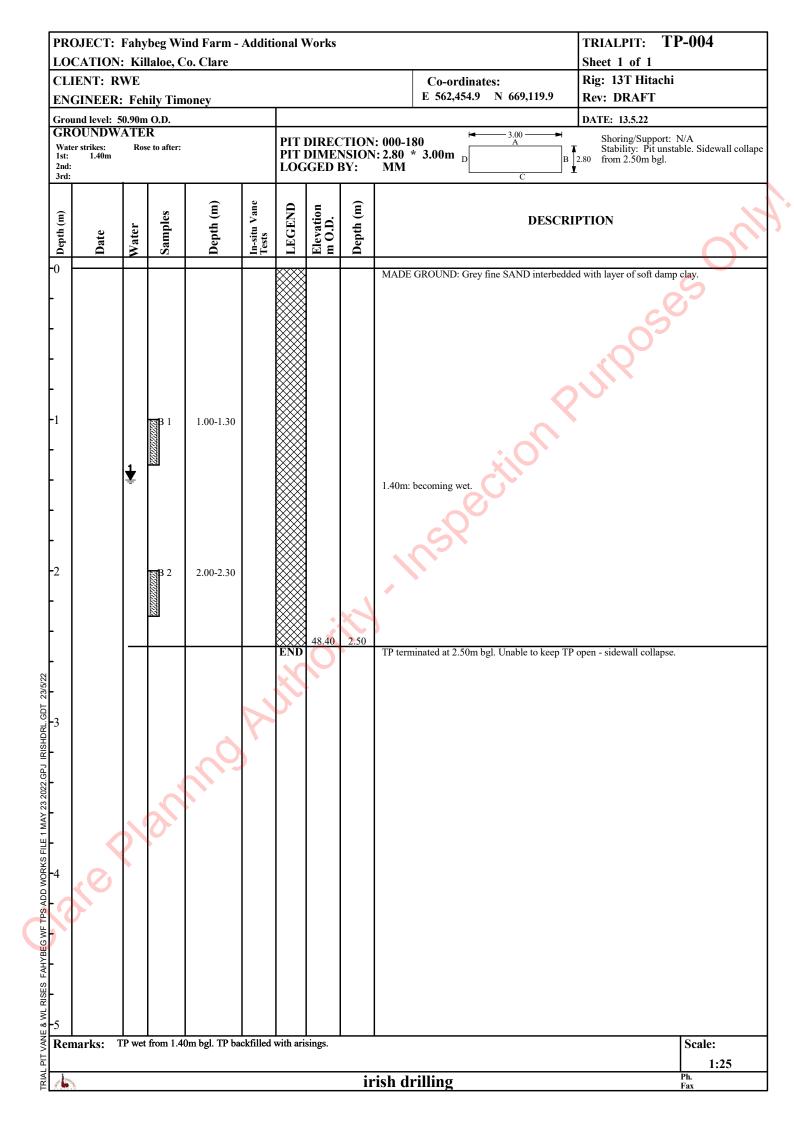
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		-	-	nd Farm - o. Clare	Additi	onal V	Vorks				TRIALPIT: T Sheet 1 of 1	P-002			
CLIF	ENT: RV	WE								Co-ordinates:	Rig: 13T Hitachi	i			
	INEER:			oney					E 562,377.9 N 669,103.9 Rev: DRAFT DATE: 13.5.22						
GRO	nd level: 5 DUNDWA strikes: 3.40m	ATE				PIT I	DIREC DIME GED 1	NSION	: 140-32 : 0.80 MM	$\begin{array}{c} 20 \\ \mathbf{4.50m} \\ \mathbf{D} \\ \mathbf{C} \end{array}$		N/A table. Sidewall collape			
Depth (m)	Date Water Samples Depth (m) LEGEND LEGEND Elevation m O.D. Depth (m)					Depth (m)		DESC	RIPTION	OUI					
0							49.69	0.80		GROUND: Grass and shrubs over g GROUND: Very soft damp grey or	rpos				
1			B 1	1.00-1.30					MADE	GROUND: Very soft damp grey org	anic clayey SIL1 with 5 to	lumm rootlets.			
2			832 888 888 888 888 888 888 888 888 888	2.00-2.30				N.		becoming bluish grey.					
3		1	в 3 Ганалагана Ганалагана	3.00-3.30	2		47.59 47.09	2.90	MADE fine san	GROUND: Very soft wet grey claye d.	y SILT interbedded with lay	vers of wet orange			
3		¥				END	<u>, , , , , , , , , , , , , , , , , , , </u>	3.40	TP term water.	inated at 3.40m bgl. Unable to keep	TP open - sidewall collapse	and ingress of			
	arks: Ir	Igress	of water	at 3.40m bgl. '	TP back	filled wi	ith arisir	ngs.				Scale:			
				•								1			

PROJECT: LOCATIO	-	-		Additi	onal V	Vorks			TRIALPIT: TP-003 Sheet 1 of 1
CLIENT: 1	RWE							Co-ordinates:	Rig: 13T Hitachi
ENGINEE			oney					E 562,413.7 N 669,079	9.4 Rev: DRAFT DATE: 13.5.22
GROUNDV GROUNDV Water strikes: 1st: 4.10m 2nd: 3rd:	WATE Ros				PIT I PIT I LOG	DIREC DIME GED	CTION NSION BY:	320-140 :0.80 * 4.20m MM □ C	DATE: 13.5.22 → Shoring/Support: N/A T Stability: Pit unstable. Sidewall collape B 0.80 from 2.50m bgl.
Depth (m) Date								DI	ESCRIPTION
)						49.85	0.50	MADE GROUND: Grass and shrubs ov MADE GROUND: Very soft wet reddie	Ses
1		а <mark>в</mark> 1	1.00-1.20					ction	RUIR
2		<mark>ав</mark> 2	2.00-2.20			47.85	2.50	1.80m: becoming greyish brown. MADE GROUND: Very soft greyish br medium sand.	rown CLAY interbedded with layers of wet grey
3			3.00-3.20						
3	₽		4.10-4.40		END	45.85	4.50	TP terminated at 4.50m bgl.	
5 Remarks:	Ingress	s of water	at 4.10m bgl. '	TP back	filled wi	ith arisir	ngs.		Scale:
							-	ish drilling	1:25 Ph. Fax

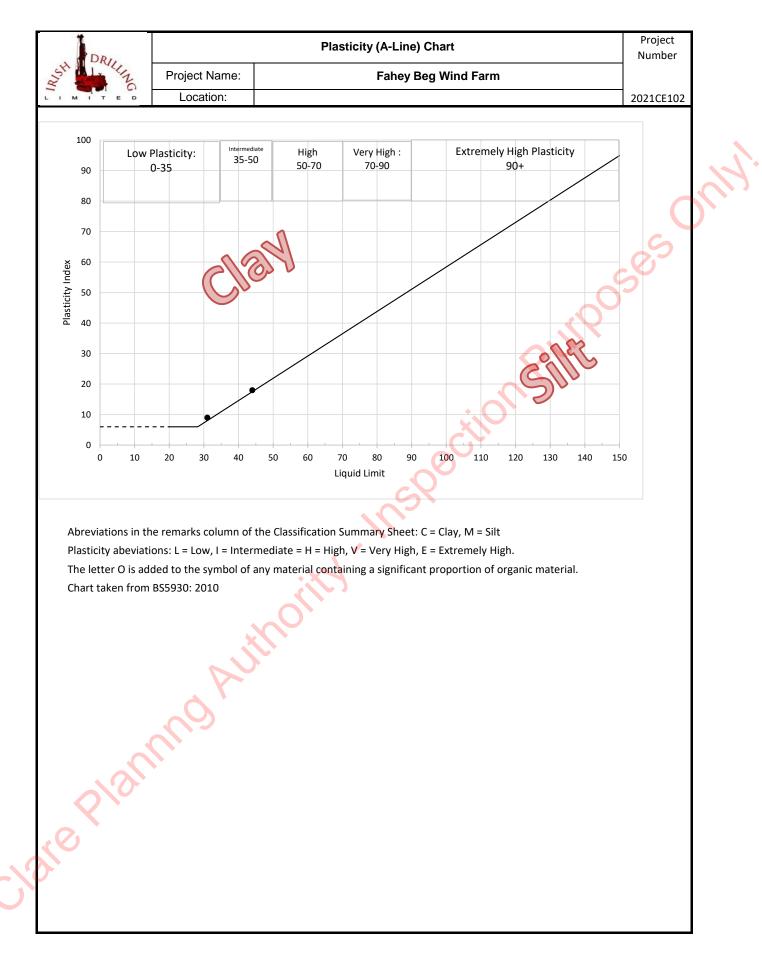


CLIENT: F	v. ixiii a	alaa C	o. Clare			Vorks				TRIALPIT: TP-005 Sheet 1 of 1				
CLIENT, F		, U							Co-ordinates: Rig: 13T Hitachi					
ENGINEER			oney						E 562,449.5 N 669,163.8	Rev: DRAFT				
Ground level: GROUNDW Water strikes: 1st: dry 2nd: 3rd:	VATER				PIT I PIT I LOG	DIREC DIME GED	CTION NSION BY:	: 000-1 : 1.90 MM		DATE: 13.5.22 Shoring/Support: N/A Stability: Pit unstable. Sidewall collape B 1.90 from 0.50m bgl.				
Depth (m) Date	Date Water Samples Depth (m) In-situ Vane Tests LEGEND Elevation m O.D. Depth (m)						Depth (m)		DESC	RIPTION				
0			0.00-1.50			50.36	1.50		E GROUND: Brown fine SAND interb	edded with layer of soft brown clay.				
2		αβ 2 2	2.00-2.30	<			W.	-	nspe					
3		B 3	3.00-3.40	6		48.86	3.00	MADI brown	E GROUND: Damp grey fine SAND ir clay.	nterbedded with layer of soft damp reddish				
					END	48.46	3.40	TP ter	minated at 3.40m bgl. Unable to keep '	TP open - sidewall collapse.				
	TP dry or	n excava	tion. TP back	filled wi	th arisin	ngs.			rilling	Scale: 1:25				

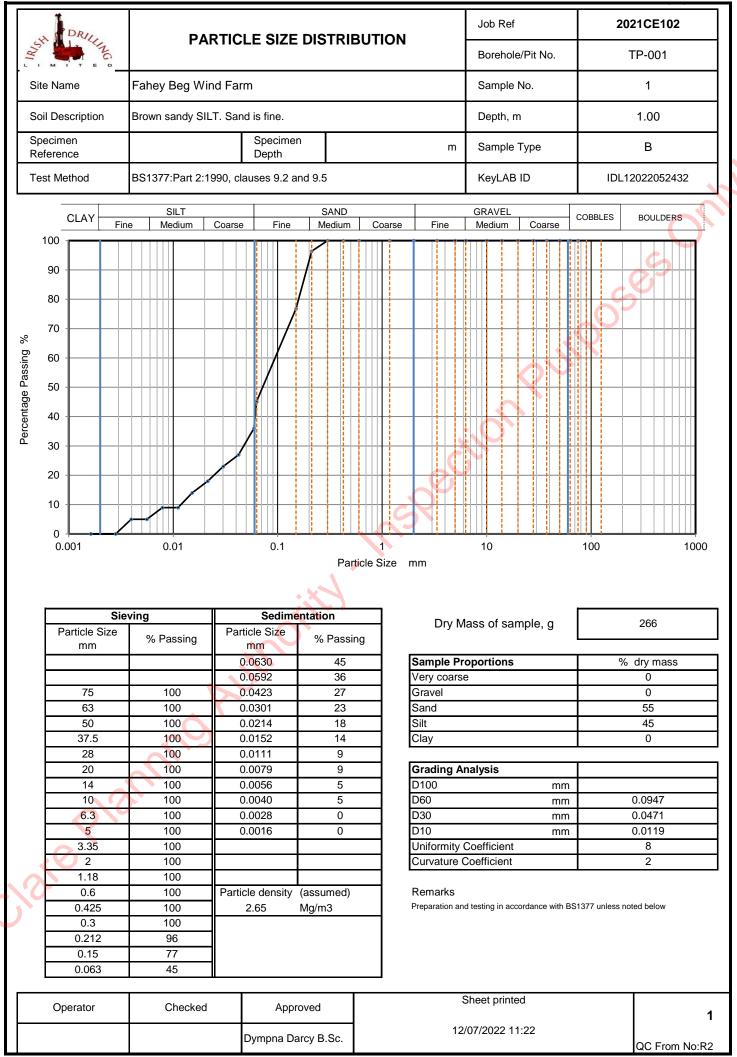
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Client <u>RWE</u> Due Date <u>24/05/2022</u> 11:12 Scheduled Date <u>24/05/2022</u> 11:12	Chemical / Concrete	sample (2;1 water / soil extract)												-	Imei
24/ FR	ರ	Water soluble sulphate content of soil	-							~				4	Joct
Client <u>RWE</u> Date 24/05 J Date 24/05		noitingl nO seo.													ent
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Project ID 2021CE102 Project Name Fahey Beg Wind Farm Schedule ID 2021CE102_2		Location	TP-001	601	FP-002	FP-002	-003	003	-003	TP-004	-005	-005	-005		//:sc
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		Summary of Classification Test Results													
Project No.		Project Name Fahey Beg Wind Farm													
2021CE102															
Hole No.	Sar Ref Top		nple Base	Туре		Soil Description	Density bulk dry		W	Passing 425µm	LL	PL	PI	Particle density	Remarks
			Dusc	Type			Mg/n	n3	% %	%	% %	%	%	Mg/m3	
TP-001	1	1.00	1.30	В		Brown sandy SILT. Sand is fine.			25.0	100					
TP-002	2	2.00	2.30	В		Brown SAND.			37.0	100					NP
TP-003	2	2.00	2.20	В		Brown SILT			44.0	100	44	26	18		М
TP-003	4	4.10	4.40	в		Brown slightly sandy SILT. Sand is fine.			30.0	100	31	22	9		CL
TP-004	1	1.00	1.30	в		Brown very silty fine and medium SAND.			17.0	98					C)
TP-005	1	0.00	1.50	в		Brown slightly sandy SILT.			47.0	99				0	
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l tests perf	ormed	in acco	rdance v	with BS	61377:	1990 unless specified othe	rwise								
	L = Liquid Limit, PL = Plas			= Plastic Limit, PI = Plastic	ticity Index Da			Date Printed			oved	Ву	Table		
Density	test	ment unles	Liquid Limit			Particle	e density		07/12/2022 00:00						1
		acement				ss : sp - small pyknometer int test gj - gas jar				QC From No: R1					sheet

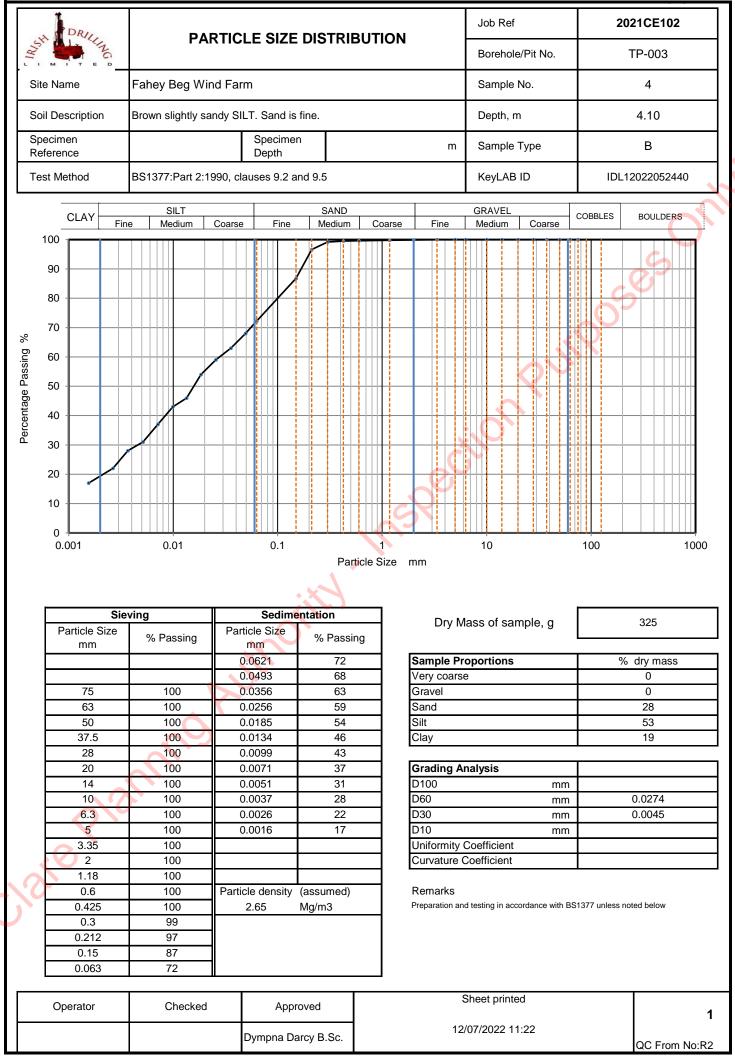
Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39 Approved Signatures: Dympna Darcy (DCD) Lab Manager, Declan Joyce (DJ) Chartered Geotechnical Engineer, Ronan Killeen (RK) Quality Manager.



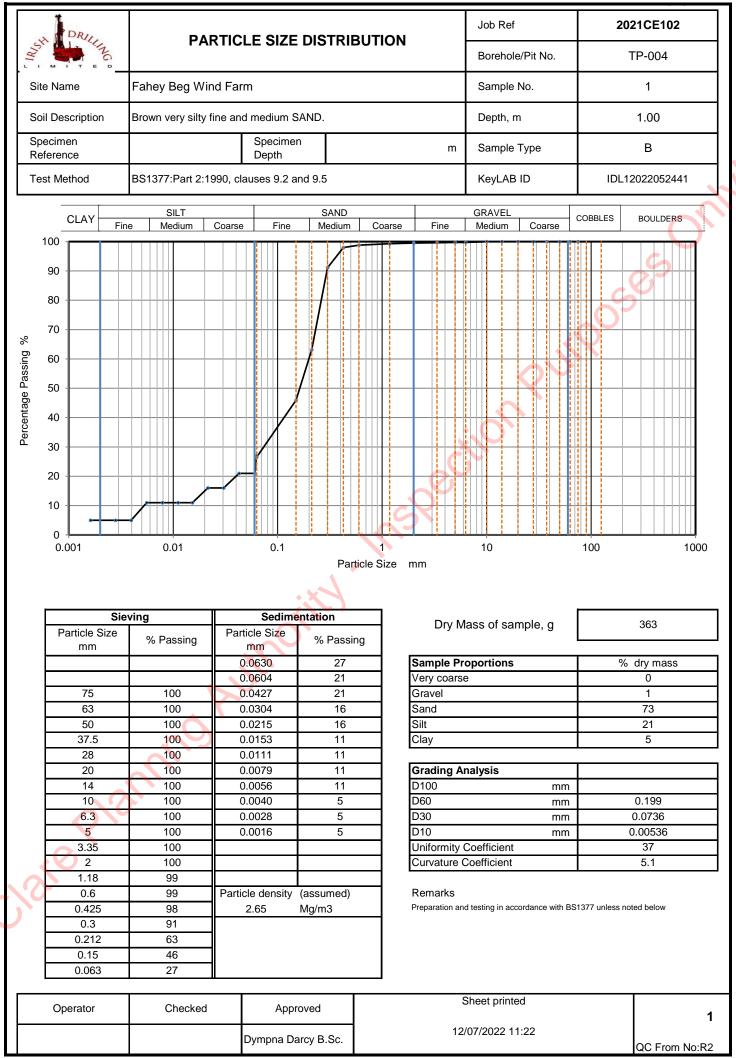
QC Form: R1



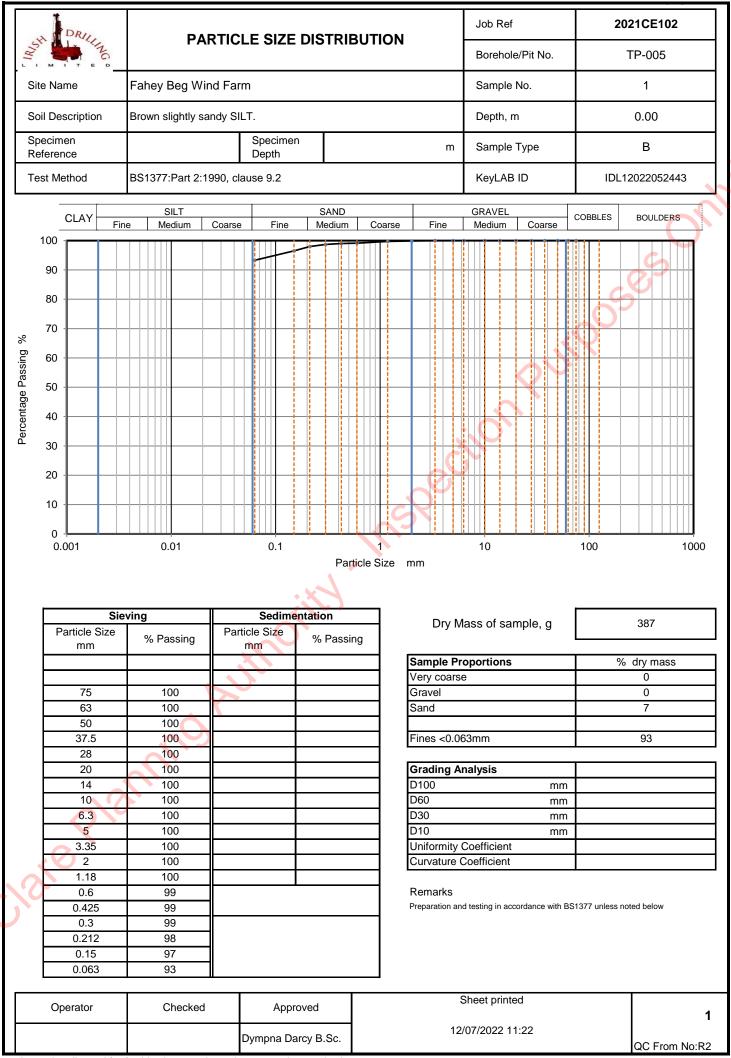
Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39



Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39



Tested in: Irish Drilling Ltd. (IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39



Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39



Irish Drilling Limited Old Galway Road Loughrea Co. Galway

Attention: Dympna Darcy

Unit 7-8 Hawarden Business Park Manor Road (off Manor Lane) Hawarden Deeside CH5 3US Tel: (01244) 528777 email: hawardencustomerservices@alsglobal.com Website: www.alsenvironmental.co.uk

urposes

CERTIFICATE OF ANALYSIS

Date of report Generation: Customer: Sample Delivery Group (SDG): Your Reference: Location: Report No: Order Number: 13 July 2022 Irish Drilling Limited 220706-92 2021CE102 Faheybeg WF 654229 11407

We received 4 samples on Wednesday July 06, 2022 and 4 of these samples were scheduled for analysis which was completed on Wednesday July 13, 2022. Accredited laboratory tests are defined within the report, but opinions, interpretations and on-site data expressed herein are outside the scope of ISO 17025 accreditation.

Should this report require incorporation into client reports, it must be used in its entirety and not simply with the data sections alone.

Chemical testing (unless subcontracted) performed at ALS Life Sciences Ltd Hawarden.

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All sample data is provided by the customer. The reported results relate to the sample supplied, and on the basis that this data is correct.

Incorrect sampling dates and/or sample information will affect the validity of results. The customer is not permitted to reproduce this report except in full without the approval of the laboratory.

Approved By:

Sonia McWhan Operations Manager



ALS Life Sciences Limited. Registered Office: Units 7 & 8 Hawarden Business Park, Manor Road, Hawarden, Deeside, CH5 3US. Registered in England and Wales No. 4057291. Version: 3.3 Version Issued: 13/07/2022 Jare Planmo Authority. Inspection Punposes Only

CERTIFICATE OF ANALYSIS



SDG: 220706-92 Client Ref.: 2021CE102

Report Number: 654229 Location: Faheybeg WF Superseded Report:

Validated

Received Sample Overview

b Sample No(s)	Received S Customer Sample Ref.	AGS Ref.	Depth (m)	Sampled Date
26546501	TP001	B1	1.00 - 1.30	13/05/2022
26546507	TP002	B2	2.00 - 2.30	13/05/2022
26546511	TP003	B4	4.10 - 4.40	13/05/2022
26546514	TP004	B1	1.00 - 1.30	13/05/2022
<pre>/ received samples which</pre>	have had analysis scheduled will b	be shown on the following pa	ges.	
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CERTIFICATE OF ANALYSIS Report Number: 654229

Location: Faheybeg WF

SDG: 220706-92

Client Ref.: 2021CE102

Results Legend

Page 3 of 8

Validated

Superseded Report:

CERTIFICATE OF ANALYSIS

SDG: 220706-92 Client Ref.: 2021CE102 Report Number: 654229 Location: Faheybeg WF Superseded Report:

Validated

Sample Descriptions

very fine <0.0	063mm fine 0.	063mm - 0.1mm	medium 0.1m	m - 2mm coa	arse 2mm - 1	0mm very coars	e >10m
Lab Sample No(s)	Customer Sample Ref.	Depth (m)	Colour	Description	Inclusions	Inclusions 2	
26546501	TP001	1.00 - 1.30	Light Brown	Loamy Sand	Vegetation	None	
26546507	TP002	2.00 - 2.30	Light Brown	Silty Clay	None	None	
26546511	TP003	4.10 - 4.40	Light Brown	Silt Loam	None	None	4
26546514	TP004	1.00 - 1.30	Light Brown	Sand	Stones	None	

These descriptions are only intended to act as a cross check if sample identities are questioned, and to provide a log of sample matrices with respect to MCERTS validation. They are not intended as full geological descriptions.

We are accredited to MCERTS for sand, clay and loam/topsoil, or any of these materials - whether these are derived from naturally ocurring soil profiles, or from fill/made ground, as long as these materials constitute the major part of the sample.

Other coarse granular materials such as concrete, gravel and brick are not accredited if they comprise the major part of the

CERTIFICATE OF ANALYSIS

Validated

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	SDG: 220706- ent Ref.: 2021CE1			Report Number: 6 Location: F	654229 Faheybeg WF	Superseded	Report:	
Results Legend # ISO17025 accredited.	C	ustomer Sample Ref.	TP001	TP002	TP003	TP004		
M mCERTS accredited. aq Aqueous / settled sample. diss.filt Dissolved / filtered sample. tot.unfilt Total / unfiltered sample. * Subcontracted - refer to subcontractor	- report for	Depth (m) Sample Type	1.00 - 1.30 Soil/Solid (S)	2.00 - 2.30 Soil/Solid (S)	4.10 - 4.40 Soil/Solid (S)	1.00 - 1.30 Soil/Solid (S)		
accreditation status. ** % recovery of the surrogate standard to efficiency of the method. The results of	to check the f individual	Date Sampled Sample Time Date Received	13/05/2022 06/07/2022	13/05/2022 06/07/2022	13/05/2022 06/07/2022	13/05/2022 06/07/2022		
compounds within samples aren't corre recovery	ected for the	SDG Ref Lab Sample No.(s)	220706-92 26546501	220706-92 26546507	220706-92 26546511	220706-92 26546514		
(F) Trigger breach confirmed 1-4+§@ Sample deviation (see appendix)		AGS Reference	B1	B2	B4	B1		
omponent	LOD/Units	Method						
isture Content Ratio (% of as eived sample)	%	PM024	19	22	23	15		
Organic Matter (SOM)	<0.35 %	TM132		0.795 @ #				
	1 pH Units	TM133	8.65 @ M	7.76 @ M	8.41 @ M	8.3 @ M		
ter Soluble Sulphate as SO4 2:1 ract	<0.004 g/l	TM243	0.01 @ M	0.0279 @ M	0.0141 @ M	0.0069 @ M	C	2
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CERTIFICATE OF ANALYSIS

Report Number: 654229 Location: Faheybeg WF Superseded Report:

Table of Results - Appendix

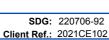
Method No PM024 TM132 TM133 TM243 not applicable. cal testing (unless	Reference Modified BS 1377 In - house Method BS 1377: Part 3 1990;BS 6068-2.5	Description Soil preparation including homogenisation, moisture screens of soils for Asbestos Containing Material ELTRA CS800 Operators Guide Determination of pH in Soil and Water using the GLpH pH Meter
TM132 TM133 TM243 not applicable.	In - house Method	Material ELTRA CS800 Operators Guide
TM133 TM243 not applicable.		
TM243 not applicable.	55 Torrin and 1990,05 0000-2.0	Potentination of DELIT COLLARG WALL USING THE GEDT DE WEEK
not applicable.		Mixed Anions In Soils By Kone
	subcontracted) performed at ALS Life Sciences Ltd Hawa	
	Plannohutn	stite
Jare		

SDG: 220706-92

Client Ref.: 2021CE102

CERTIFICATE OF ANALYSIS Report Number: 654229

Superseded Report:



Test Completion Dates

	3: 220706-9 :: 2021CE10				Number: 654 .ocation: Fat	
			Tes	st Com	pletion	Dates
Lab Sam	ple No(s)	26546501	26546507	26546511	26546514	
Customer Sar	mple Ref.	TP001	TP002	TP003	TP004	
	AGS Ref.	B1	B2	B4	B1	
-	Depth	1.00 - 1.30	2.00 - 2.30	4.10 - 4.40	1.00 - 1.30	
	Туре	Soil/Solid (S)	Soil/Solid (S)	Soil/Solid (S)	Soil/Solid (S)	
ons by Kone (soil)		13-Jul-2022 11-Jul-2022	12-Jul-2022 11-Jul-2022	13-Jul-2022 08-Jul-2022	13-Jul-2022 11-Jul-2022	
mple description		07-Jul-2022	07-Jul-2022	07-Jul-2022	07-Jul-2022	
al Organic Carbon			13-Jul-2022			
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:19:29 13/07/2022				D	and 7 of 9	

CERTIFICATE OF ANALYSIS



220706-92 2021CE102 Report Number: 654229 Location: Faheybeg WF Superseded Report:

Appendix

General

1. Results are expressed on a dry weight basis (dried at 35°C) for all soil analyses except for the following: NRA and CEN Leach tests, flash point LOI, pH, ammonium as NH4 by the BRE method, VOC TICs and SVOC TICs.

2. If sufficient sample is received a sub sample will be retained free of charge for 30 days after analysis is completed (e-mailed) for all sample types unless the sample is destroyed on testing. The prepared soil sub sample that is analysed for asbestos will be retained for a period of 6 months after the analysis date. All bulk samples will be retained for a period of 6 months after the analysis date. All samples received and not scheduled will be disposed of one month after the date of receipt unless we are instructed to the contrary. Once the initial period has expired, a storage charge will be applied for each month or part thereof until the client cancels the request for sample storage. ALS reserve the right to charge for samples received and stored but not analysed.

3. With respect to turnaround, we will always endeavour to meet client requirements wherever possible, but turnaround times cannot be absolutely guaranteed due to so many variables beyond our control.

4. We take responsibility for any test performed by sub-contractors (marked with an asterisk). We endeavour to use UKAS/MCERTS Accredited Laboratories, who either complete a quality questionnaire or are audited by ourselves. For some determinands there are no UKAS/MCERTS Accredited Laboratories, in this instance a laboratory with a known track record will be utilised.

5. If no separate volatile sample is supplied by the client, or if a headspace or sediment is present in the volatile sample, the integrity of the data may be compromised. This will be flagged up as an invalid VOC on the test schedule and the result marked as deviating on the test certificate.

6. NDP - No determination possible due to insufficient/unsuitable sample.

7. Results relate only to the items tested.

8. LoDs (Limit of Detection) for wet tests reported on a dry weight basis are not corrected for moisture content.

9. **Surrogate recoveries** - Surrogates are added to your sample to monitor recovery of the test requested. A % recovery is reported, results are not corrected for the recovery measured. Typical recoveries for organics tests are 70-130%. Recoveries in soils are affected by organic rich or clay rich matrices. Waters can be affected by remediation fluids or high amounts of sediment. Test results are only ever reported if all of the associated quality checks pass; it is assumed that all recoveries outside of the values above are due to matrix affect.

10. Stones/debris are not routinely removed. We always endeavour to take a representative sub sample from the received sample.

11. In certain circumstances the method detection limit may be elevated due to the sample being outside the calibration range. Other factors that may contribute to this include possible interferences. In both cases the sample would be diluted which would cause the method detection limit to be raised.

12. For dried and crushed preparations of soils volatile loss may occur e.g volatile mercury.

13. For leachate preparations other than Zero Headspace Extraction (ZHE) volatile loss may occur.

14. For the BSEN 12457-3 two batch process to allow the cumulative release to be calculated, the volume of the leachate produced is measured and filtered for all tests. We therefore cannot carry out any unfiltered analysis. The tests affected include volatiles GCFID/GCMS and all subcontracted analysis.

15. Analysis and identification of specific compounds using GCFID is by retention time only, and we routinely calibrate and quantify for benzene, toluene, ethylbenzenes and xylenes (BTEX). For total volatiles in the C5-C12 range, the total area of the chromatogram is integrated and expressed as ug/kg or ug/l. Although this analysis is commonly used for the quantification of gasoline range organics (GRO), the system will also detect other compounds such as chlorinated solvents, and this may lead to a falsely high result with respect to hydrocarbons only. It is not possible to specifically identify these non-hydrocarbons, as standards are not routinely run for any other compounds, and for more definitive identification, volatiles by GCMS should be utilised.

16. We are accredited to MCERTS for sand, clay and loam/topsoil, or any of these materials - whether these are derived from naturally occurring soil profiles, or from fill/made ground, as long as these materials constitute the major part of the sample. Other coarse granular material such as concrete, gravel and brick are not accredited if they comprise the major part of the sample.

17 Data retention. All records, communications and reports pertaining to the analysis are archived for seven years from the date of issue of the final report.

18. Tentatively Identified Compounds (TICs) are non-target peaks in VOC and SVOC analysis. All non-target peaks detected with a concentration above the LoD are subjected to a mass spectral library search. Non-target peaks with a library search confidence of >75% are reported based on the best mass spectral library match. When a non-target peak with a library search confidence of <75% is detected it is reported as "mixed hydrocarbons". Non-target compounds identified from the scan data are semi-quantified relative to one of the deuterated internal standards, under the same chromatographic conditions as the target compounds. This result is reported as a semi-quantitative value and reported as Tentatively Identified Compounds (TICs). TICs are outside the scope of UKAS accreditation and are not moisture corrected.</p>

19. Sample Deviations

If a sample is classed as deviated then the associated results may be compromised.

	Container with Headspace provided for volatiles analysis
2	Incorrect container received
3	Deviation from method
4	Matrix interference
•	Sample holding time exceeded in laboratory
(\mathbf{a})	Sample holding time exceeded due to late arrival of instructions or samples
§	Sampled on date not provided

20. Asbestos

When requested, the individual sub sample scheduled will be analysed in house for the presence of asbestos fibres and asbestos containing material by our documented in house method TM048 based on HSG 248 (2021), which is accredited to ISO17025. If a specific asbestos fibre type is not found this will be reported as "Not detected". If no asbestos fibre types are found all will be reported as "Not detected" and the sub sample analysed deemed to be clear of asbestos. If an asbestos fibre type is found it will be reported as detected (for each fibre type found). Testing can be carried out on asbestos positive samples, but, due to Health and Safety considerations, may be replaced by alternative tests or reported as No Determination Possible (NDP). The quantity of asbestos present is not determined unless specifically requested.

Identification of Asbestos in Bulk Materials & Soils

The results for identification of asbestos in bulk materials and soils are obtained from supplied bulk materials and soils which have been examined to determine the presence of asbestos fibres using ALS (Hawarden) in-house method of transmitted/polarised light microscopy and central stop dispersion staining, based on HSG 248 (2021).

The results for identification of asbestos in soils are obtained from a homogenised sub sample which has been examined to determine the presence of asbestos fibres using ALS (Hawarden) in-house method of transmitted/polarised light microscopy and central stop dispersion staining.

Asbe stos Type	Common Name		
Chrysof le	WhiteAsbestos		
Amosite	Brow n Asbestos		
Cio d dolite	Blue Asbe stos		
Fibrous Act nolite	-		
Fib no us Anthop hyll ite	-		
Fibrous Tremolite	-		

Visual Estimation Of Fibre Content

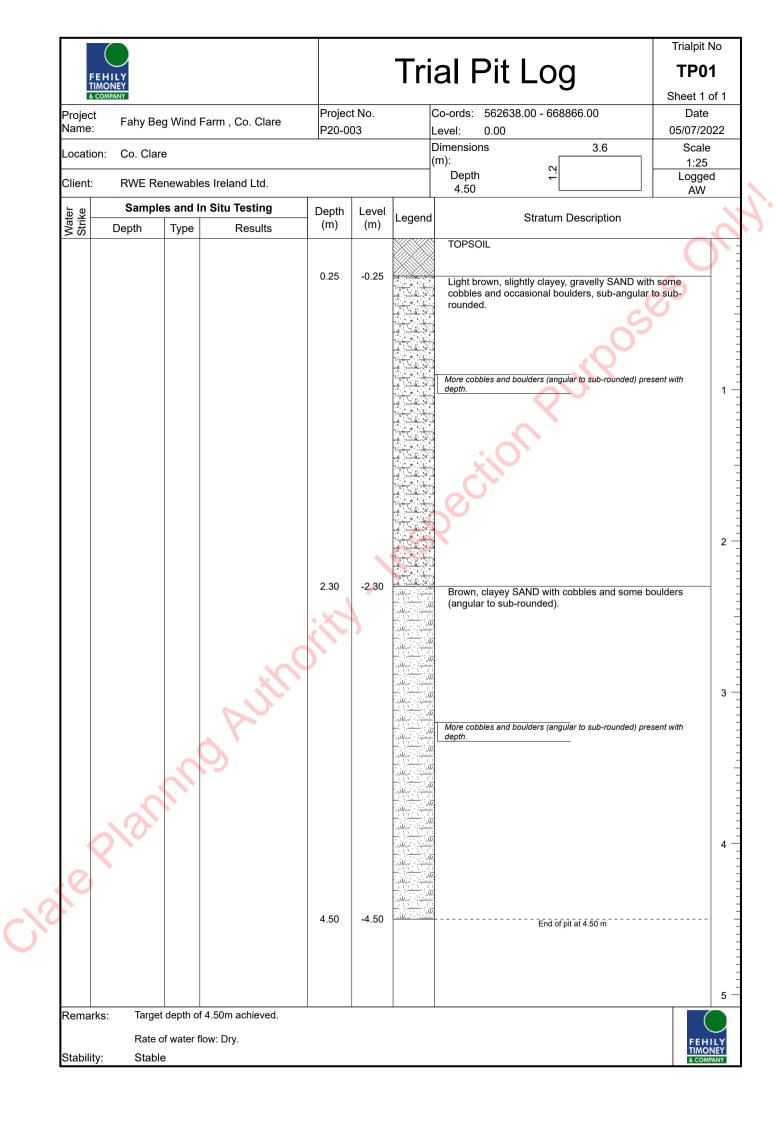
Estimation of fibre content is not permitted as part of our UKAS accredited test other than: - Trace - Where only one or two asbestos fibres were identified.

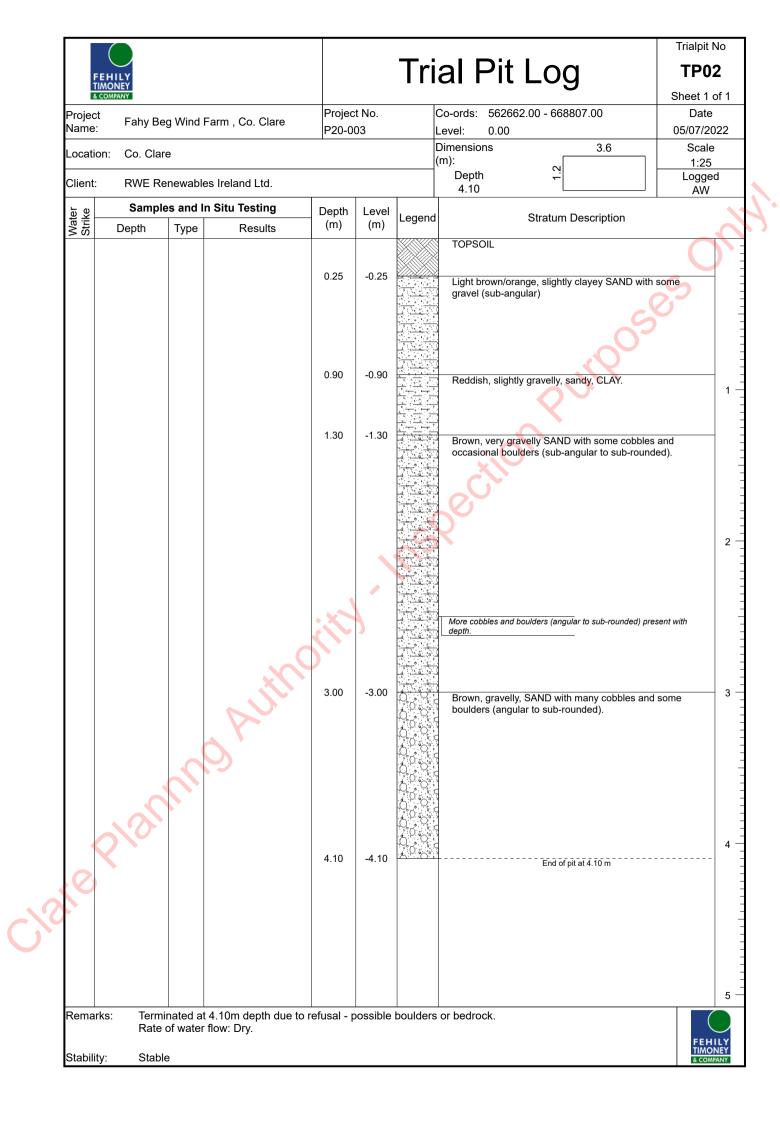
Respirable Fibres

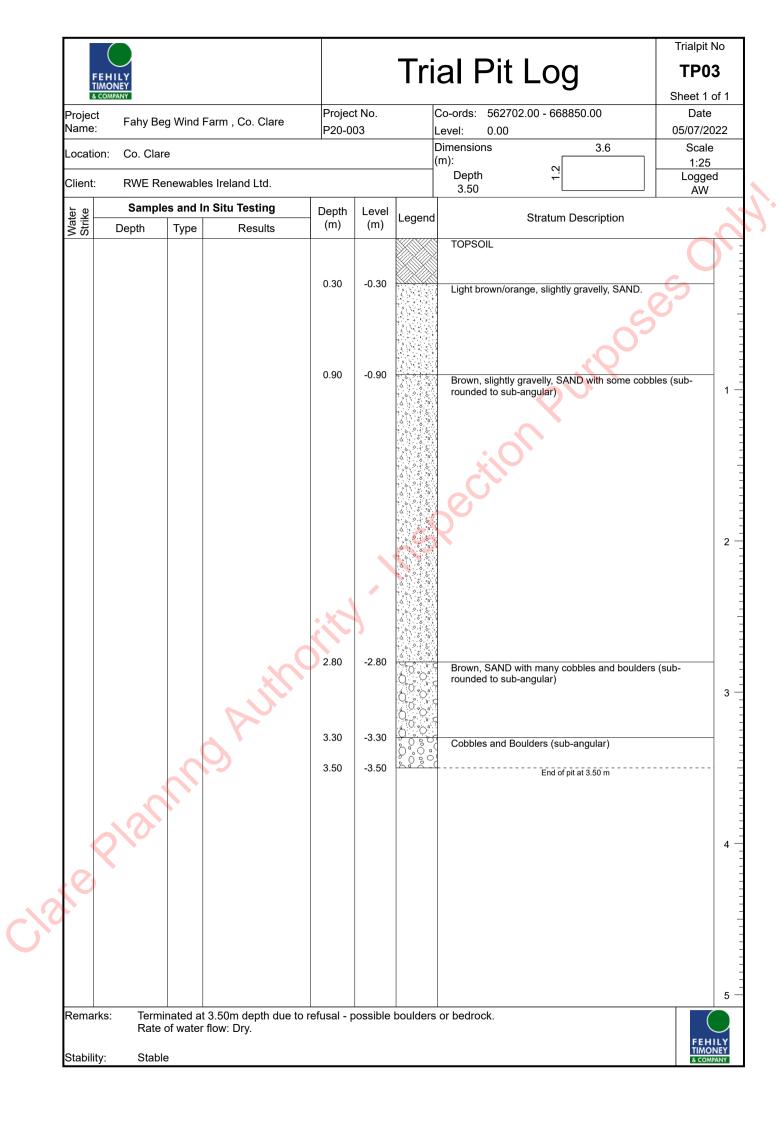
Respirable fibres are defined as fibres of <3 μ m diameter, longer than 5 μ m and with aspect ratios of at least 3:1 that can be inhaled into the lower regions of the lung and are generally acknowledged to be most important predictor of hazard and risk for cancers of the lung.

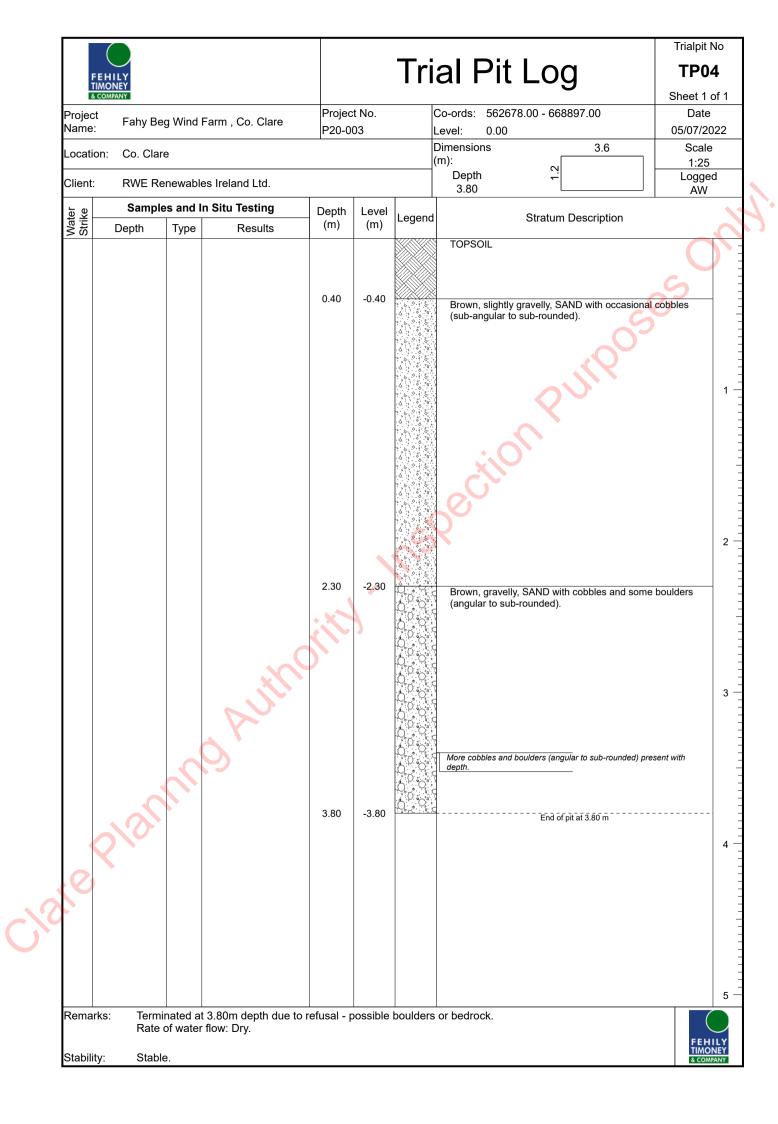
Further guidance on typical asbestos fibre content of manufactured products can be found in HSG 264.

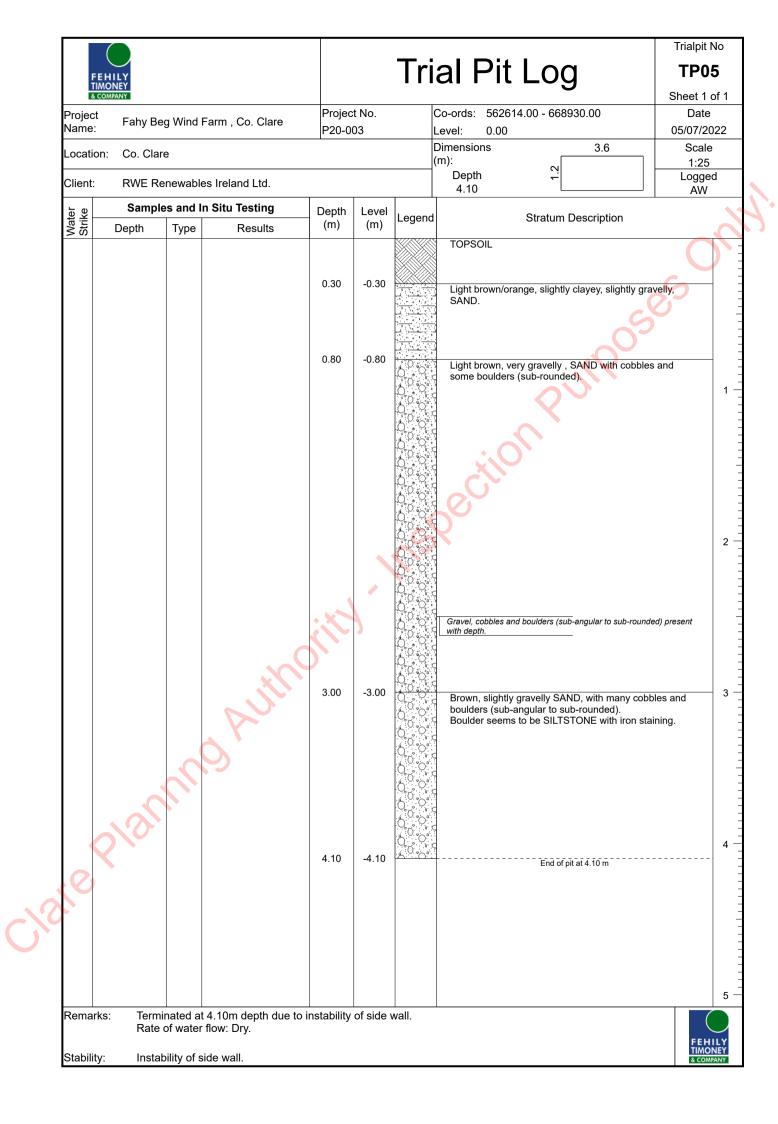
The identification of asbestos containing materials and soils falls within our schedule of tests for which we hold UKAS accreditation, however opinions, interpretations and all other information contained in the report are outside the scope of UKAS accreditation.

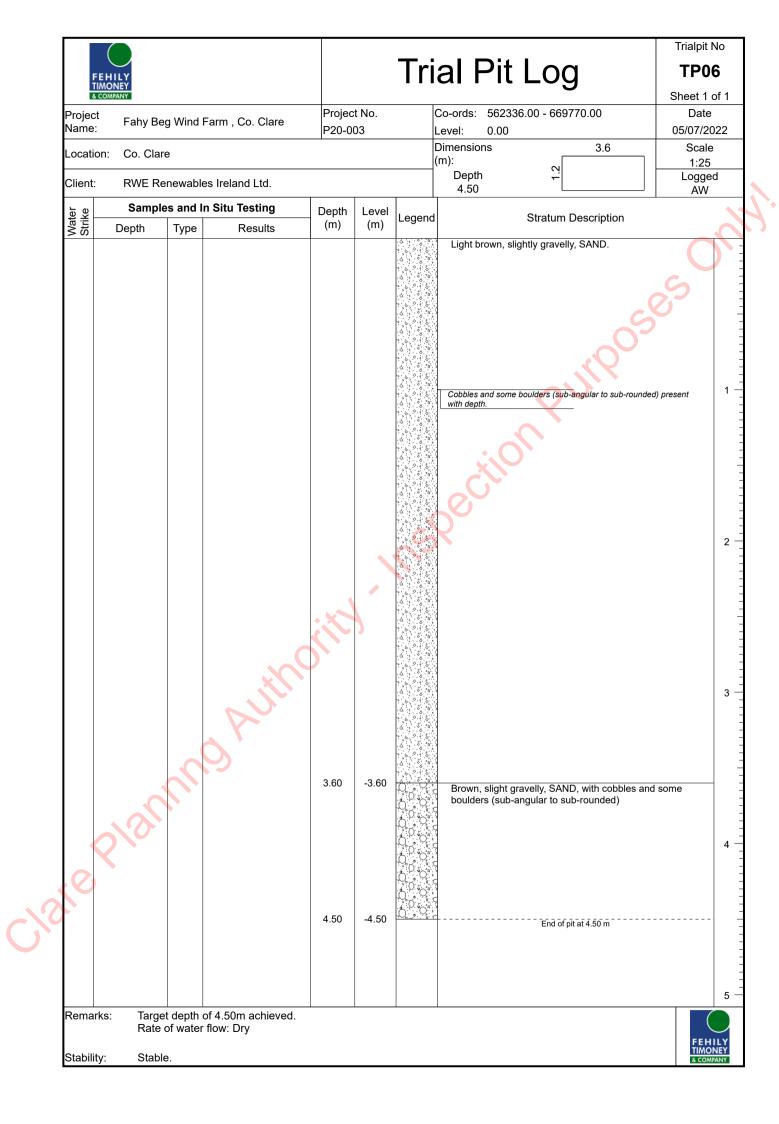


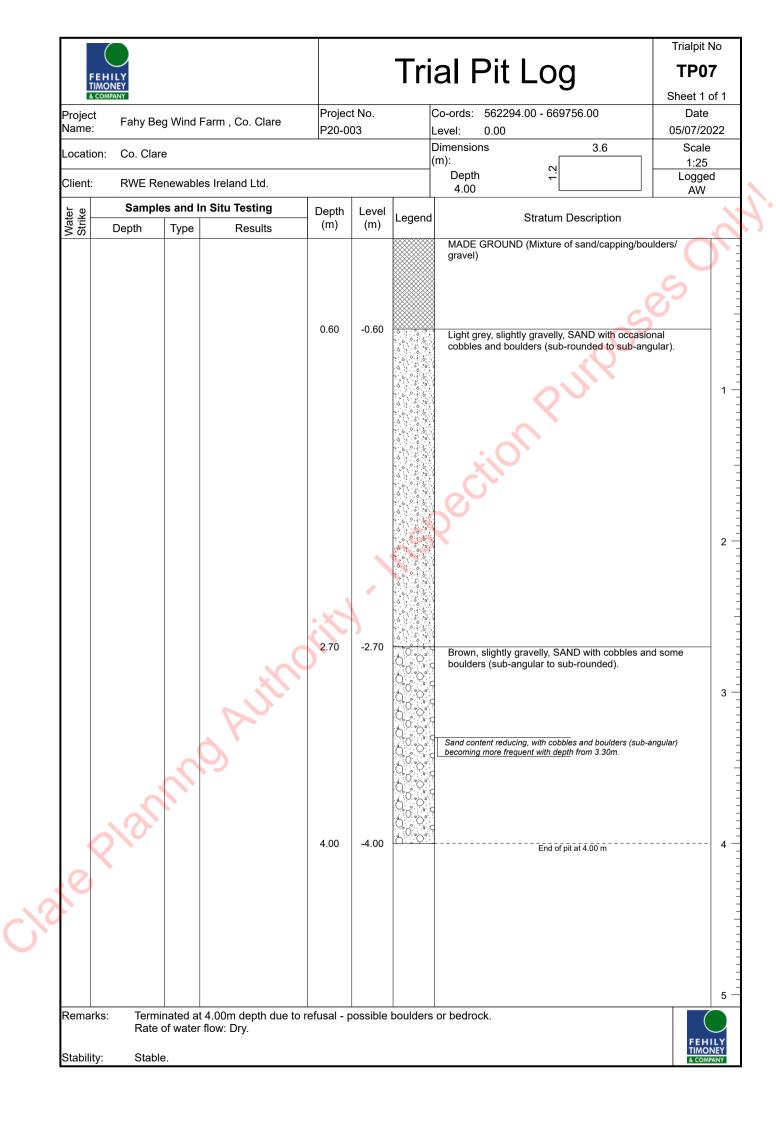


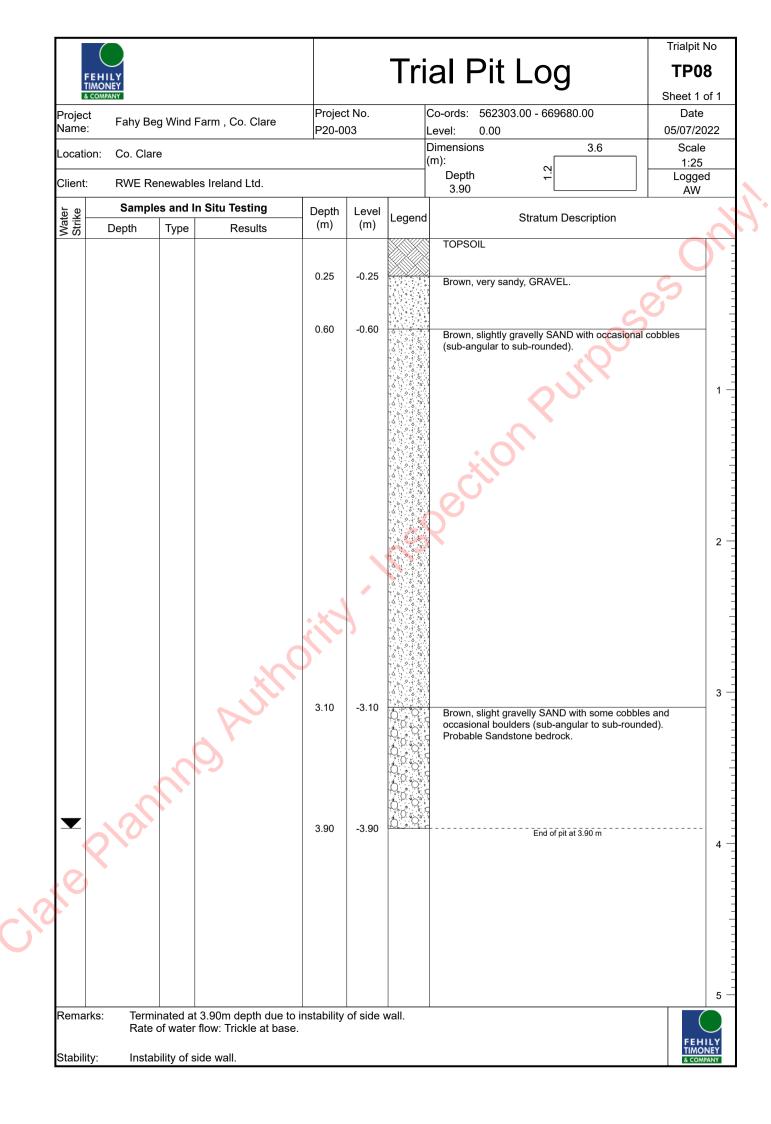












Jare Planmo Authority. Inspection Purposes Only



CONSULTANTS IN ENGINEERING, ENVIRONMENTAL SCIENCE & PLANNING



Factual Ground Investigation de planno Authority Report

Jare Planno Authority. Inspection Purposes Only

IRISH DRILLING LIMITED



LOUGHREA, CO. GALWAY, IRELAND

CONTRACT DRILLING SITE INVESTIGATION

Phone: (091) 841 274 Fax: (091) 847 687

email:

info@irishdrilling.ie

FAHY BEG WIND FARM

SITE INVESTIGATION CONTRACT FACTUAL REPORT

RWE, RWE Platz 1, 45141, Essen. Fehily Timoney & Company, Consulting Engineers, Singleton's Lane, Bagenalstown, Carlow.

al a	Prepared by	Approved by	Rev. Issue Date:	Revision No.
	Ronan Killeen	Declan Joyce	8 th August 2022	21_CE_102/03
<u>Signature</u>				

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Jare Planno Authority. Inspection Purposes Only

FOREWORD

The trial pit records have been compiled from an examination of the samples by a Geotechnical Engineer and from the Drillers' descriptions.

The report presents an opinion on the configuration of the strata within the site based on the trial pit results.

The fieldwork was carried out in accordance with IS EN 1997-2 and BS5930, 2015 Code of Practice for Site



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Appendix 1	Trial Pit Records
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- Appendix 1a Trial Pit Records (Phase 2)
- Appendix 2 Laboratory Test Results
- Appendix 2a Laboratory Test Results (Phase 2)
- Appendix 3 Site Plan
- Appendix 4 Trial Pit Photographs (Phase 2)
- Appendix 5 AGS Data



1.0 Introduction.

Irish Drilling Ltd. (IDL) was instructed by Fehily Timoney & Partners, Consulting Engineers, on behalf of RWE Ireland, to carry out a site investigation at the site of the proposed Fahey Beg Wind Farm Project.

This site investigation was carried out to provide detailed factual geotechnical information of the underlying ground conditions at the location of the proposed works.

Phase 1 fieldwork commenced on February 3rd 2022 and was completed on February 7rd 2022.

Phase 2 fieldwork was carried out on May 13th 2022.

2.0 Site & Geology

The site is located southwest of Killaloe, County Clare.

The site is agricultural in nature and the fieldwork was carried out predominantly on agricultural lands.

Weather conditions in general were quite variable with the majority of the fieldwork carried out over a typical winter period in Ireland.

Geological Survey maps of the area indicate that the site is underlain by Limestone and Sandstone Rock Formations.

A Site Plan, prepared by the client's representatives to show approximate fieldwork locations, is included with this report.

3.0 Fieldwork.

The following plant was mobilised to site to carry out fieldwork operations:

Phase 1 Fieldwork:

1nr Kobelco 7T Tracked Excavator.

Fieldwork carried out to date has included the following:

Ten trial pits were excavated on site using a tracked excavator.

The pits were logged and photographed by an Engineer with observations made on ground conditions, pit stability, water ingress and services encountered.

Small and bulk disturbed soil samples were recovered at each change in strata and returned to the laboratory and presented for testing.

The pits were excavated to depths ranging from 0.40m to 3.00m below ground level.

Phase 2 Fieldwork:

1nr Hitachi 13T Tracked Excavator.

Fieldwork carried out to date has included the following:

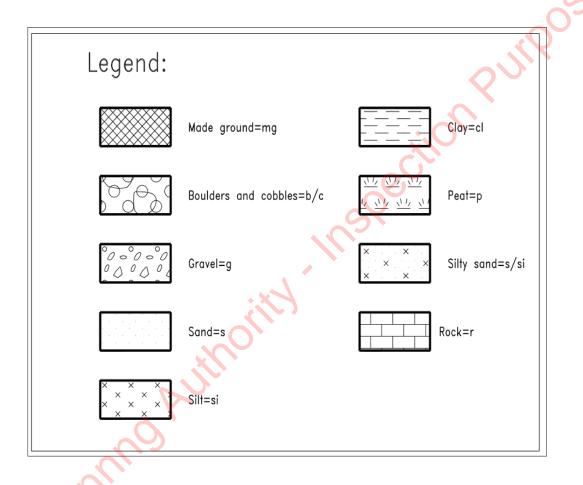
Five trial pits were excavated on site using a tracked excavator.

The pits were logged and photographed by an Engineer with observations made on ground conditions, pit stability, water ingress and services encountered.

Small and bulk disturbed soil samples were recovered at each change in strata and returned to the laboratory and presented for testing.

The pits were excavated to depths ranging from 2.30m to 3.40m below ground level.

The following Key Legend Table details the symbology used on the engineering logs to describe ground conditions encountered:



Ground conditions encountered during the completion of the fieldwork were typical and as expected for this region and predominantly consisted of Glacial Tills overlying bedrock.

The Glacial Tills in general consisted of grey and brown slightly gravelly sandy silt with cobbles and boulders and/or silty clayey sandy gravel with cobbles and boulders.

Possible weathered bedrock was also encountered at trial pit T005 while a number of trial pits encountered 'refusal' on possible bedrock at relatively shallow depths.

Phase 2 fieldwork operations encountered possible made ground to depths of up to 3.40m and predominantly consisted of silty sands interbedded with silt/clay. It is possible that the area where the trial pits were excavated was previously used as a settlement pond for an adjacent quarry.



For detailed descriptions of the ground conditions encountered please refer to the engineering logs included as Appendix 1 and Appendix 1a of this report.

The fieldwork was carried out in accordance with IS EN 1997-2 and BS5930, 2015 Code of Practice for Site Investigations with precedence given to IS EN 1997-2 where applicable.

The fieldwork locations were set out on site using a Trimble CU Bluetooth GPS Surveying Unit and the co-ordinates are included on the logs presented in the appendices. All fieldwork co-ordinates are reported to Irish Transverse Mercator (ITM) with Reduced Levels recorded relative to Malin Head Datum and with an accuracy level of + or -0.10m.

4.0 Laboratory Testing

Representative samples recovered from the boreholes and trial pits were scheduled for testing in the laboratory.

The test schedules were prepared by the Client's Representative and included some or all of the following tests on disturbed soil samples:

- * Moisture Content.
- * Atterberg Limits.
- * Particle Size Distribution.
- * Sedimentation.
- * Chemical (pH, Sulphate).
- * Organic Content.

The records of these laboratory tests results are included as Appendix 2 and Appendix 2a of this factual report.

The soil descriptions as noted on the trial pit logs are in general visual descriptions as observed and logged by our Engineers and are described in accordance with IS EN 1997-2 and BS5930, 2015 Code of Practice for Site Investigations.

Soils descriptions (cohesive or otherwise) are also initially assessed based on the texture and 'feel' of the soil materials as witnessed by our Geotechnical Engineers and in accordance with IS EN 1997-2 and BS5930.

Where laboratory classification tests have been carried out on soil or rock samples then these visual descriptions have been amended accordingly to take into account the results of these classification tests.

The records of all fieldwork, laboratory test results and photographs are included in the appendices of this Factual Report.

Ronan Killeen Chartered Engineer Irish Drilling Limited July 19th 2022 Jare Planno Authority. Inspection Purposes Only



tare planned without inspection purposes on

Jare Planno Authority. Inspection Purposes Only

PROJECT: LOCATION									TRIALPIT: TP-C1 Sheet 1 of 1
CLIENT: H								Co-ordinates:	Rig: 7T Tracked Kobelco
ENGINEEF			oney					E 562,569.6 N 668,946.8	Rev: FINAL
Ground level: GROUNDV Water strikes: 1st: 1.50m 2nd: 3rd:	VATE	O.D. R e to after:			PIT I PIT I LOG	DIREC DIME GED 1	CTION NSION BY:	990-270 A .10 * 2.20 D MM C	DATE: 3.2.22 Shoring/Support: N/A Stability: Pit unstable. Sidewall 1.10 collapse.
Depth (m) Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		RIPTION
0 	⊥	B 1	1.00			47.46	1.50	teeds over very soft wet slightly sandy organ and is fine.	epth.
3		20			S.C				
5 Remarks:	Ingress	of water	at 1.50m bgl.	TP back	filled w	ith arisir		Irilling LTD	Scale: 1:25 Ph. Fax

LOCATION:	: Killalo	eg Wind Farm De, Co. Clare						TRIALPIT: TP-S1 Sheet 1 of 1		
CLIENT: RV ENGINEER:		Timoney					Co-ordinates: E 562,539.6 N 668,774.4	Rig: 7T Tracked Kobelco Rev: FINAL		
Ground level: 4	5.19m O.I							DATE: 3.2.22		
GROUNDWA Water strikes: 1st: 1.70m 2nd: 3rd:	Rose to after: PIT DIRECTION: 090-270 A							Shoring/Support: N/A T Stability: Pit unstable. B 1.10		
Depth (m) Date	Water	Samples Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)	DESC	CRIPTION		
-2 -2 -3 -3 -3 -4 -4 -4 -5					43.49		Reeds over very soft wet grey silty CLAY. Wet rounded to subrounded BOULDERS a bluish grey sandy rounded to subrounded ff Fast water inflow at 1.70m depth. TP terminated at 2.20m bgl. Unable to prog	Ind rounded to subrounded COBBLES with a ine to coarse gravel infill. Sand is coarse.		
	ngress of v	water at 1.70m bgl.	TP back	filled w	ith arisir	igs.		Scale:		
Remarks: Ir	•	-						1:25		

	DJECT:	-	-	ind Farm						TRIALPIT: TP-T001 Sheet 1 of 1
CLI	ENT: R	WE							Co-ordinates:	Rig: 7T Tracked Kobelco
	GINEER:			oney		1			E 563,039.4 N 670,613.	
GRC	nd level: 1 DUNDW: r strikes: 1.80m	ATE				PIT	DIREC DIME GED 1	NSION	090-270 → 3.40 1.00 * 3.40m D MM C	DATE: 3.2.22 → Shoring/Support: N/A Stability: Pit stable. B 1.00
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)	DES	SCRIPTION
0 -							115.01	0.35	Grass over firm grey TOPSOIL with root	es es
						ו×× × •× *•× *•×	•	0.55	Firm orangish grey slightly gravelly sand subrounded fine to medium.	y SILT. Sand is medium. Gravel is rounded to
1						× • × • × • × • × • × • × • × • × • × •	114.46	0.90	Stiff damp grey slightly gravelly sandy Sl shale.	ILT. Gravel is angular to rounded of siltstone and
						× · · × × · · · × × · · · × × · · · × × · · · × · · · · · × · · · · · · × · · · · · · · · · · · · · · · · · · ·			ection	
2		Ţ				× × × × × × × × × × × × × × × × × × ×			Fast water inflow from southeast corner of	of pit at 1.80m depth.
			3 1 2020/00/00/00/00/00/00/00/00/00/00/00/00	2.40-2.70		× × × × · × · × × · × · × × · × · × × · × ·				
3						$\frac{1}{8}$	112.36	3.00	TP terminated at 3.00m bgl. Unable to m	ake progress - hard digging.
4			0							
3										
3 4 5 Rem										
5 Rem	narks: ^h	ngress	of water	at 1.80m bgl.	TP back	filled w	ith arisir	ngs.		Scale:
									drilling LTD	1:25

	DJECT: 1 CATION:	-	-	ind Farm Co. Clare								TRIALPIT: T Sheet 1 of 1	P-T002
CLI	ENT: R	WE								Co-ordinates: E 563,245.5 N 670,310		Rig: 7T Tracked Rev: FINAL	Kobelco
	GINEER: nd level: 1			oney						E 505,245.5 IN 070,510		Rev: FINAL DATE: 3.2.22	
GR(OUNDW. r strikes: dry	АТЕ				PIT	DIREC DIME GED	NSION	: 000-1 1: 1.00 MM	80 → 3.0		Shoring/Support: Stability: Pit stabl	N/A 2.
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DE	SCRIP	PTION	on'i
0						×o × × × ×			Grass o	ver firm brown gravelly SILT. G	ravel is a	ngular to subangular fin	e to medium.
						× _{ox} × × _{ox} ×	142.02	0.30	Browni	sh orange gravelly SILT.		S	
			B 1	0.60-1.00		× × × × × × × × ×	141.72	0.60	Brown	silty clayey very sandy GRAVEI is fine to medium.	L with tra	ces of rootlets.	
			0000000			* * *	141.32	1.00	Graver	is line to medium.	Q	<i>S</i> , ,	
1			8					1.00	and san	with low cobble content. Cobble dstone. ey gravelly sandy SILT. Sand is 1			
						* * * * * * * * * *			to coars	e.			a to Subungului Inio
						ו• וו וו×				000			
			ав 2 Серения	1.80-2.00		x° Q.X Q.X Q.X Q.X				(St			
2						×O×°× ×_~×							
						*`````````````````````````````````````	139.82	2.50					
						END	0		TP tern	ninated at 2.50m bgl. Unable to r	nake proş	gress - hard digging.	
3					2	5							
5				Ó	Y								
			2										
1	.0		*										
Ż													
5				tion TD 1 1	£11. J								C l
кет	arks: T	r ary	on excava	ation. TP back	med w	un arisi	ngs.						Scale: 1:25
1								Irich	n drill	ing LTD			Ph. 1.23

	DJECT: 1 CATION:	•	-	ind Farm								TRIALPIT: TP-T003 Sheet 1 of 1
CLI	ENT: R	WE								Co-ordinates:		Rig: 7T Tracked Kobelco
	GINEER:		•	oney		1				E 563,748.2 N 669,95		Rev: FINAL
GRC	nd level: 1 DUNDWA r strikes: 0.00m	ATE				PIT 1	DIREC DIMEI GED I	NSION	: 000-1 : 1.10 MM	$\begin{array}{c c} 3.40 & 3.40 \\ \mathbf{3.40m} \\ \mathbf{D} \\ \mathbf{C} \end{array}$		DATE: 4.2.22 Shoring/Support: N/A Stability:
Depth (m)	Date	♦ Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DI	ESCRIF	PTION
•0 -		<u> </u>					153.67	0.40	Water s	IL: Grass over firm brown sligh eepage from GL to 1.00m deptl ey mottled orange slightly sand to subrounded gravel and cobb	h observed	
·1						$\begin{array}{c} \begin{array}{c} \times \\ \times \\ \end{array} \\ \times \\ \end{array} \\ \times \\ \times \\ \times \\ \times \\ \times \\$				o 1.60m: large boulder within s	Q	JRP
			ह्य B 1	1.60-2.00		$\begin{array}{c} & \times \\ \\ & \times \\ & \times \\ \\ & \times \\ \\ & \times \\ \\ & \\$	152.47	1.60		ctio		
2				1.00-2.00					Stiff bh rounded	ish grey slightly sandy gravelly to subangular fine to medium.	y SILT/CL	AY with low cobble content. Gravel is
							151.57	2.50	TP term	inated at 2.50m bgl. Obstructic	on as possi	ible rock or boulder.
3				0		50	•					
			2									
4	,e											
5												
	arks: ^h	l ngress	of water 1	from g/l to 1.0	l 10m bgl.	TP bac	kfilled w	ith arisir	ıgs.			Scale:
												1:25

C.I.F.NT: RWF. ENCINCER: Fehily Timoney Conductor 125, 213 no 0.0. GROUNDWATER TO DECINCE 100 - 1			-	-	ind Farm o. Clare						TRIALPIT: TP-T004 Sheet 1 of 1
Zeronalized: 2320 nO. DATE: 4122 GROUNDWATER PTT DIRECTION: 600-180 With Mide PTT DIRECTION: 600-180 Statistic Construction PTT DIRECTION: 600-180 Statisticon PTT DIRECTION: 600-180 <t< th=""><th>CLI</th><th>ENT: R</th><th>WE</th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th>Rig: 7T Tracked Kobelco</th></t<>	CLI	ENT: R	WE								Rig: 7T Tracked Kobelco
GROUNDWATER PT FIREXCTORS (00-180 project) State and the subset of					oney					E 303,8/2.U N 669,577.9	
0 TOPSOIL: Grass over firm brown SULT. 1 125.12 0.20 1 0.70-1.00 1 124.60 0 124.60 0 124.60 1 0.70-1.00 1 124.60 1 124.60 1 0.70-1.00 1 124.60 1 124.60 1 0.70-1.00 1 124.60 1 124.	GRO Water 1st: 2nd:	UNDW	ATE	R			PIT	DIME	NSION	1000-180 A 1.10 * 3.00m D MM	► Shoring/Support: N/A T Stability: Pit stable. B 1.10
1 10750L Units over limit BOOL SLI. 1 125.0 0.20 1 125.0 0.20 1 0.70-1.00 125.0 0.20 1 0.70-1.00 125.0 0.20 1 0.70-1.00 125.0 0.20 1 0.70-1.00 123.0 1.70 1 1.70 123.0 1.70 2 1.70-2.30 123.0 1.70 2 1.70-2.30 123.0 1.70 2 1.70-2.30 123.0 1.70 2 1.70-2.30 123.0 1.70 3 1.70 123.0 1.70 4 1.70 123.0 1.70 5 1.70 123.0 1.70 6 1.70 1.70 7 1.70 1.70 8 1.70 1.70 9 1.70 1.70 9 1.70 1.70 10 1.70 1.70 11 1.70 1.70 12 1.70 1.70 13 1.70 1.70 14 1 1 15 1 1 16 1 1		Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)	DESC	CRIPTION
Image: Second	0							-		TOPSOIL: Grass over firm brown SILT.	6
1 angular for subcoulded of skale and sandstore: 1 1 1 <td></td> <td></td> <td></td> <td></td> <td>0.70.1.00</td> <td></td> <td>* × ° × × × × × × × × × × × ×</td> <td></td> <td></td> <td></td> <td>1 POSE</td>					0.70.1.00		* × ° × × × × × × × × × × × ×				1 POSE
2 1.70-2.30 3 4 5 Remarks: Ingress of water at 1.80m by UT P backfilled with arisings. Ingress of water at 1.80m by UT P backfilled with arisings. Ingress of water at 1.80m by UT P backfilled with arisings. Ingress of water at 1.80m by UT P backfilled with arisings. Ingress of water at 1.80m by UT P backfilled with arisings. Ingress of water at 1.80m by UT P backfilled with arisings.	1			1951 1970 1970 1970 1970 1970 1970	0.70-1.00					Firm grey mottled orange slightly sandy gra angular to subrounded of shale and sandsto sandstone.	avelly SILT with low cobble content. Gravel is ne. Cobbles are rounded to subangular of
Slight water seepage from pit side walls observed at 1.80m depth.			Ţ	aB 2	1.70-2.30		·× · • • •		1.70	subangular fine to medium.	
3 4 4 5 Remarks: Ingress of water at 1.80m byl. TP backfilled with arisings. FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND FND	2			10000000000						Slight water scepage from pit side walls ob	served at 1.80m depth.
Remarks: Ingress of water at 1.80m bgl. TP backfilled with arisings. Scale:				3					2.30	TP terminated at 2.30m bgl. Unable to mak	te progress - hard digging.
Remarks: Ingress of water at 1.80m bgl. TP backfilled with arisings. Scale:	3				0	4	Š				
Remarks: Ingress of water at 1.80m bgl. TP backfilled with arisings. Scale:				2							
Remarks: Ingress of water at 1.80m bgl. TP backfilled with arisings. Scale:	4	Se									
Remarks: Ingress of water at 1.80m bgl. TP backfilled with arisings. Scale:	5										
1.25		arks: E	ngress	of water a	at 1.80m bgl. '	TP back	 filled w	ith arisir	l 1gs.		Scale:
Irish drilling LTD											1:25

	DJECT: 1 CATION:	-	-	ind Farm o. Clare							TRIALPIT: T Sheet 1 of 1	Р-Т005
CLII	ENT: RV GINEER:	WE								ordinates: 3,791.9 N 670,570.4	Rig: 7T Tracked Rev: FINAL	Kobelco
Grou	nd level: m DUNDWA	1 O.D.		v					1		DATE: 7.2.22	
	r strikes: dry		N e to after:			PIT	DIREC DIME GED I	NSION	090-270 2.00 * 3.30r MM	A	Shoring/Support: Stability: Pit stab	N/A lle.
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DESC	CRIPTION	oni
) -								0.20		s roots and briars over firm bunded of sandstone.	brown gravelly SILT with b	oulder. Boulders are
				0.40				0.40		•	asts with orange sandy grave	l infill.
1						END				t 0.40m bgl. Obstruction as	RUIR	
3				no	A	<u>z</u> e	0	N.				
4			2									
5 Rem	arks: T	P dry	on excava	tion. TP back	filled wi	 ith arisi	ngs.	•		with a putting.		Scale:
	U	nable	to survey represent	due to dense	toliage -	 locatio 	n set out	t using h	dhelkd GPS and	with co-ordinates as receiv	ved from	1:25

	DJECT: 1 CATION:	-	-	ind Farm								TRIALPIT: TP-T006 Sheet 1 of 1
CLI	ENT: RV	WE								Co-ordinates: E 564,167.0 N 670,388]	Rig: 7T Tracked Kobelco Rev: FINAL
	JINEER: nd level: n			oney						E 304,107.0 IN 070,388		Rev: FINAL DATE: 7.2.22
GRO	DUNDWA strikes: dry	ATE				PIT	DIREC DIME GED 1	NSION	: 000-1 : 1.00 MM	80 → 3.2		Shoring/Support: N/A Stability:
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DE	SCRIP	TION
) –								0.38		IL: Grass and briars over firm da	-	n silty CLAY.
l						* • × • × * • × • ×			to med	um of shale.		SILT. Graver is angular to subangular line
			B 1	1.20-1.80		\$\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$		1.20	Stiff da siltston	mp blue mottled grey slightly san e and shale.	ndy gravel	lly silty CLAY. Gravel is fine to coarse of
2			3 2 200000000000000000000000000000000000	2.10-2.70				2.10		grey brown silty sandy GRAVE is fine to coarse, angular to subar		
3			5.	0,00	6	END			TP tern	ninated at 2.70m bgl. Obstruction	n as possib	ole slate rock.
	ie i		G									
5 Rema	arks: T	P dry o Inable	on excava to record	ation. TP back reduced level	filled wi	ith arisin foliage.	ngs.					Scale:
										ing LTD		1:25

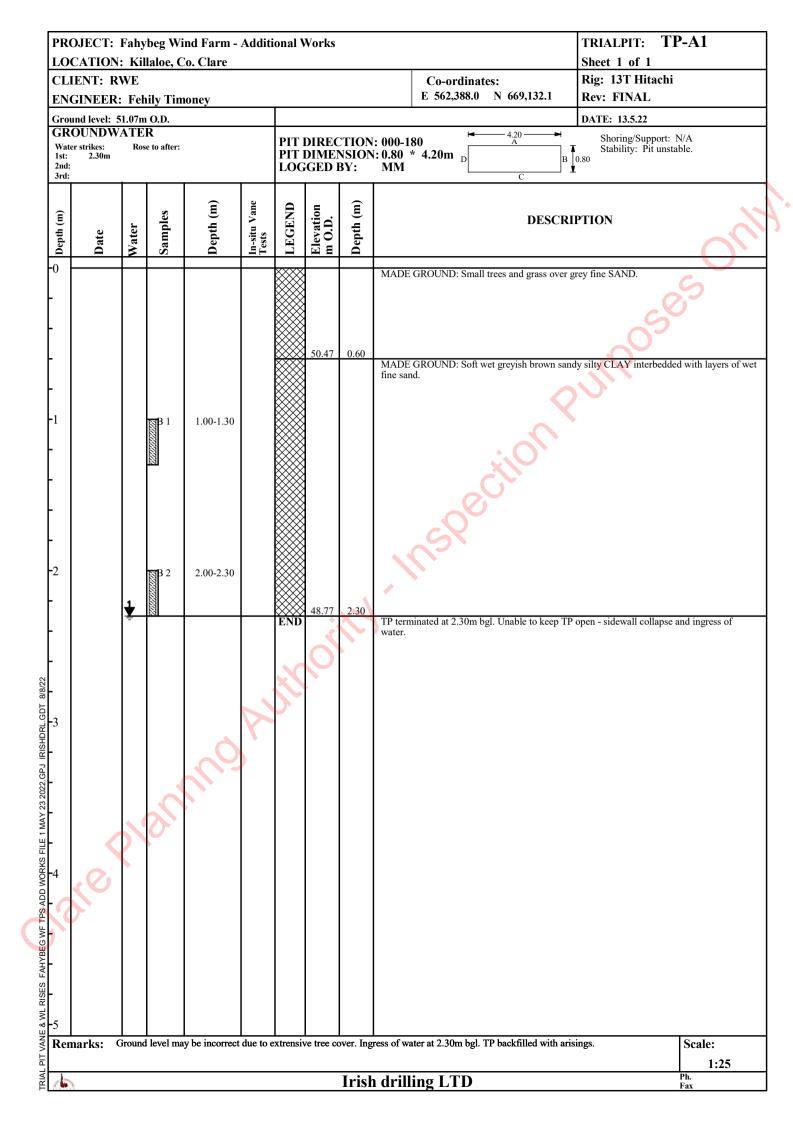
			ind Farm Co. Clare								TRIALPIT: TP-T007 Sheet 1 of 1
CLIENT	: RWE								Co-ordinates: E 564,519.2 N 670,704.		Rig: 7T Tracked Kobelco Rev: FINAL
ENGINE Ground lev			ioney						2 301,317.2 11 070,704.	-	DATE: 7.2.22
GROUN Water strike 1st: dr 2nd: 3rd:	DWATI s: R				PIT 1	DIREC DIME GED 1	NSION	: 320-1 : 1.10 MM	40 3.30 A ★ 3.30 D C		Shoring/Support: N/A Stability:
Depth (m)	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DES	SCRI	PTION
)					× × × × × × × × × × × × × × × × × × ×		1.00			2	ge slightly sandy sightly gravelly SILT.
		ана 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1.00-1.50		₩ 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		1.50		mottled grey silty clayey very same is fine to coarse.	•	
2									with high boulder content. Bould		CLAY. Gravel is flat and angular to rounded to subrounded of sandstone and
	_						2.30				
3 4		21				Or			ninated at 2.30m bgl. Obstruction	ar bosz	
5 Remarks	: TP dr	y on excave	ation. TP back	filled wi	ith arisi	ngs.					Scale:
	Unabi	e to record	ation. TP back reduced level	due to f	foliage.	.					1:25
Day							Irish	dril	ling LTD		Ph. Fax

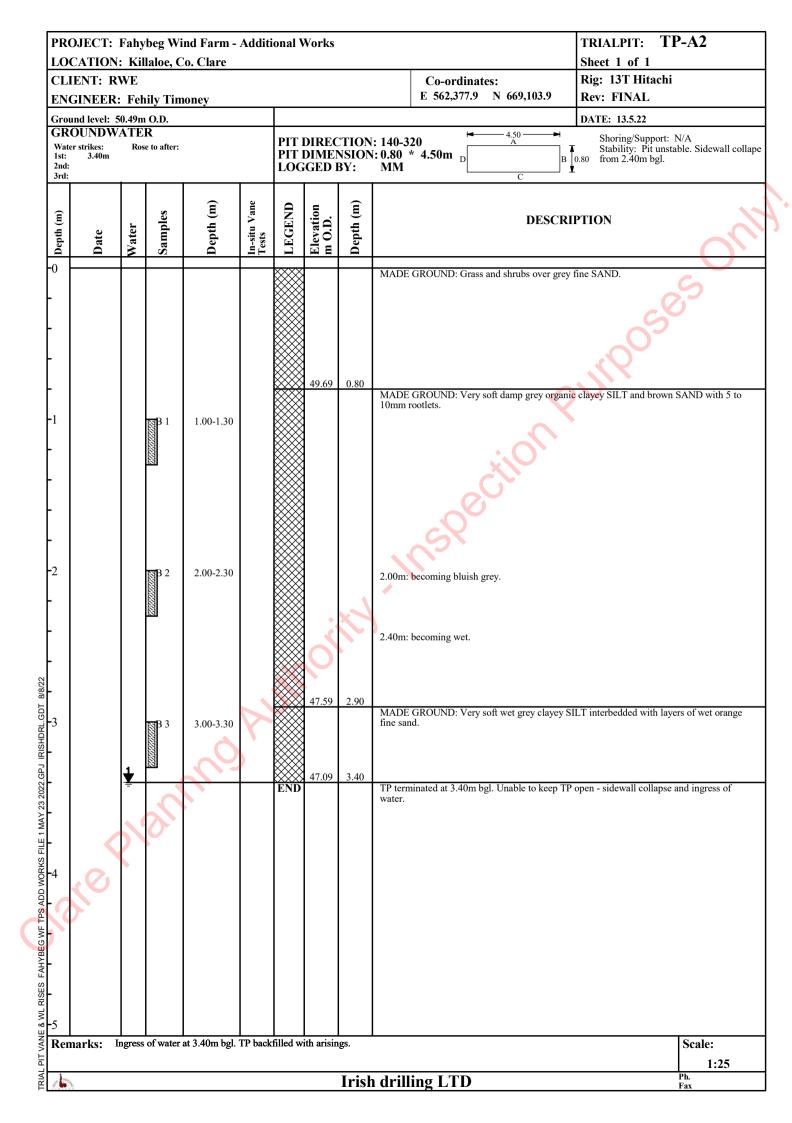
LOC	CATION	Kil	-	'ind Farm Co. Clare							Sheet 1 of 1	P-T008
	ENT: RV GINEER:		nily Tim	oney						Co-ordinates: E 564,593.3 N 670,291.8	Rig: 7T Tracked Rev: FINAL	Kobelco
Grou	nd level: 1 DUNDW	57.16	m O.D.	v							DATE: 7.2.22	
	r strikes: dry		n after:			PIT	DIREC DIME GED 1	NSION	: 040-2 : 1.00 MM	$\begin{array}{c c} 330 & 320 \\ 330 & \mathbf{320m} \\ 5 & \mathbf{320m} \\ 5 & 5 \\ 5 \\ 5 & 5 \\ 5 & 5 \\ $	Shoring/Support: Stability: Pit stabl	N/A e.
Depth (m)	Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)		DESC	RIPTION	00
0							156.96	0.20		DIL: Grass over firm brown gravelly S		S
			ज्ञ B 1	0.50-1.00			156.66	0.50			S	
				0.50-1.00					Pinkisl boulde Traces	n grey brown silty clayey very sandy or r content. Gravel is angular to subang of rootlets.Gravel is fine to coarse.	GRAVEL with low cobble co gular of shale and siltstone.	ntent and medium
1			22							ion		
							155.66	1.50	Blocky BOUL	and angular to subrounded COBBLI DERS with a silty gravelly infill.	ES and blocky and angular to	subrounded
							155.36	1.80				
2						END			I P teri	ninated at 1.80m bgl. Obstruction as	possible rock.	
							Ś					
						X						
3				A								
				200								
			0									
4	<i>.</i>											
2												
5												
Rem	narks: T	P dry	on excava	ation. TP back	filled w	ith arisi	ngs.					Scale: 1:25
1								Irisł	dril	ling LTD		Ph. Fax



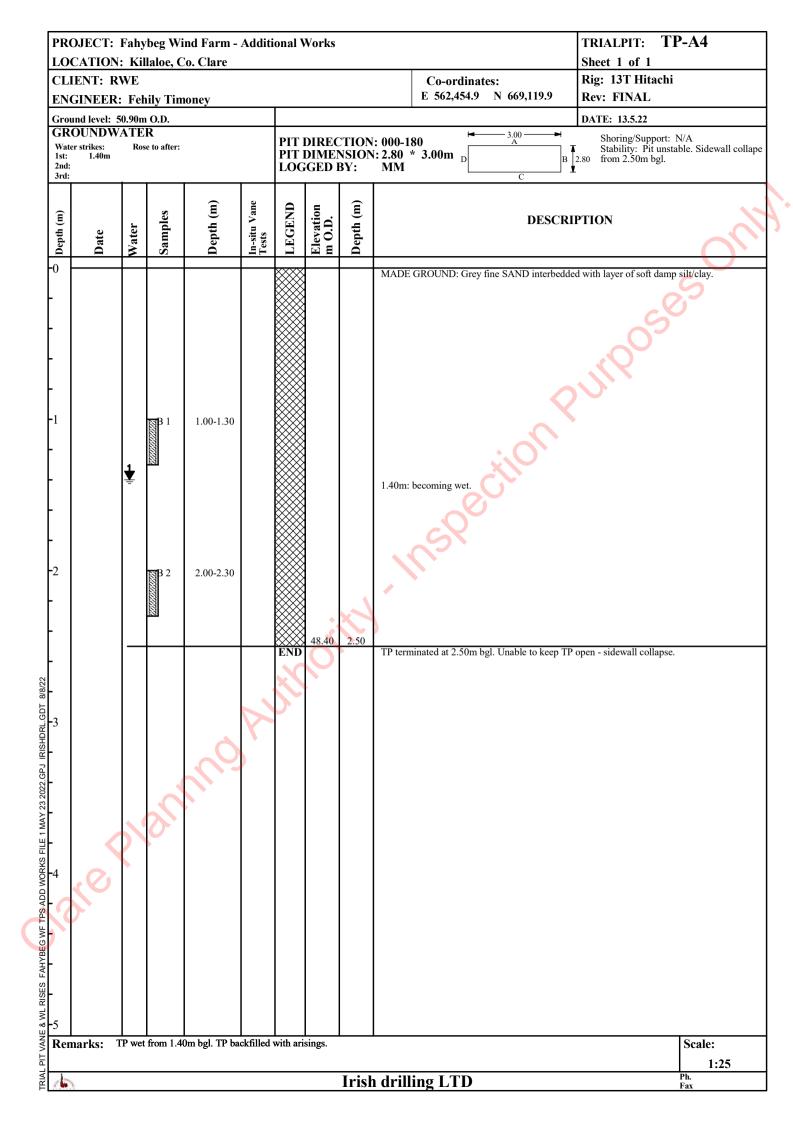


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LOCATION: CLIENT: RV ENGINEER: Ground level: 5 GROUNDWA Water strikes: 1st: 4.10m 2nd: 3rd:	WE <u>Fehi</u> 0.35m ATEF	ily_Tim O.D.							Sheet 1 of 1
Ground level: 5 GROUNDWA Water strikes: 1st: 4.10m 2nd:	0.35m ATEF	O.D.	oney					Co-ordinates:	Rig: 13T Hitachi
GROUNDWA Water strikes: 1st: 4.10m 2nd:	ATEF							E 562,413.7 N 669,079.4	Rev: FINAL DATE: 13.5.22
	·	K to after:			PIT I PIT I LOG	DIREC DIMEI GED I	CTION NSION BY:		Shoring/Support: N/A Stability: Pit unstable. Sidewall collape 0.80 from 2.50m bgl.
Depth (m) Date	Water	Samples	Depth (m)	In-situ Vane Tests	LEGEND	Elevation m O.D.	Depth (m)	DESCH	RIPTION
0						49.85	0.50	ADE GROUND: Grass and shrubs over fir	605
1		833 I	1.00-1.20					ADE GROUND: Very soft wet reddish bro	wn SILT/CLAY with rootlets.
2		32 300000	2.00-2.20			47.85	2.50	30m: becoming greyish brown. ADE GROUND: Very soft greyish brown S edium sand.	SILT/CLAY interbedded with layers of wet grey
3		B ³³	3.00-3.20						
4	↓	13 4	4.10-4.40		END	45.85	4.50	terminated at 4.50m bgl.	
5 Remarks: Ir	ngress	of water a	at 4.10m bgl. '	TP back	filled wi	th arisin	ıgs.		Scale: 1:25

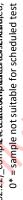


CLIENT: RWE ENGINEER: Fehily TimoneyCo-ordinates: E 562,449.5 N 669,163.8Rig: 13T H Rev: FINA DATE: 13.5.7Ground level: 51.86m O.D. GROUNDWATER Water strikes: Rose to after:DATE: 13.5.7 PIT DIRECTION: 000-180 A Shoring/S Stability:Shoring/S Stability:	RIALPIT: TP-A5 eet 1 of 1	I			Vorks	onal V	Additi	nd Farm -	-	-		
Subscription Date: Date: Ground level: Rose to after: PIT DIRECTION: 000-180 Numer order: Rose to after: PIT DIRECTION: 000-180 Numer order: Rose to after: PIT DIRECTION: 000-180 Numer order: Stability: PIT DIRECTION: 000-180 Numer order: Stability: PIT DIRECTION: 000-180 Numer order: Stability: PIT DIRECTION: 000-180 Stability: Stability: PIT DIRECTION: 000-180 Stability: Stability: PIT DIRECTION: PIT DIRECTION: Stability: Stability: Stability: <t< th=""><th>g: 13T Hitachi</th><th>-</th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th>VE</th><th>ENT: RV</th><th>CLI</th></t<>	g: 13T Hitachi	-								VE	ENT: RV	CLI
GROUNDWATER Inter tridue and: Rose to after: PIT DIRECTION: 000-180 PIT DIMENSION: 1.90 * 3.30m Image: Stability: Image: Stability: Imag			E 562,449.5 N 669,163					oney				
0 B1 0.00-1.50 MADE GROUND: Brown fine SAND interbedded with layer of the sand interbedded with layer of solution interbedded withedded withedded with layer of solution interbedded with	Shoring/Support: N/A Stability: Pit unstable. Sidewall co		* 3.30m _D	DN: 1.90	DIMEN	PIT			R	ATEI	OUNDWA er strikes:	GRO Wate 1st: 2nd:
0 3 3 0.00-1.50 MADE GROUND: Brown fine SAND interbedded with layer of the sand state of the sand sta	ON	DESCRIP	DF		Elevation m O.D.	TEGEND	In-situ Vane Tests	Depth (m)	Samples	Water	Date	Depth (m)
- MADE GROUND: Damp grey fine SAND interbedded with lay brown clay.	ith layer of soft brown silt/clay.	on P'	ctio	0	50.36							- - - - - - -
			clay.	0 MADE		END	2	3.00-3.40	toppon			· · ·
									3		Xe X	4
-5 Remarks: TP dry on excavation. TP backfilled with arisings.	Scale:				ıgs.	th arisi	filled wi	ation. TP back	on excava	P dry o	narks: T	
	1:25											





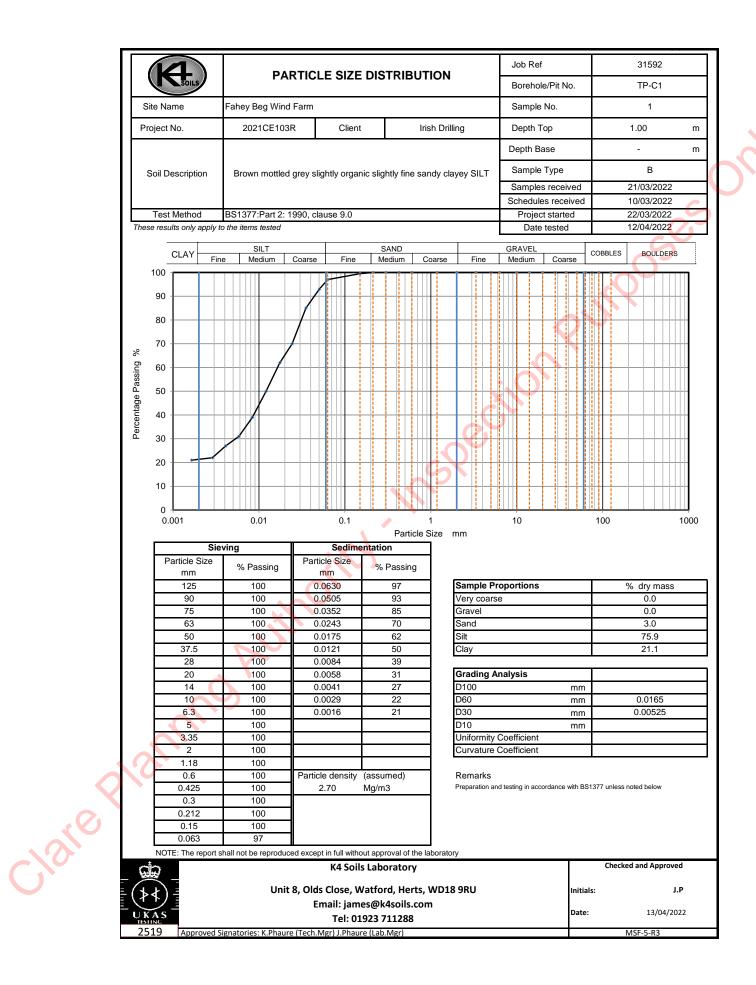
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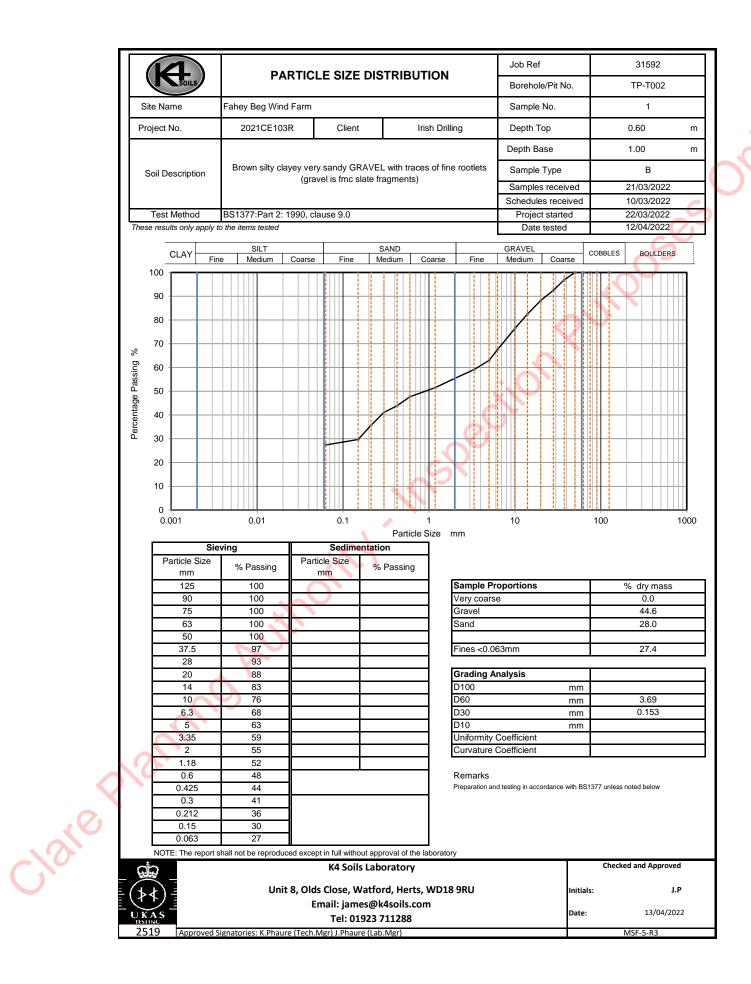


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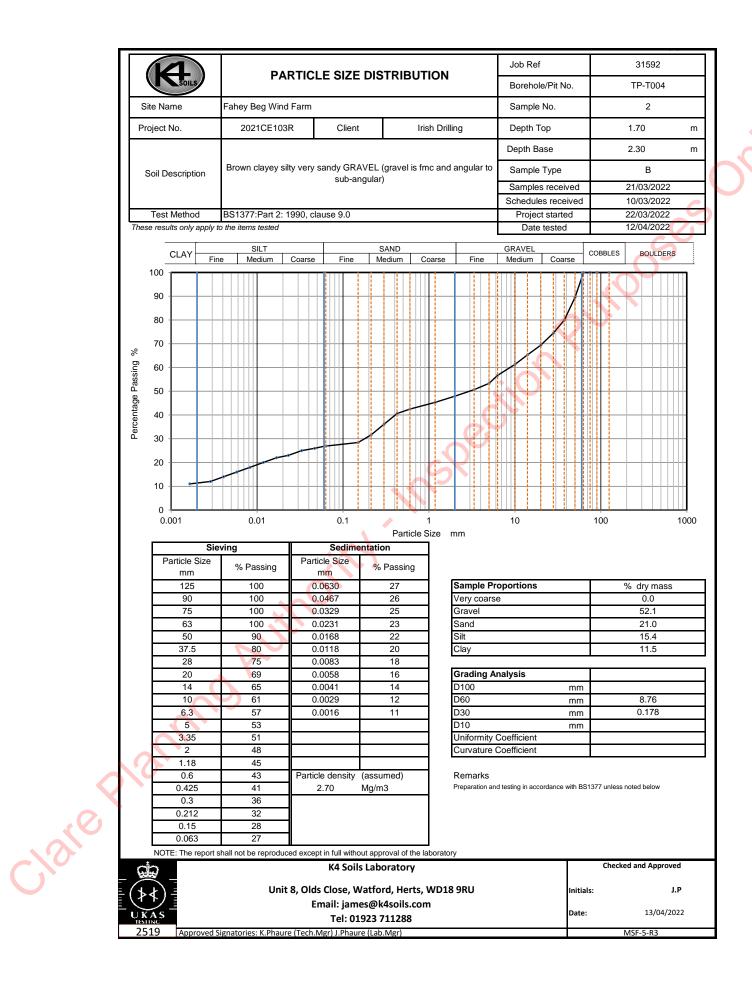
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R/ 19		hem	ample (2;1 water / soil extract) ulphate Total								_				n
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Client Report date Scheduled Date			rganic Content	S											-
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Wind		Sample	ase Depth	8	2.20	2.70	1.00	2.00	1.00	2.30	1 80	2.70	1.50	1.00	si o
Project ID 2021CE103R Project Name Fahey Beg Wind Farm Schedule ID 2021CE103R_1				+	+		╞┼	╈	\uparrow		╉	+	H		Completed in K4 Soils
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Proj ject ľ			ocation		TP-S2	TP-T001	FP-T002	01-7	TP-T004	TP-T004	TP-T005	TP-T006	TP-T007	01-0	
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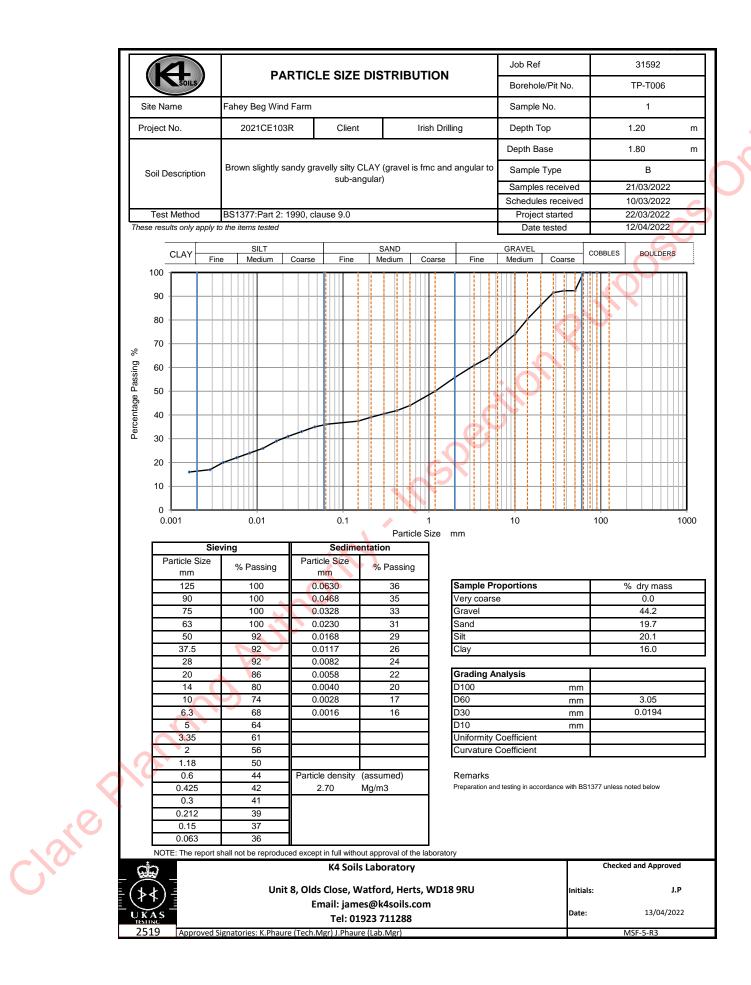
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2021CE 103R Inish Drilling Testing Started Hole No. Sample Soil Description NMC Pairing Started PL PI Remarks TP-C1 1 1.00 - B Brown mottled grey slightly organic slightly fire sandy clayey SLT 34 - - - NMC Only TP-C1 1 0.60 1.00 B Brown mottled grey slightly organic slightly fire sandy clayey SLT 34 - - - NMC Only TP-T002 1 0.60 1.00 B brown slightly sandy gravely slightly organic slate fragments) 34 - - - NMC Only TP-T002 1 1.20 1.80 B Brown slightly sandy gravely slightly CLAY (gravel is fire collets (gravel is fire. 14 - - - NMC Only TP-T008 1 1.20 1.80 B Brown slightly sandy gravely slightly fire - - NMC Only TP-T008 1 1.20 1.80 B Brown slightly sandy gravely slightly fire -					Beg W	ind Farm				Schedule	received	10/03/2022
Sample Parameter of the par												22/03/2022
Hole No. Ref Top Base Type Soil Description NMC Passing W LL PL PI Remats TP-C1 1 1.00 - B Brown motiled grey slightly organic slightly line sandy clayey SLT 34 - - - NMC Only TP-C1 1 0.60 1.00 B Brown motiled grey slightly organic slightly fine sandy clayey very sandy GRAVEL with traces of the rootets (gravelis fine slight regenetis) 14 - - - NMC Only TP-T002 1 0.60 1.00 B Brown slightly sandy gravely slip CLAY angular) 14 - - - NMC Only TP-T006 1 1.20 1.80 B Gravelis fine and angular to sub- angular) 13 - - - NMC Only TP-T006 1 1.20 1.8 I Image: I	2021	CE103	R	Irish Dri	illing					Testing S	tarted	
Reif Top Base Type Addition St. St. <th< th=""><th>Hole No</th><th></th><th>San</th><th>nple</th><th></th><th>Soil Description</th><th>NMC</th><th></th><th>LL</th><th>PL</th><th>PI</th><th>Pemarke</th></th<>	Hole No		San	nple		Soil Description	NMC		LL	PL	PI	Pemarke
TP-C1 1 1.00 · B Brown motiled grey slightly organic slightly organic slightly fine sandy clayey SiLT 34 · · · · NMC Only TP-T002 1 0.60 1.00 B Brown silty clayey very sandy GRAVEL state fragments) 14 · · · · NMC Only TP-T002 1 0.60 1.00 B Brown silty clayey very sandy GRAVEL state fragments) 14 · · · · NMC Only TP-T006 1 1.20 1.80 B Brown silty sandy gravely silty CLAY angular to sub-angular to sub-angular) 13 · · · · NMC Only TP-T006 1 1.20 1.80 B Brown silty fine sandy gravely silty CLAY angular to sub-angular to sub-angular) 13 · · · · NMC Only T 1 1 1 Image: Silty silty clayer silty c	Hole No.	Ref			Туре	Soli Description	9/		9/	9/	9/	Kellidiks
TP-T002 1 0.80 1.00 B with traces of fine rootiets (gravel is fmc later fragments) 14 - - - NMC Only TP-T006 1 1.20 1.80 B Brown slightly sandy gravelly slity CLAY gravells fmc and angular to sub-angular) 13 - - - NMC Only TP-T006 1 1.20 1.80 B Brown slightly sandy gravelly slity CLAY gravells fmc and angular to sub-angular) 13 - - - NMC Only TP-T006 1 1.20 1.80 B Brown slightly sandy gravelly slity CLAY gravells fmc and angular to sub-angular) 13 - - - NMC Only TP-T006 1 1.80 B <td>TP-C1</td> <td>1</td> <td></td> <td></td> <td>в</td> <td>Brown mottled grey slightly organic slightly fine sandy clayey SILT</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>NMC Only</td>	TP-C1	1			в	Brown mottled grey slightly organic slightly fine sandy clayey SILT						NMC Only
TP-T006 1 1.20 1.80 B Brown slightly sandy gravelly slity CLAY (graveli is fine and angular to sub-angular) 13 . . NMC Only TP-T006 1 1.20 1.80 B Graveli is fine and angular to sub-angular to sub-angular) 13 . . . NMC Only Image: Strength of the strengt of the strength of the strength of the strengend of the strengen	TP-T002	1	0.60	1.00	в	with traces of fine rootlets (gravel is fm		-	-		S.C	NMC Only
	TP-T006	1	1.20	1.80	В	(gravel is fmc and angular to sub-		-	~	-	-	NMC Only
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	<i>⊳</i> √2											
	â	Natural	Moisture	Content	: clause	e 3.2 Te	st Report by	K4 SOILS	LABOR	ATORY		Checked and Approved
Test Methods: BS1377: Part 2: 1990: Natural Moisture Content : clause 3.2 Checked and Test Report by K4 SOILS LABORATORY Checked and Approved	$\overline{\mathbf{A}}$.0	Unit 8 Olds	Close Old	ls Appro	ach		Initials J.P
Test Methods: BS1377: Part 2: 1990: Natural Moisture Content : clause 3.2 Test Report by K4 SOILS LABORATORY Checked and Approved Atterberg Limits: clause 4.3 and 5.0 Unit 8 Olds Close Olds Approach Initials J.P		NOTE:	The repo	t shall no	t be rep	produced except in full	Tel:	01923 71 [.]	1 288			Date: 13/04/202
Natural Moisture Content : clause 3.2 Atterberg Limits: clause 4.3 and 5.0 These results only apply to the items tested NOTE: The report shall not be repreduced event in full NOTE: The report shall not be repreduced event in full NOTE: The report shall not be repreduced event in full NOTE: The report shall not be repreduced event in full Note: 01923 711 288 Note: 01923 711 288 Note: 01923 711 288	UKAS TESTING 2519			of the lab		re (Tech.Mgr) J.Phaure (Lab.Mgr)	Email: Ja	mes@k4	soils.coi	n		MSF-5-R1(b)



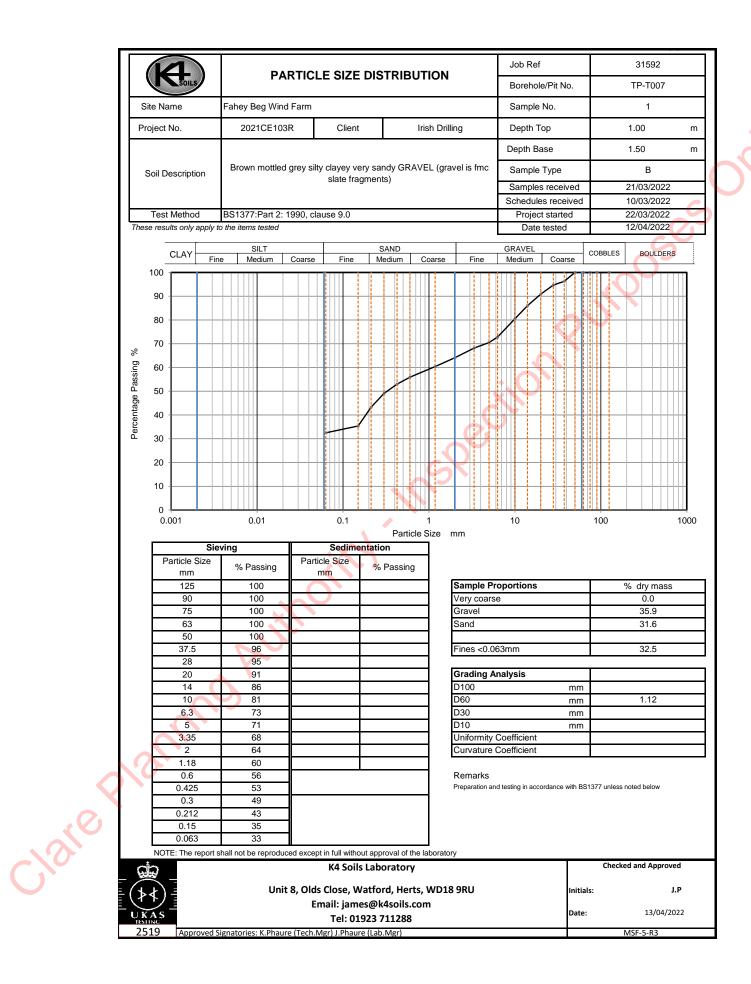


										Job Ref		31592	
	SOIL		PA	RTIC	LE SIZ	ZE DI	STRI	BUTIO	N	Borehole/Pit N	No.	TP-T003	
		-		_									
Si	ite Name		Fahey Beg Wind	Farm						Sample No.		1	
Pro	oject No.		2021CE103	R	Cli	ent		Irish D	rilling	Depth Top		1.60	m
										Depth Base		2.00	m
			Brown slightly me	ottled g	rey sligh	itly sand	dy grav	elly silty C	LAY (gravel is	Sample Type		В	
5	Soil Descript	ion	0,	fmc a	nd angu	ular to s	ub-rour	nded)	10	Samples recei		21/03/2022	,
										Schedules rece		10/03/2022	
	Test Metho	d	BS1377:Part 2: 1	990, cl	ause 9.0)				Project starte		22/03/2022	
Thes	e results only	apply t	o the items tested							Date tested	d	12/04/2022	
			SILT				SAND			GRAVEL	0	OBBLES BOULDER	
	CLAY	Fin	e Medium	Coarse	Fir	ne	Medium	Coars	e Fine	Medium Coa	arse	OBBLES BOULDEF	50
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	5	-	73						D30		mm		
	3.35	5	69							Coefficient			
	2		64							Coefficient			
$\langle Q \rangle$	1.18	3	60										
1	0.6		55]	Remarks				
•	0.42	5	53						Preparation a	nd testing in accordance	with BS13	// unless noted below	
	0.3	2	49 47										
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			shall not be reproduc	ed excer	ot in full w	/ithout a	oproval o	of the labora	atory				
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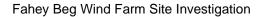
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	SOILS		F 7					SUTIC	'IN		Boreho	ole/Pit No.		TP-T006	
Site N	ame		Fahey Beg Win	d Farm							Sample	e No.		2	
Project	t No.		2021CE10	3R	Clie	nt		Irish I	Drilling		Depth	Тор		2.10	m
					-						Depth E	Base		2.70	m
Soil [Descriptio	'n	Brown silty clay						rootlets (gravel	Sample	е Туре		В	
	·			is fmo	c and angu	lar to s	sub-an	jular)				es received		21/03/2022	
Tes	t Method		BS1377:Part 2:	1990 c	lause 9.0							les received ct started	-	10/03/2022 22/03/2022	
			the items tested									e tested		12/04/2022	
	CLAY		SILT				SAND				GRAVEL		COBBLE	S BOULDER	s
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	Particle S	Siev	ing	Dor		nentati	on								
	mm	ize	% Passing	Fai	ticle Size mm	%	Passi	ng							
	125 90		100							ple Pr	oportions	i		% dry mass 0.0	;
	75		100						Grav		6			51.6	
Ⅰ ⊢	63 50		100 86						Sand	b				15.4	
	37.5		82						Fine	s <0.06	63mm			33.0	
Ⅰ ⊢	28 20		80 75			_			Grad	lina A	nalysis		1		
	14		72						D10	0	laiyolo	mm			
	10 6.3		68 63			_			D60 D30			mm mm	-	5.71	
	5		57						D10			mm	_		
	3.35 2		53 48	-							Coefficient Coefficient		_		
	1.18		45						our	aturo	ocomolorit				
	0.6		41 40	-						arks	d testing in ac	cordance with B	S1377 unle	ess noted below	
	0.420		38								<u>9</u>				
	0.212		36 34	-											
	0.15		34 33												
	TE: The re	port sh	all not be reprodu	ced exce					atory				~	aluard a surf a	
					K4 So	ils Lab	orato	ry					Che	cked and Approv	red
-(≯≮)			Uni		ds Close,				018 9RL	J		Initi	als:		J.P
				E	Email: jaı Tel· (nes@)1923						Date	:	13/04/2	022
2519	Appro	ved Sig	gnatories: K.Phau	re (Tech				-						MSF-5-R3	



	2		DA	DTIC	LE SIZ		וסוסי		I	Job Ref		31592	
	Soils	/	F <i>P</i>			E DIS I	KID			Borehole/Pit I	No.	TP-T00	8
Sit	te Name	F	ahey Beg Win	d Farm						Sample No.		1	
Pro	oject No.		2021CE103	3R	Clie	nt		Irish Dri	lling	Depth Top		0.50	m
										Depth Base		1.00	m
s	oil Descripti	on	Brown silty cla						ine rootlets	Sample Type		В	
				(gra	avel is fmc	slate frag	gments	5)		Samples rece	eived	21/03/20	22
										Schedules rec		10/03/20	
	Test Methor		BS1377:Part 2: the items tested	1990, c	clause 9.0					Project star		22/03/202 12/04/202	
			SILT		-	54	ND			GRAVEL			
	CLAY	Fine		Coarse	e Fine		lium	Coarse	Fine		arse	COBBLES BOULD	DERS
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	90			_									
	80												
%	70												
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V	SO	ILS			Tested in accordance with BS1377 : Part 3 : 20		se 7.0 &		
Job No.			Project N					Samples re	Programme received 21/03/2022
31592			Fahey B	Beg Wind I	Farm			Samples re Schedule r	
Project No	0.		Client					Project s	started 22/03/2022
2021CE10	03R		Irish Drill	ling				Testing S	Started 11/04/2022
		s	ample			Dry Mass			
Hole No.	Ref	Тор	Base	Туре	Soil description	passing 2mm	SO4 Content	рН	Remarks
		m	m	├──		%	mg/l		
TP-T002	1	0.60	1.00		Brown silty clayey very sandy GRAVEL with traces of fine rootlets (gravel is fmc slate fragments)	55	320	7.85	
TP-T006	1	1.20	1.80	В	Brown slightly sandy gravelly silty CLAY (gravel is fmc and angular to sub-angular)	56	210	7.82	
TP-T007	1	1.00	1.50		Brown mottled grey silty clayey very sandy GRAVEL (gravel is fmc slate fragments)	64	270	7.83	
					SPE				
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0	0								
					Test Report by K4 SOILS LABORATORY Unit 8 Olds Close Olds Approach Watford Herts WD18 9RU Tel: 01923 711 288 Email: James@k4soils.com These results only apply to the items tested NOTE: The report shall not be reproduced except in full without authority of the labor.	,			Checked and Approved Initials J.P Date: 13/04/202

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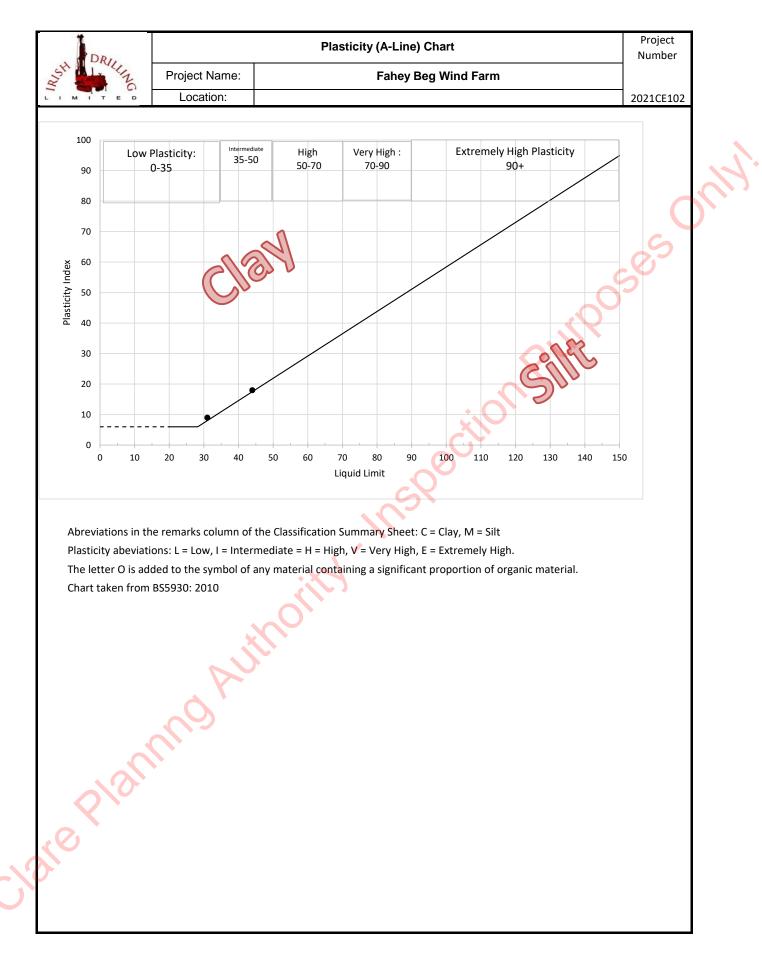


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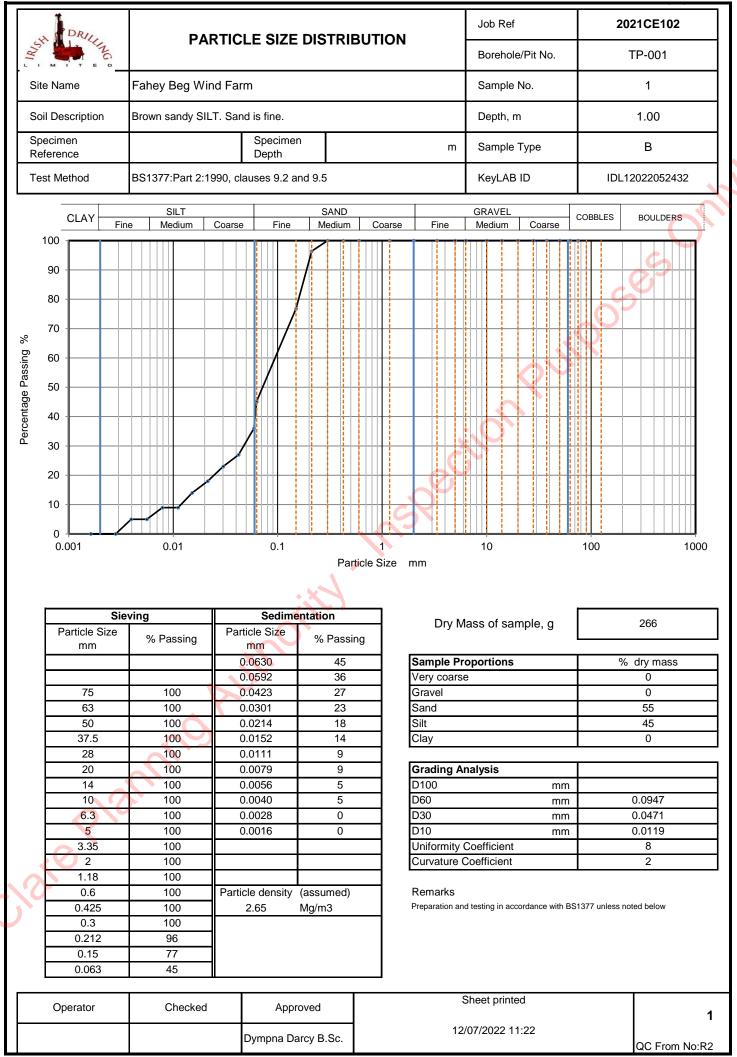
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roject No.			Project	Name											
2021	CE102				1				g Wind	Farm		1	-		
Hole No.	Ref	Sar Top	nple Base	Туре		Soil Description	Dens bulk	ity dry	W	Passing 425µm	LL	PL	PI	Particle density	Remarks
		100	Dusc	Type			Mg/n	n3	%	%	%	%	%	Mg/m3	
TP-001	1	1.00	1.30	В		Brown sandy SILT. Sand is fine.			25.0	100					
TP-002	2	2.00	2.30	В		Brown SAND.			37.0	100					NP
TP-003	2	2.00	2.20	В		Brown SILT			44.0	100	44	26	18		М
TP-003	4	4.10	4.40	в		Brown slightly sandy SILT. Sand is fine.			30.0	100	31	22	9		CL
TP-004	1	1.00	1.30	в		Brown very silty fine and medium SAND.			17.0	98					C)
TP-005	1	0.00	1.50	в		Brown slightly sandy SILT.			47.0	99				0	
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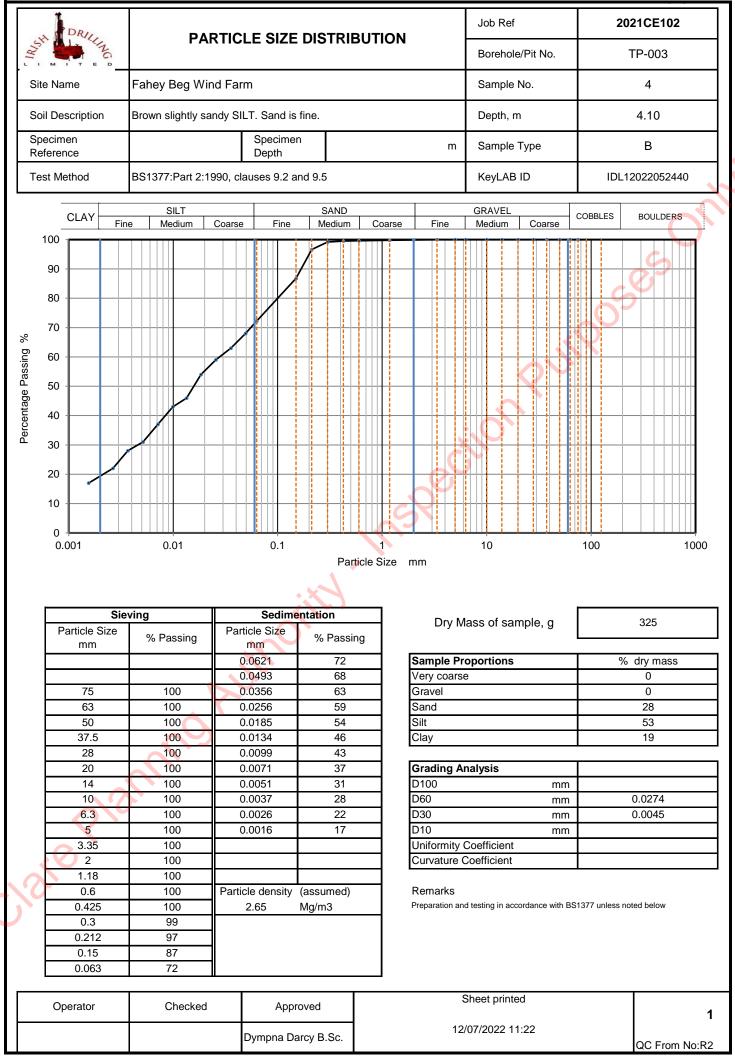
Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39 Approved Signatures: Dympna Darcy (DCD) Lab Manager, Declan Joyce (DJ) Chartered Geotechnical Engineer, Ronan Killeen (RK) Quality Manager.



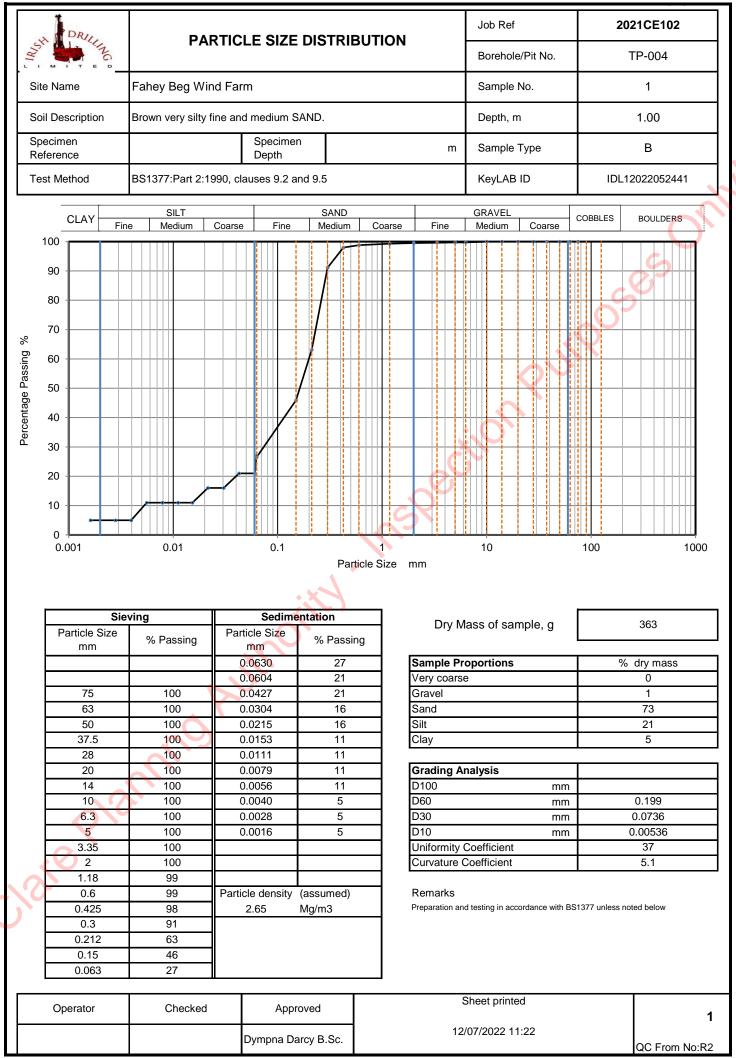
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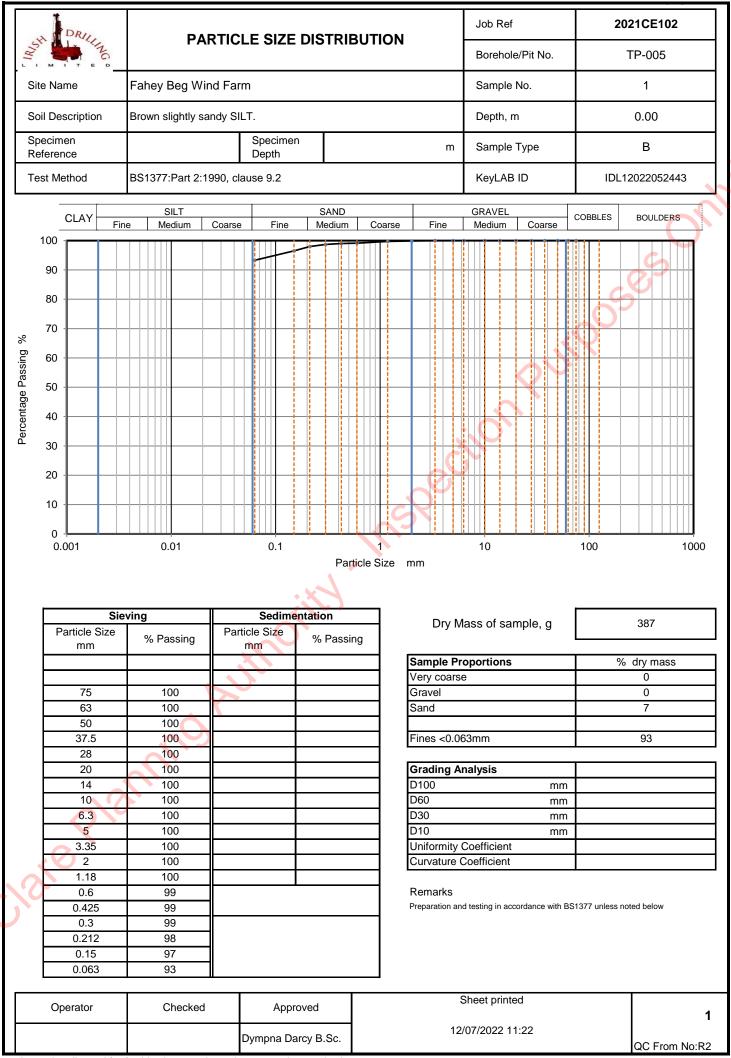
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Tested in: Irish Drilling Ltd. (IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39



Tested in: Irish Drilling Ltd.(IDL), Old Galway Road, Loughrea, Co. Galway, Ireland. H62VX39



Irish Drilling Limited Old Galway Road Loughrea Co. Galway

Attention: Dympna Darcy

Unit 7-8 Hawarden Business Park Manor Road (off Manor Lane) Hawarden Deeside CH5 3US Tel: (01244) 528777 email: hawardencustomerservices@alsglobal.com Website: www.alsenvironmental.co.uk

urposes

CERTIFICATE OF ANALYSIS

Date of report Generation: Customer: Sample Delivery Group (SDG): Your Reference: Location: Report No: Order Number: 13 July 2022 Irish Drilling Limited 220706-92 2021CE102 Faheybeg WF 654229 11407

We received 4 samples on Wednesday July 06, 2022 and 4 of these samples were scheduled for analysis which was completed on Wednesday July 13, 2022. Accredited laboratory tests are defined within the report, but opinions, interpretations and on-site data expressed herein are outside the scope of ISO 17025 accreditation.

Should this report require incorporation into client reports, it must be used in its entirety and not simply with the data sections alone.

Chemical testing (unless subcontracted) performed at ALS Life Sciences Ltd Hawarden.

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All sample data is provided by the customer. The reported results relate to the sample supplied, and on the basis that this data is correct.

Incorrect sampling dates and/or sample information will affect the validity of results. The customer is not permitted to reproduce this report except in full without the approval of the laboratory.

Approved By:

Sonia McWhan Operations Manager



ALS Life Sciences Limited. Registered Office: Units 7 & 8 Hawarden Business Park, Manor Road, Hawarden, Deeside, CH5 3US. Registered in England and Wales No. 4057291. Version: 3.3 Version Issued: 13/07/2022

CERTIFICATE OF ANALYSIS



SDG: 220706-92 Client Ref.: 2021CE102

Report Number: 654229 Location: Faheybeg WF Superseded Report:

Validated

Received Sample Overview

b Sample No(s)	Customer Sample Ref.	AGS Ref.	Depth (m)	Sampled Date
26546501	TP001	B1	1.00 - 1.30	13/05/2022
26546507	TP002	B2	2.00 - 2.30	13/05/2022
26546511	TP003	B4	4.10 - 4.40	13/05/2022
26546514	TP004	B1	1.00 - 1.30	13/05/2022
<pre>/ received samples which</pre>	have had analysis scheduled will b	e shown on the following pa	ges.	
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### **CERTIFICATE OF ANALYSIS** Report Number: 654229

Location: Faheybeg WF

SDG: 220706-92

Client Ref.: 2021CE102

**Results Legend** 

Page 3 of 8

Validated

Superseded Report:

### **CERTIFICATE OF ANALYSIS**

SDG: 220706-92 Client Ref.: 2021CE102 Report Number: 654229 Location: Faheybeg WF Superseded Report:

Validated

## Sample Descriptions

very fine <0.0	063mm fine 0.	063mm - 0.1mm	medium 0.1m	n - 2mm coa	rse 2mm - 1	0mm very coarse	>10m
Lab Sample No(s)	Customer Sample Ref.	Depth (m)	Colour	Description	Inclusions	Inclusions 2	
26546501	TP001	1.00 - 1.30	Light Brown	Loamy Sand	Vegetation	None	
26546507	TP002	2.00 - 2.30	Light Brown	Silty Clay	None	None	
26546511	TP003	4.10 - 4.40	Light Brown	Silt Loam	None	None	4
26546514	TP004	1.00 - 1.30	Light Brown	Sand	Stones	None	

These descriptions are only intended to act as a cross check if sample identities are questioned, and to provide a log of sample matrices with respect to MCERTS validation. They are not intended as full geological descriptions.

We are accredited to MCERTS for sand, clay and loam/topsoil, or any of these materials - whether these are derived from naturally ocurring soil profiles, or from fill/made ground, as long as these materials constitute the major part of the sample.

Other coarse granular materials such as concrete, gravel and brick are not accredited if they comprise the major part of the

### **CERTIFICATE OF ANALYSIS**

Validated

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	SDG: 220706- Ref.: 2021CE1			Report Number: Location:	654229 Faheybeg WF	Superseded	Report:	
Results Legend # ISO17025 accredited.	C	ustomer Sample Ref.	TP001	TP002	TP003	TP004		
M mCERTS accredited. aq Aqueous / settled sample. diss.filt Dissolved / filtered sample. tot.unfilt Total / unfiltered sample. * Subcontracted - refer to subcontractor report	for	Depth (m) Sample Type	1.00 - 1.30 Soil/Solid (S)	2.00 - 2.30 Soil/Solid (S)	4.10 - 4.40 Soil/Solid (S)	1.00 - 1.30 Soil/Solid (S)		
accreditation status. ** % recovery of the surrogate standard to chec efficiency of the method. The results of indivi	k the dual	Date Sampled Sample Time Date Received	13/05/2022 06/07/2022	13/05/2022 06/07/2022	13/05/2022 06/07/2022	13/05/2022 06/07/2022		
compounds within samples aren't corrected f recovery	or the	SDG Ref Lab Sample No.(s)	220706-92 26546501	220706-92 26546507	220706-92 26546511	220706-92 26546514		
(F) Trigger breach confirmed 1-4+§@ Sample deviation (see appendix)		AGS Reference	B1	B2	B4	B1		
omponent	LOD/Units	Method						
isture Content Ratio (% of as eived sample)	%	PM024	19	22	23	15		
l Organic Matter (SOM)	<0.35 %	TM132		0.795 @#				
	1 pH Units	TM133	8.65 @ M	7.76 @ M	8.41	8.3 @ M		6
ter Soluble Sulphate as SO4 2:1 tract	<0.004 g/l	TM243	0.01 @ M	0.0279	0.0141	0.0069 @ M	C	2
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### **CERTIFICATE OF ANALYSIS**

Report Number: 654229 Location: Faheybeg WF Superseded Report:

# **Table of Results - Appendix**

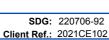
Method No	Reference	of Results - Appendix Description
PM024	Modified BS 1377	Soil preparation including homogenisation, moisture screens of soils for Asbestos Containing
		Material
TM132 TM133	In - house Method BS 1377: Part 3 1990;BS 6068-2.5	ELTRA CS800 Operators Guide Determination of pH in Soil and Water using the GLpH pH Meter
	60 1011. Fail 0 1330,60 0000-2.0	
TM243 ot applicable. al testing (unless	s subcontracted) performed at ALS Life Sciences Ltd Hawa	arden (Method codes TM).
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SDG: 220706-92

Client Ref.: 2021CE102

### **CERTIFICATE OF ANALYSIS** Report Number: 654229

Superseded Report:



# **Test Completion Dates**

	SDG: 220706-9 nt Ref.: 2021CE10				Number: 654 .ocation: Fat	
			Tes	st Com	pletion	Dates
Lab	Sample No(s)	26546501	26546507	26546511	26546514	
Custome	r Sample Ref.	TP001	TP002	TP003	TP004	
	AGS Ref.	B1	B2	B4	B1	
	Depth	1.00 - 1.30	2.00 - 2.30	4.10 - 4.40	1.00 - 1.30	
	Туре	Soil/Solid (S)	Soil/Solid (S)	Soil/Solid (S)	Soil/Solid (S)	
ons by Kone (soil)		13-Jul-2022 11-Jul-2022	12-Jul-2022 11-Jul-2022	13-Jul-2022 08-Jul-2022	13-Jul-2022 11-Jul-2022	
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### **CERTIFICATE OF ANALYSIS**



220706-92 2021CE102 Report Number: 654229 Location: Faheybeg WF Superseded Report:

### Appendix

### General

1. Results are expressed on a dry weight basis (dried at 35°C) for all soil analyses except for the following: NRA and CEN Leach tests, flash point LOI, pH, ammonium as NH4 by the BRE method, VOC TICs and SVOC TICs.

2. If sufficient sample is received a sub sample will be retained free of charge for 30 days after analysis is completed (e-mailed) for all sample types unless the sample is destroyed on testing. The prepared soil sub sample that is analysed for asbestos will be retained for a period of 6 months after the analysis date. All bulk samples will be retained for a period of 6 months after the analysis date. All samples received and not scheduled will be disposed of one month after the date of receipt unless we are instructed to the contrary. Once the initial period has expired, a storage charge will be applied for each month or part thereof until the client cancels the request for sample storage. ALS reserve the right to charge for samples received and stored but not analysed.

3. With respect to turnaround, we will always endeavour to meet client requirements wherever possible, but turnaround times cannot be absolutely guaranteed due to so many variables beyond our control.

4. We take responsibility for any test performed by sub-contractors (marked with an asterisk). We endeavour to use UKAS/MCERTS Accredited Laboratories, who either complete a quality questionnaire or are audited by ourselves. For some determinands there are no UKAS/MCERTS Accredited Laboratories, in this instance a laboratory with a known track record will be utilised.

5. If no separate volatile sample is supplied by the client, or if a headspace or sediment is present in the volatile sample, the integrity of the data may be compromised. This will be flagged up as an invalid VOC on the test schedule and the result marked as deviating on the test certificate.

6. NDP - No determination possible due to insufficient/unsuitable sample.

7. Results relate only to the items tested.

8. LoDs (Limit of Detection) for wet tests reported on a dry weight basis are not corrected for moisture content.

9. **Surrogate recoveries** - Surrogates are added to your sample to monitor recovery of the test requested. A % recovery is reported, results are not corrected for the recovery measured. Typical recoveries for organics tests are 70-130%. Recoveries in soils are affected by organic rich or clay rich matrices. Waters can be affected by remediation fluids or high amounts of sediment. Test results are only ever reported if all of the associated quality checks pass; it is assumed that all recoveries outside of the values above are due to matrix affect.

10. Stones/debris are not routinely removed. We always endeavour to take a representative sub sample from the received sample.

11. In certain circumstances the method detection limit may be elevated due to the sample being outside the calibration range. Other factors that may contribute to this include possible interferences. In both cases the sample would be diluted which would cause the method detection limit to be raised.

12. For dried and crushed preparations of soils volatile loss may occur e.g volatile mercury.

13. For leachate preparations other than Zero Headspace Extraction (ZHE) volatile loss may occur.

14. For the BSEN 12457-3 two batch process to allow the cumulative release to be calculated, the volume of the leachate produced is measured and filtered for all tests. We therefore cannot carry out any unfiltered analysis. The tests affected include volatiles GCFID/GCMS and all subcontracted analysis.

15. Analysis and identification of specific compounds using GCFID is by retention time only, and we routinely calibrate and quantify for benzene, toluene, ethylbenzenes and xylenes (BTEX). For total volatiles in the C5-C12 range, the total area of the chromatogram is integrated and expressed as ug/kg or ug/l. Although this analysis is commonly used for the quantification of gasoline range organics (GRO), the system will also detect other compounds such as chlorinated solvents, and this may lead to a falsely high result with respect to hydrocarbons only. It is not possible to specifically identify these non-hydrocarbons, as standards are not routinely run for any other compounds, and for more definitive identification, volatiles by GCMS should be utilised.

16. We are accredited to MCERTS for sand, clay and loam/topsoil, or any of these materials - whether these are derived from naturally occurring soil profiles, or from fill/made ground, as long as these materials constitute the major part of the sample. Other coarse granular material such as concrete, gravel and brick are not accredited if they comprise the major part of the sample.

17 Data retention. All records, communications and reports pertaining to the analysis are archived for seven years from the date of issue of the final report.

18. Tentatively Identified Compounds (TICs) are non-target peaks in VOC and SVOC analysis. All non-target peaks detected with a concentration above the LoD are subjected to a mass spectral library search. Non-target peaks with a library search confidence of >75% are reported based on the best mass spectral library match. When a non-target peak with a library search confidence of <75% is detected it is reported as "mixed hydrocarbons". Non-target compounds identified from the scan data are semi-quantified relative to one of the deuterated internal standards, under the same chromatographic conditions as the target compounds. This result is reported as a semi-quantitative value and reported as Tentatively Identified Compounds (TICs). TICs are outside the scope of UKAS accreditation and are not moisture corrected.</p>

### 19. Sample Deviations

If a sample is classed as deviated then the associated results may be compromised.

	Container with Headspace provided for volatiles analysis
2	Incorrect container received
3	Deviation from method
4	Matrix interference
•	Sample holding time exceeded in laboratory
(a)	Sample holding time exceeded due to late arrival of instructions or samples
§	Sampled on date not provided

### 20. Asbestos

When requested, the individual sub sample scheduled will be analysed in house for the presence of asbestos fibres and asbestos containing material by our documented in house method TM048 based on HSG 248 (2021), which is accredited to ISO17025. If a specific asbestos fibre type is not found this will be reported as "Not detected". If no asbestos fibre types are found all will be reported as "Not detected" and the sub sample analysed deemed to be clear of asbestos. If an asbestos fibre type is found it will be reported as detected (for each fibre type found). Testing can be carried out on asbestos positive samples, but, due to Health and Safety considerations, may be replaced by alternative tests or reported as No Determination Possible (NDP). The quantity of asbestos present is not determined unless specifically requested.

### Identification of Asbestos in Bulk Materials & Soils

The results for identification of asbestos in bulk materials and soils are obtained from supplied bulk materials and soils which have been examined to determine the presence of asbestos fibres using ALS (Hawarden) in-house method of transmitted/polarised light microscopy and central stop dispersion staining, based on HSG 248 (2021).

The results for identification of asbestos in soils are obtained from a homogenised sub sample which has been examined to determine the presence of asbestos fibres using ALS (Hawarden) in-house method of transmitted/polarised light microscopy and central stop dispersion staining.

Asbe stos Type	Common Name		
Chrysof le	WhiteAsbestos		
Amosite	Brow n Asbestos		
Cio d dolite	Blue Asbe stos		
Fibrous Act nolite	-		
Fibrous Anthophyllite	-		
Fibrous Tremolite	-		

### Visual Estimation Of Fibre Content

Estimation of fibre content is not permitted as part of our UKAS accredited test other than: - Trace - Where only one or two asbestos fibres were identified.

### Respirable Fibres

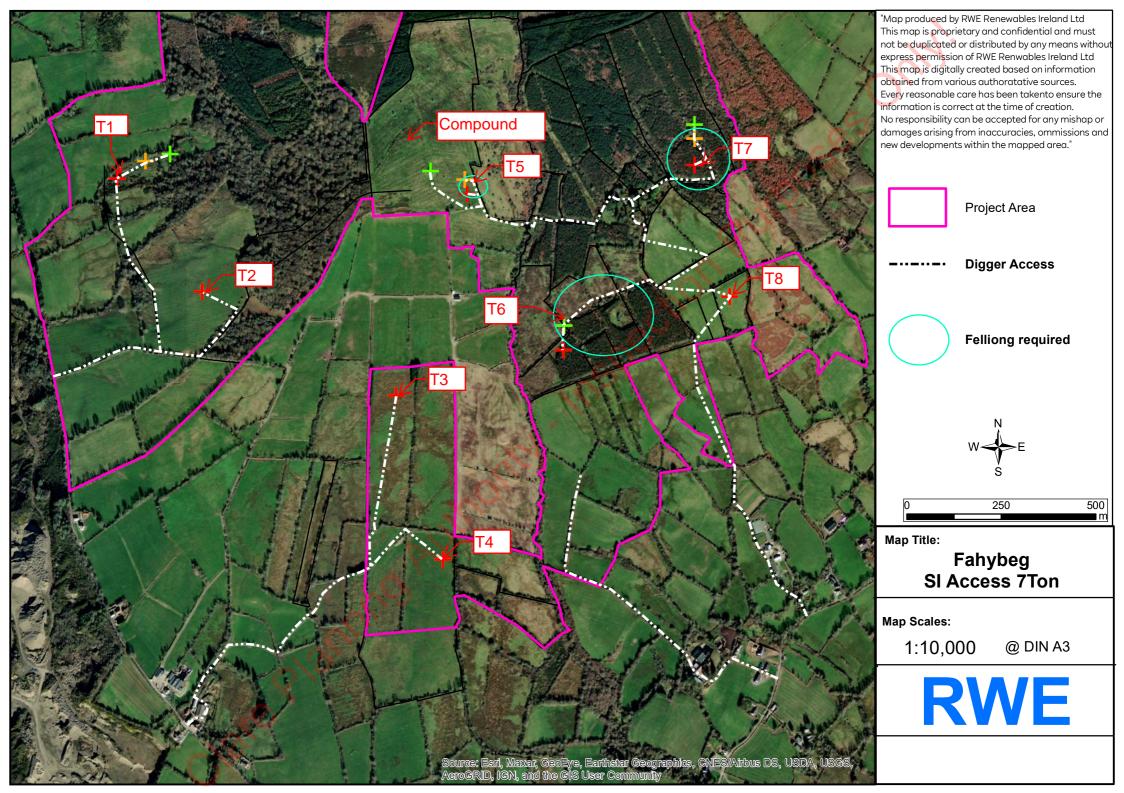
Respirable fibres are defined as fibres of <3  $\mu$ m diameter, longer than 5  $\mu$ m and with aspect ratios of at least 3:1 that can be inhaled into the lower regions of the lung and are generally acknowledged to be most important predictor of hazard and risk for cancers of the lung.

Further guidance on typical asbestos fibre content of manufactured products can be found in HSG 264.

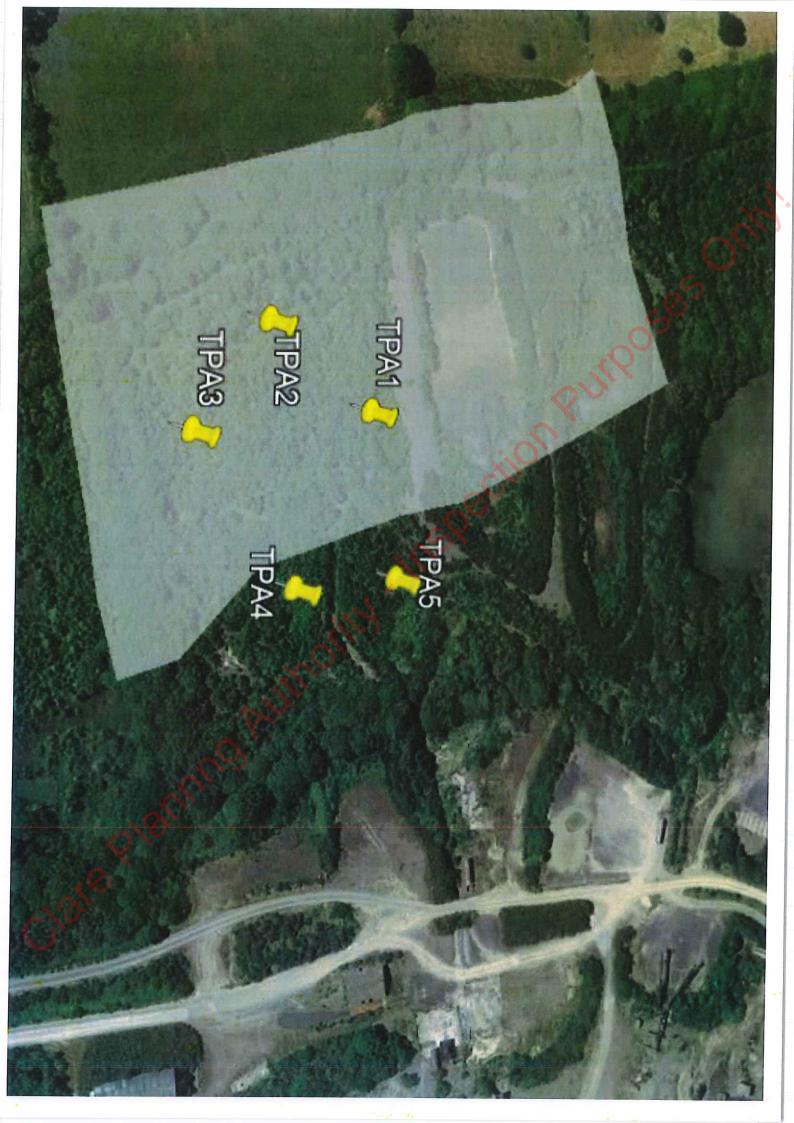
The identification of asbestos containing materials and soils falls within our schedule of tests for which we hold UKAS accreditation, however opinions, interpretations and all other information contained in the report are outside the scope of UKAS accreditation.

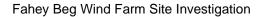


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Figure 1 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-1 (1).JPG



Figure 2 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-1 (2).JPG



Figure 3 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-2 (1).JPG



Figure 4 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-2 (2).JPG



Figure 5 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-3 (1).JPG



Figure 6 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-3 (2).JPG

# Irish Drilling Ltd: Trial Pit Photos:



Figure 7 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-4 (1).JPG



Figure 10 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-5 (2).JPG



Figure 8 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-4 (2).JPG



Figure 9 H:\21CE102.Fahey Beg Windfarm New sub-station site\TP-5 (1).JPG



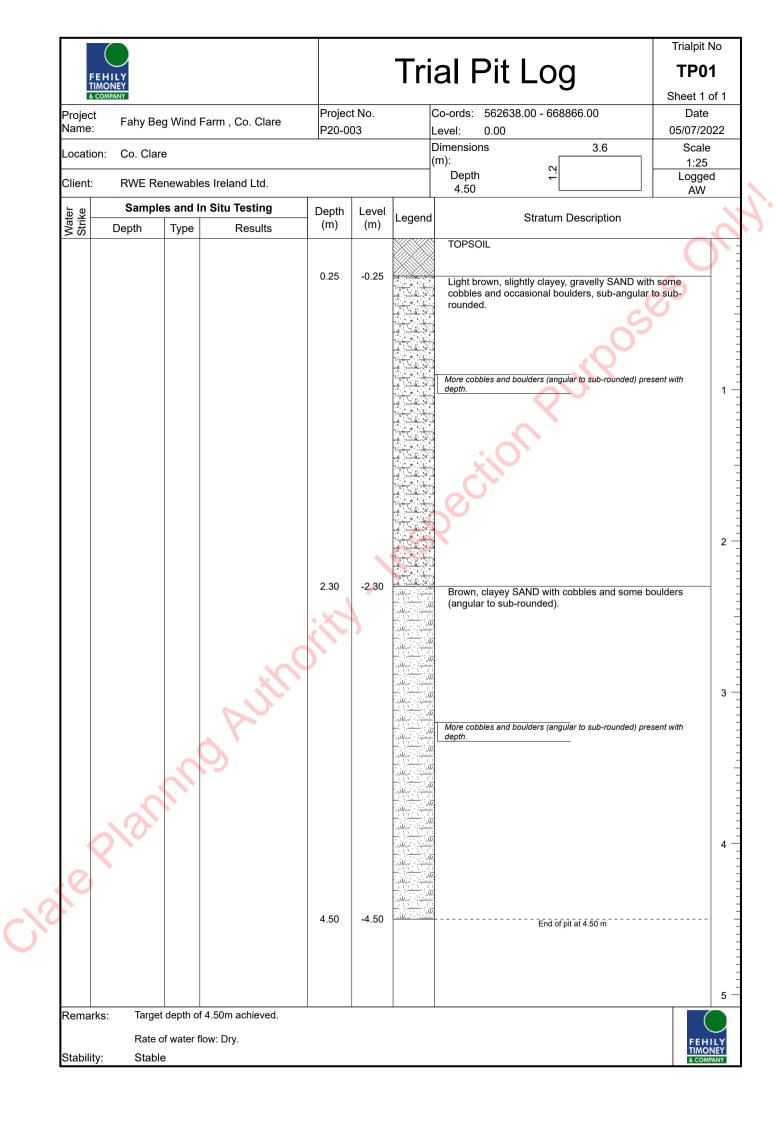
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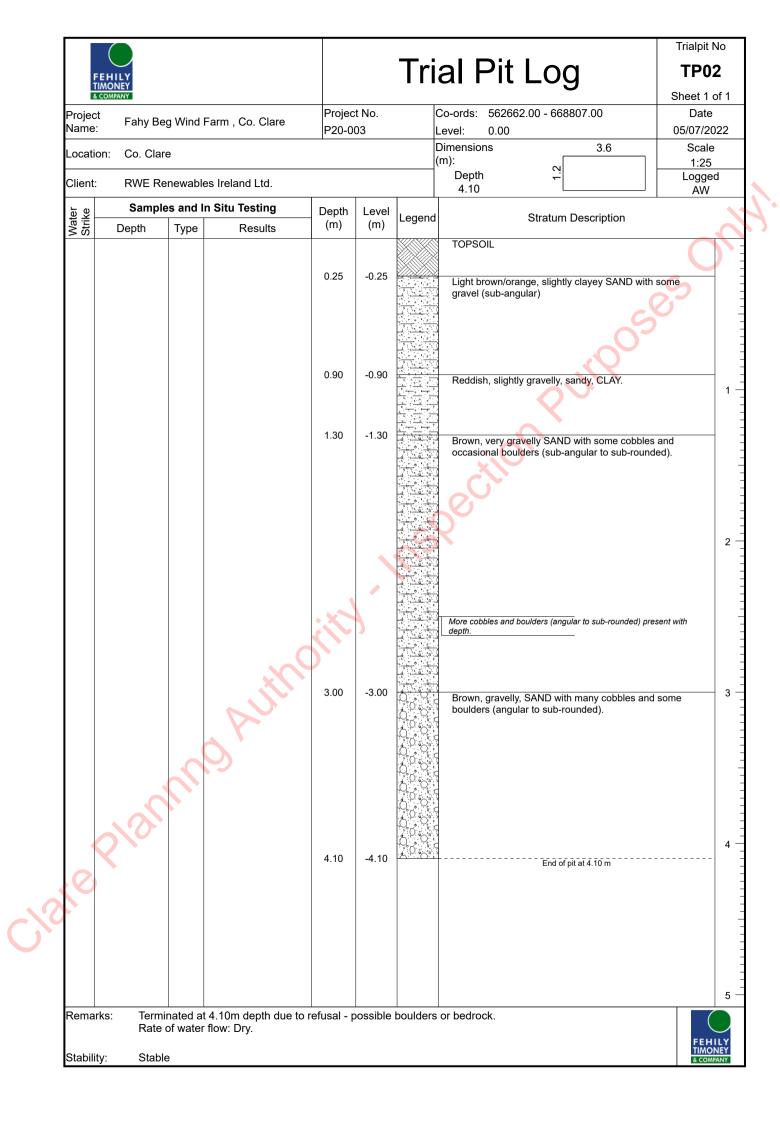


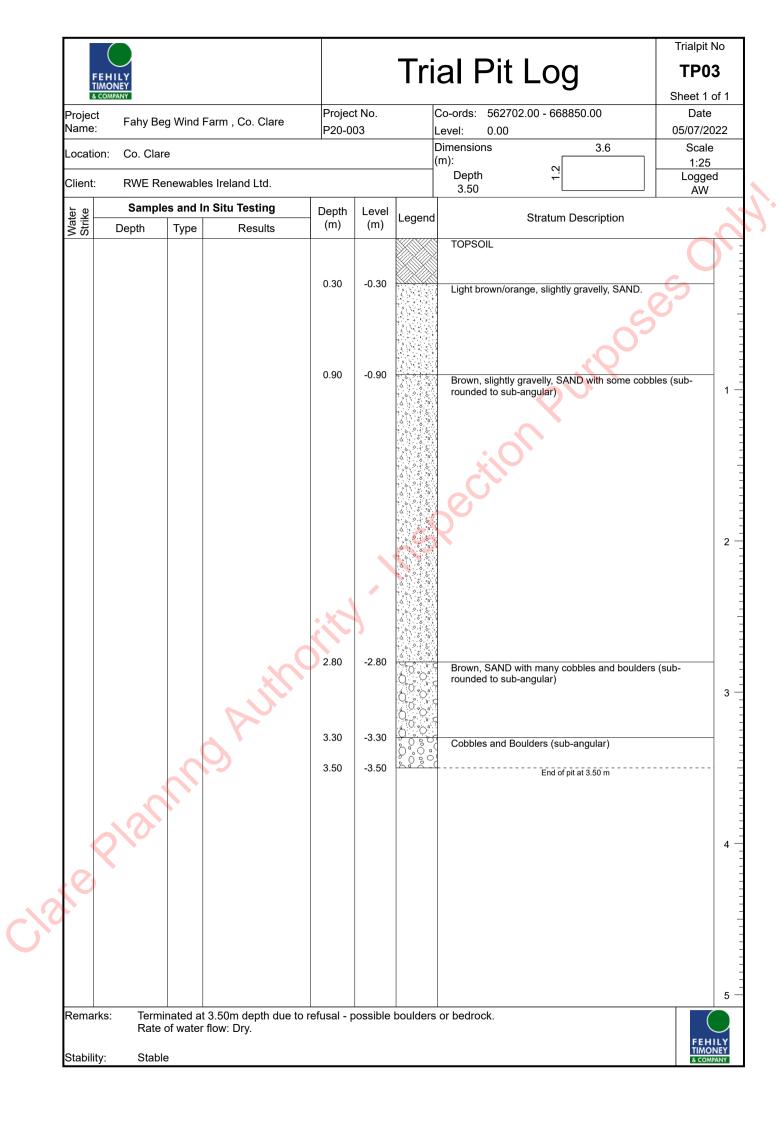
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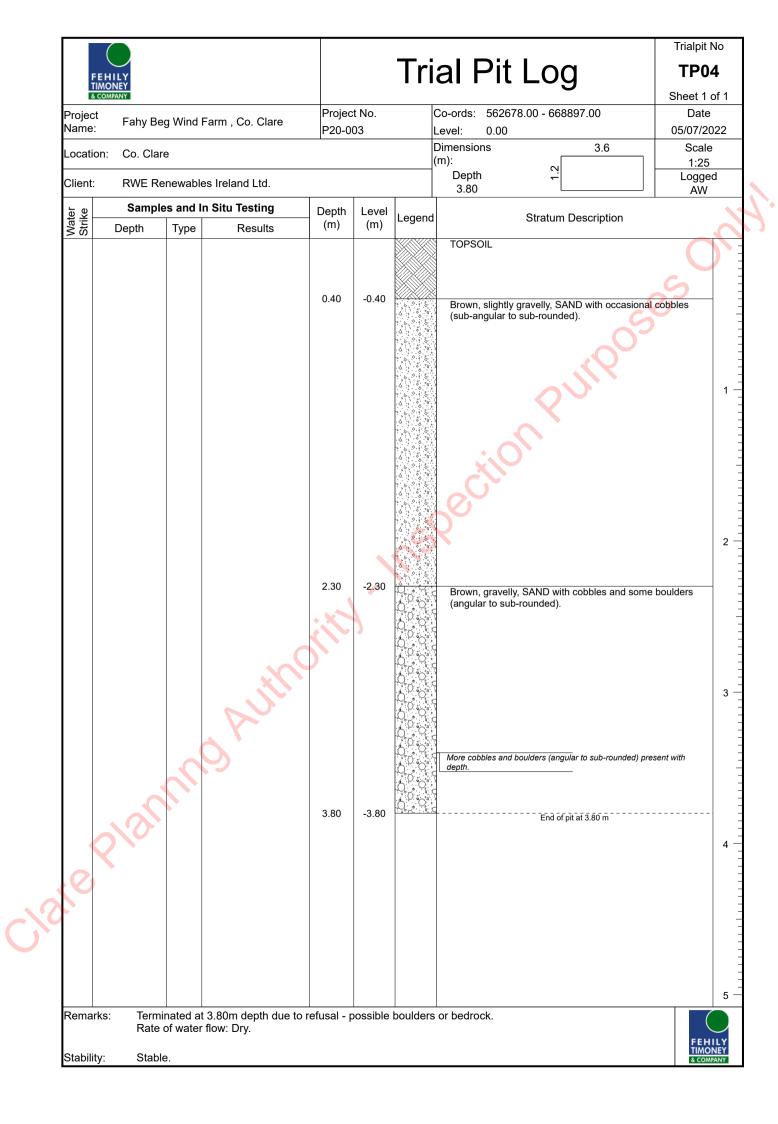
# APPENDIX 9.3

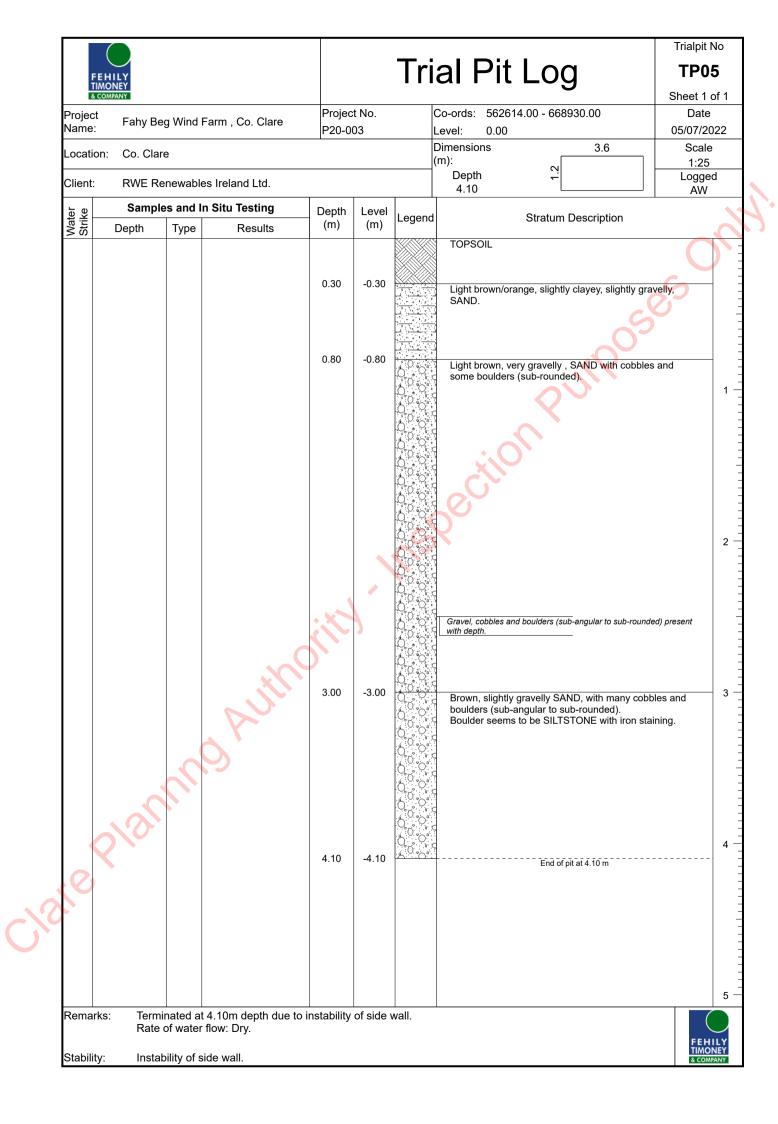
Trial Pit Logs at Proposed Substation Locations clare planno Authority

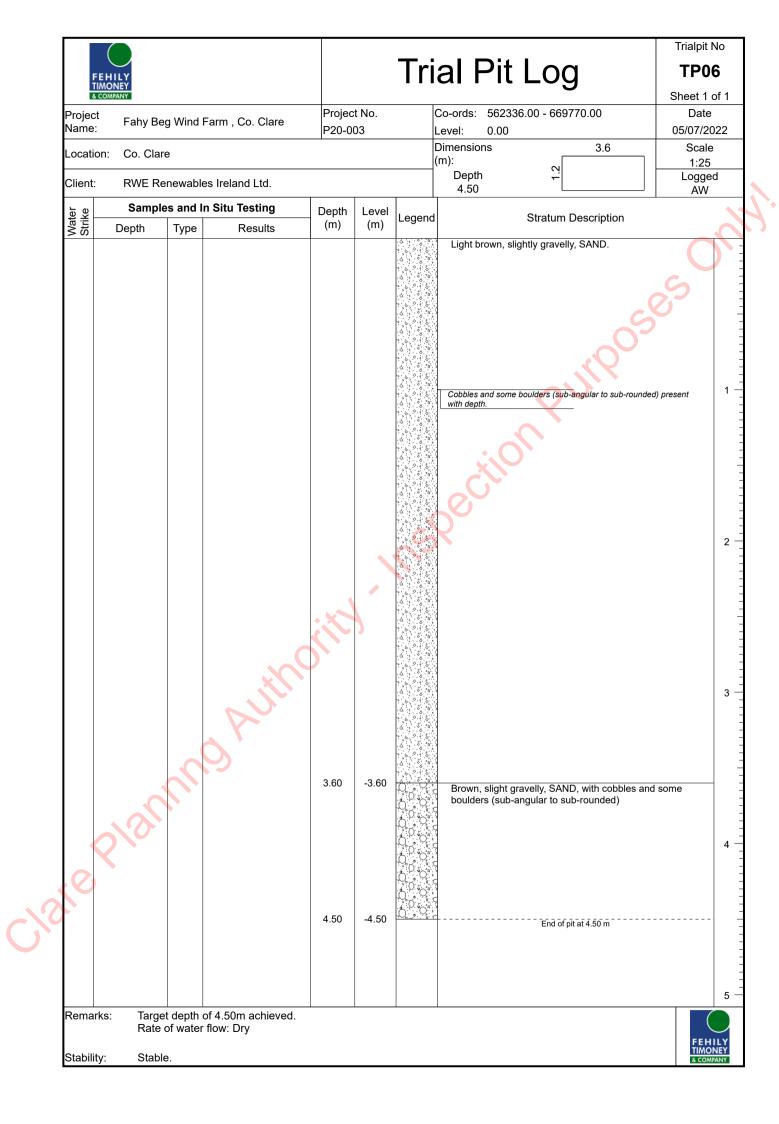


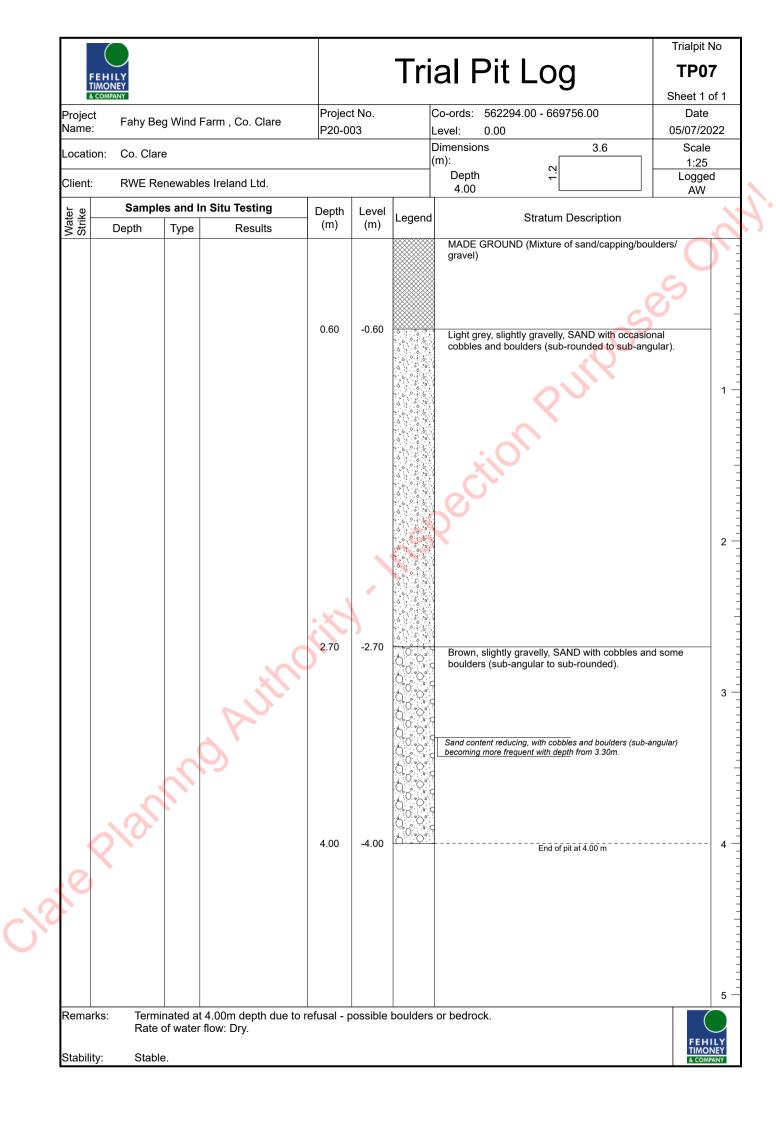


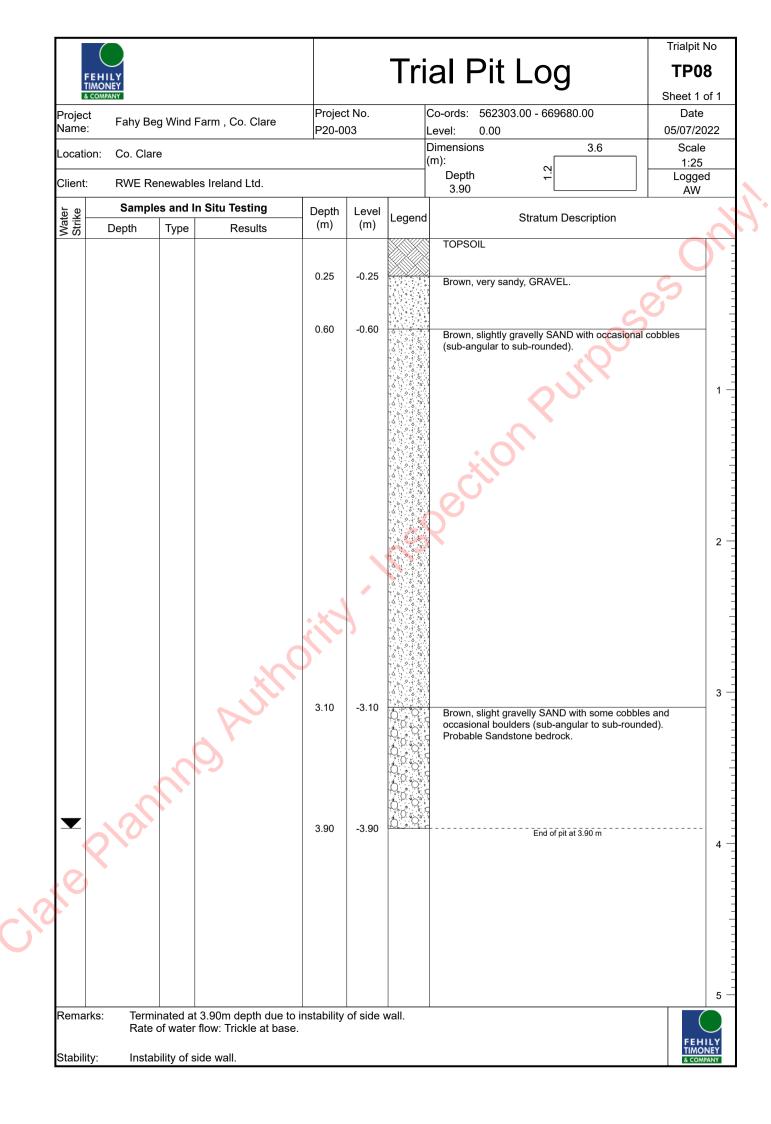














## CONSULTANTS IN ENGINEERING, **ENVIRONMENTAL SCIENCE & PLANNING**

www.fehilytimoney.ie date planno Autin

> **ORK OFFICE** Core House Pouladuff Road, Cork, T12 D773, Ireland +353 21 496 4133

**O** Dublin Office J5 Plaza,

North Park Business Park, North Road, Dublin 11, D11 PXTO, Ireland +353 1 658 3500

**Carlow Office** Unit 6, Bagenalstown Industrial Park, Royal Oak Road,

Co. Carlow, R21 XW81,

Muine Bheag,

Ireland +353 59 972 3800

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